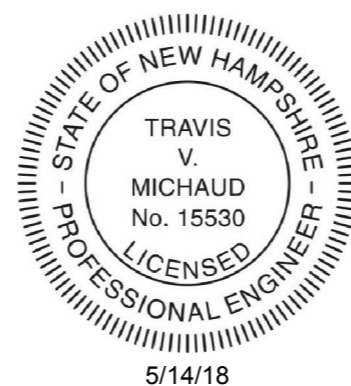


400112 KUA OFFICE

ALLIED STEEL BUILDINGS

FO# 21767
 Building 2 of 2



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GENERAL

All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 2.1)

DESIGN RESPONSIBILITY

The manufacturer is responsible only for the structural design of the Metal Building System it sells to the purchaser / customer. Neither the manufacturer nor the manufacturer's engineer is the design professional or engineer of record for the construction project. The manufacturer is not responsible for the design of any component or materials not sold by it, or their interface and connection with Metal Building System unless such design responsibility is specifically required by the order documents. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.1)

FOUNDATION DESIGN AND ANCHOR BOLTS

The manufacturer is not responsible for the design, materials, and workmanship of the foundation. The anchor bolt plans prepared by the manufacturer are intended to show only the anchor bolt location, diameter (based on ASTM A36 bolts), and quantity required to connect the Metal Building System to the foundation. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.2). It is the responsibility of the end customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and / or associated items embedded in the concrete foundation, as well as foundation design based on the loads imposed by the Metal Building System, or other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.2) U.S.-Anchor bolts shall be accurately set to a tolerance of +/- 1/8 in both elevation and location (AISC Code of Standard Practice for Steel Buildings and Bridges). Canada -Anchor bolts shall be accurately set in accordance with CISC Code of Standard Practice, June 2008, Clause 7.7.1

ADJACENT EXISTING BUILDINGS

The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.5)

SHOP-PRIMED STEEL

All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC-SP2 (Steel Manual, Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud and other contamination of the primer film. Repairs of damage to primed surfaces and or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, June 2008, Clause 6.8; (MBMA 2012 Metal Building Systems Manual, Part IV, Section 4.2.4).

ERECTION-GENERAL

The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, June 2008, Clause 7.2; (MBMA 2012 Metal Building System Manual, Part IV, Section 6.9).

The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (February 2012), and / or the Seam-Lok Technical - Erection manual (May 2012) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field-erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection.

The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 7.10.3; CISC Code of Standard Practices, June, 2008, Clause 1.5; MBMA 2012 Metal Buildings System Manual, Part IV, Section 6.2.1.5).

ERECTION TOLERANCES

U.S. ; Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1:500. (AISC Code of Standard Practice for Steel Buildings and Bridges April 14, 2010 Section 7.13.1; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.8) Canada; Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. (CISC Handbook of Steel Construction, Tenth Edition, Second Revised Printing, Part 1, Clause 29.3; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.8)

BOLT TIGHTENING

The proper tightening and inspection of all fasteners is the responsibility of the erector (Reference RCSC for structural joints using high strength bolts; August 1, 2014). All high strength (ASTM F3125, A325, A490) bolts and nuts must be tightened by the "turn-of-the-nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation procedures are compatible prior to the start of erection (CISC Handbook of Steel Construction, Tenth Edition, Second Revised Printing, Part 1, Clause 23.8.2), (MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.9).

MATERIALS	ASTM DESIGNATION	MINIMUM YIELD	MATERIALS	ASTM DESIGNATION	MINIMUM YIELD
Hot-Rolled Mill Sections	A 36, A 572, A 992	Fy = 36 ksi and/or 50 ksi	Roof and Wall Sheeting	A 792, Gr. 50 Class 1 A 792, Gr. 80	Fy = 50 ksi Fy = 80 ksi
Structural Steel Plates	A 572, A 1011	Fy = 55 ksi	Mild Steel Bolts	A 307	Fy = 36 ksi
Structural Steel Bars	A 572 or A 529	Fy = 55 ksi	High Strength Bolts	F3125: A 325-N A 490-N	Fy = 92 or 81 ksi N/A
Cold Formed Light Gauge Shapes	A 653 Gr. 55	Fy = 55 ksi	Anchor Rods (If supplied)	A 36	Fy = 36 ksi
Cable Bracing	A 475, EHS	N/A	Pipe and Hollow Structural Sections	A 500 Gr. B	Fy = 42 ksi, 46 ksi
Rod Bracing	A 36	Fy = 36 ksi			

CORRECTION OF ERRORS AND REPAIRS

The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping, and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 7.14; CISC Code of Standard Practice, June 2008, Clause 7.15; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.10).

DRAWING DISCREPANCIES

In case of discrepancies between the manufacturers steel plans and plans for other trades, the manufacturers steel plans govern. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 3.3; CISC Code of Standard Practice, June 2008, Clause 3.4; MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.1).

DELIVERIES

Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am - 12pm (morning) and 12pm - 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.

SHORTAGES

The purchaser /customer should make an inspection upon arrival of all building components. The purchaser/customer must note on the freight bill any missing item(s) and notify the manufacturers customer service department immediately; otherwise, the manufacturer cannot be held responsible for any shortages. If any item is damaged, note on the bill of lading and file a claim with the freight agent. Concealed shortages must be reported to the manufacturers customer service department within the following time frames (date from receipt of first delivery), based on the project shipment size, i.e., number of truck loads used in delivery.
1 to 3 loads...2 weeks 4 loads and over...3 weeks The manufacturers responsibility for shortages expires at the end of these time periods.

FABRICATION ERRORS

The purchaser/customer is responsible for contacting the customer service department to advise the manufacturer of fabrication problems and corresponding cost estimates. The manufacturer will be responsible for providing the builder with verbal approval to proceed with appropriate field corrections. This will be done in a timely manner. IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT THE MANUFACTURERS APPROVAL, HE DOES SO AT HIS OWN RISK. The manufacturer shall not be responsible for any claims where the purchaser/customer has not documented the problem, its correction, and reasonable costs for repair, and submitted this documentation for payment within 30 days of the occurrence.

INVOICE PAYMENT

By acceptance of the materials of services set forth in the invoice, the purchaser/customer agrees to pay the invoice amount within the time period specified on the invoice. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE.

SAFETY PROCEDURES

The manufacturer is committed to manufacturing a quality product that can be erected safely. Although good job site practices and a commitment to safety by the erector are beyond the control of the manufacturer, the manufacturer highly recommends the erector provide good, safe working conditions on the job site. The erector should follow all local, state, and federal health and safety regulations at all times. Accident prevention practices should be implemented and each employee should know emergency procedures. The manufacturer also recommends daily meetings to discuss erection safety procedures. For additional information concerning federal health and safety regulations, contact the occupational safety and health administration (osha).

U.S. Department of Labor
Occupational Safety and Health Administration
200 Constitution Avenue, N.W.
Washington, DC 20210
www.osha.gov

The manufacturer shall not be responsible for personal injury or property damage as a result of failure to follow all applicable safety regulations and material handling and installation recommendations.



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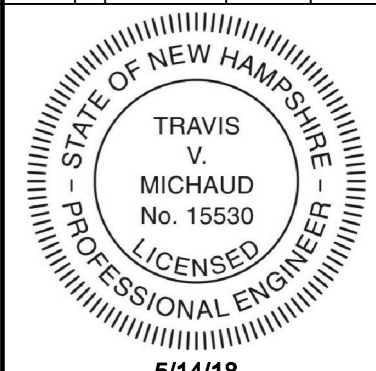
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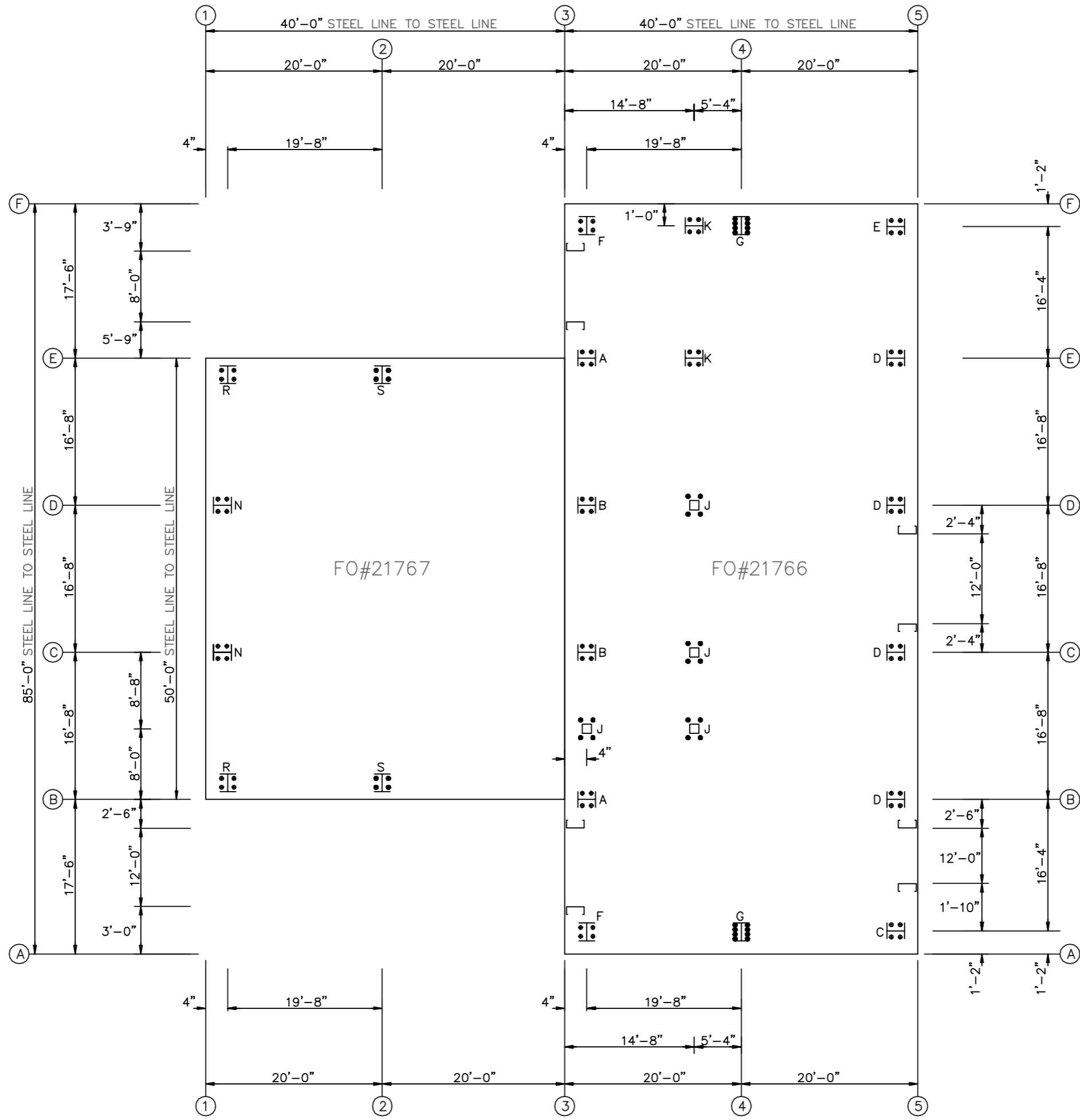
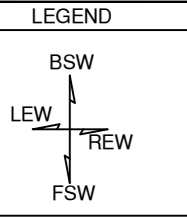
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5/14/18



ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)
 Finished Floor @ 100'-0"



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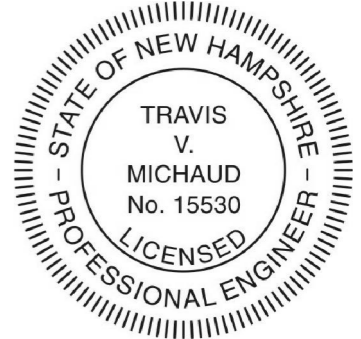
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01	UPDATED BUILDING DIMENSIONS
	DATE 5-10-18

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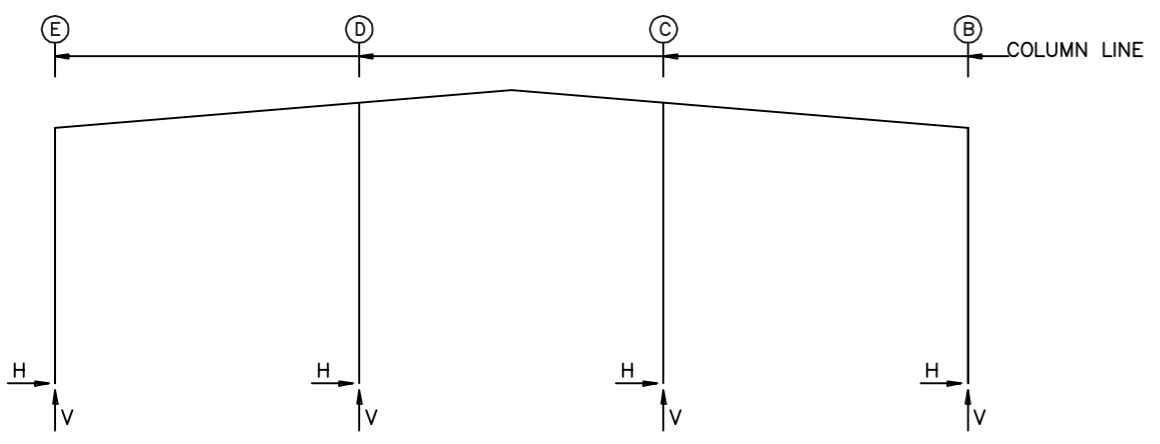
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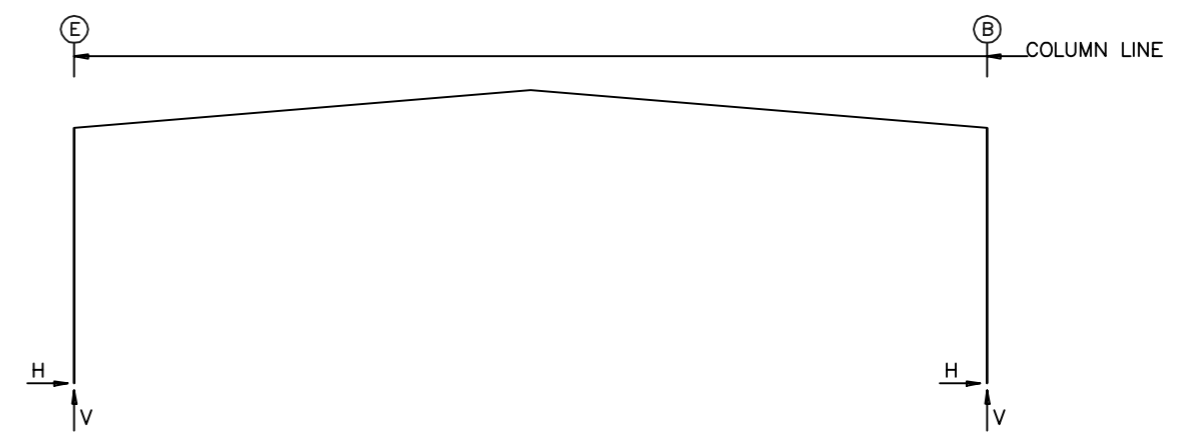


5/14/18

FRAME LINES: 1



FRAME LINES: 2



RIGID FRAME: BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead		Collateral		Live		Snow		Wind_Left1		Wind_Right1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	E	0.1	1.0	0.1	0.6	0.3	2.5	0.9	7.8	-1.8	-3.6	1.7	-0.7
1	B	-0.1	1.0	-0.1	0.6	-0.3	2.5	-0.9	7.8	-1.7	-0.7	1.8	-3.6
1	D	0.0	2.0	0.0	1.4	0.0	5.5	0.0	17.4	0.0	-4.4	0.0	-4.8
1	C	0.0	2.0	0.0	1.4	0.0	5.5	0.0	17.4	0.0	-4.4	0.0	-4.8

Frame Line	Column Line	Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left		Seismic_Right	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	E	-2.3	-2.6	1.2	0.3	0.8	-2.3	0.3	-1.3	-1.2	-1.3	1.2	1.3
1	B	-1.2	0.3	2.3	-2.6	-0.8	-2.3	-0.3	-1.3	-1.2	1.3	1.2	-1.3
1	D	0.0	-2.8	0.0	-3.1	0.0	-4.1	0.0	-2.5	0.0	1.9	0.0	-1.9
1	C	0.0	-3.1	0.0	-2.8	0.0	-4.1	0.0	-2.5	0.0	-1.9	0.0	1.9

Frame Line	Column Line	LWIND1_L2E		LWIND1_R2E		LWIND2_L2E		LWIND2_R2E		F1UNB_SL_L		F1UNB_SL_R	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
1	E	0.0	-0.7	-0.1	-0.1	0.0	-0.7	-0.1	-0.1	0.6	9.0	0.7	2.7
1	B	0.1	-0.1	0.0	-0.7	0.1	-0.1	0.0	-0.7	-0.7	2.7	-0.6	9.0
1	D	0.0	-0.3	0.0	0.2	0.0	-0.3	0.0	0.2	0.0	22.2	0.0	5.5
1	C	0.0	0.2	0.0	-0.3	0.0	0.2	0.0	-0.3	0.0	5.5	0.0	22.2

Frame Line	Column Line	Dead		Collateral		Live		Snow		Snow_Drift		Wind_Left1	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	E	2.8	4.8	2.1	3.3	8.5	13.0	26.8	40.9	2.4	3.6	-7.3	-10.1
2	B	-2.8	4.8	-2.1	3.3	-8.5	13.0	-26.8	41.0	-2.4	3.6	3.0	-7.0

Frame Line	Column Line	Wind_Right1		Wind_Left2		Wind_Right2		Wind_Long1		Wind_Long2		Seismic_Left	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	E	-3.0	-7.0	-5.6	-5.8	-1.3	-2.6	-4.8	-10.5	-3.1	-6.1	-1.8	-0.9
2	B	7.3	-10.1	1.3	-2.6	5.6	-5.8	4.8	-10.5	3.1	-6.1	-1.8	0.9

Frame Line	Column Line	Seismic_Right		LWIND1_L2E		LWIND1_R2E		LWIND2_L2E		LWIND2_R2E		F2UNB_SL_L	
		Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert	Horiz	Vert
2	E	1.8	0.9	-0.1	-1.3	-0.2	-0.1	-0.1	-1.3	-0.2	-0.1	21.2	41.8
2	B	1.8	-0.9	0.2	-0.1	0.1	-1.3	0.2	-0.1	0.1	-1.3	-21.3	22.2

Frame Line	Column Line	F2UNB_SL_R	
		Horiz	Vert
2	E	21.3	22.2
2	B	-21.2	41.8

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc_Bolt Qty	Bolt Dia	Base_Plate (in)			Grout (in)
				Width	Length	Thick	
1	E	4	0.750	8.000	9.875	0.500	0.0
1	B	4	0.750	8.000	9.875	0.500	0.0
1	D	4	0.750	8.000	8.140	0.500	0.0
1	C	4	0.750	8.000	8.140	0.500	0.0

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc_Bolt Qty	Bolt Dia	Base_Plate (in)			Grout (in)
				Width	Length	Thick	
2	E	4	1.000	8.000	16.81	0.500	0.0
2	B	4	1.000	8.000	16.81	0.500	0.0



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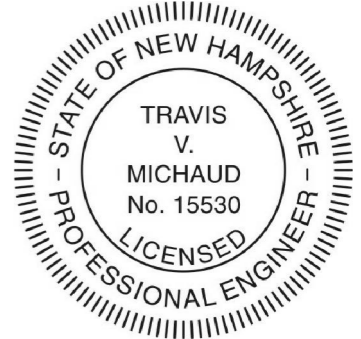
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ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Wind Press Horz	Wind Suct Horz
1	D	-1.8	2.0
1	C	-1.8	2.0

ANCHOR BOLT SUMMARY

Qty	Locate	Dia (in)	Type
8	Endwall	3/4"	
8	Frame	3/4"	
8	Frame	1"	

BUILDING BRACING REACTIONS

Loc	Wall Line	Col Line	Reactions in plane of wall ± Reactions(k)				Panel Shear (lb/ft)	Note
			Wind Horz	Wind Vert	Seismic Horz	Seismic Vert		
L_EW	1						(h)	
F_SW	B		Braced by Adjacent					
R_EW	3		Braced by Adjacent					
B_SW	E		Braced by Adjacent					

(h) Rigid frame at endwall

DESIGN INFORMATION

- All loading conditions are examined and only the maximum / minimum H or V and the corresponding H or V are reported.
- Positive reactions are shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:

DESIGN CRITERIA	SEISMIC CRITERIA	DEFLECTION LIMITS
Width (ft) = 50	Seismic Importance = 1.00	ENDWALL COLUMN L/ 120
Length (ft) = 40.67	Occupancy Category = II - Normal	ENDWALL RAFTER (Live) L/ 180
Eave Height (ft) = 14		ENDWALL RAFTER (Wind) L/ 180
Roof Slope (rise/12) = 1.0:12	Mapped Spectral Response Accelerations	WALL GIRTS L/ 90
Building Code = IBC 09	Ss = 0.2960	PURLIN (LIVE) L/ 150
Local Code (State/Prov) = IBC 09	S1 = 0.0820	PURLIN (WIND) L/ 150
Dead Load (psf) = 5.350	---Spectral Response Coefficients---	WALL PANEL L/ 90
Collateral Load (psf) = 5.00	Sds = 0.3085	ROOF PANEL (Live) L/ 120
Roof Live Load (psf) = 20.00	Sd1 = 0.1312	ROOF PANEL (Wind) L/ 120
Frame Live Load (psf) = 20.00	Site Class = D	Main Frame (Horiz) L/ 60
	Seismic Design Category = B	Main Frame (Vert) L/ 180
Snow:	-----Base Shear-----	WIND BRACING L/ 60
Ground Snow Load (psf) = 90.00	Expanded Formula = 0.667*ie*Fa*Ss*W/R	Main Frame (Crane) L/ 100
Snow Importance = 1.00	Longitudinal Base Shear = 0.00	Main Frame (Seismic) L/ 50
Thermal Coefficient = 1.00	Transverse Base Shear = 6.10	SEISMIC BRACING L/ 50
Snow Exposure Factor = 1.00		PARTITION COLUMN L/ 120
Slippery Roof = N	---Seismic Response Coefficients---	PARTITION GIRT L/ 120
Roof Snow Load (psf) = 63	Frame = 0.103	PARTITION PANEL L/ 120
	FSW = 0.103	
Wind:	BSW = 0.103	
Ultimate Wind Speed (mph) = 100 mph	---Response Modification Factors---	
Occupancy Category = II - Normal	Frame = 3	
Importance - Wind = 1.00	FSW = 3	
Wind Exposure = C	BSW = 3	
Enclosure Classification = C		
---Internal Pressure Coefficients---		
Pressure = 0.18		
Suction = -0.18		
----Components & Cladding----		
Design Pressure: Pressure (psf) = 19.98		
Suction (psf) = -26.58		
Equivalent Lateral Brace Force Procedure.		
Steel systems not specifically detailed for seismic resistance.		



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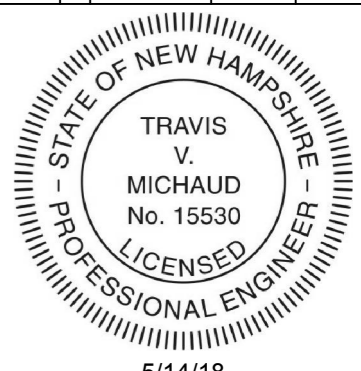
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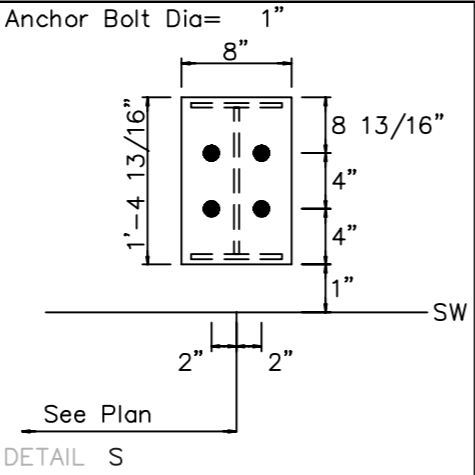
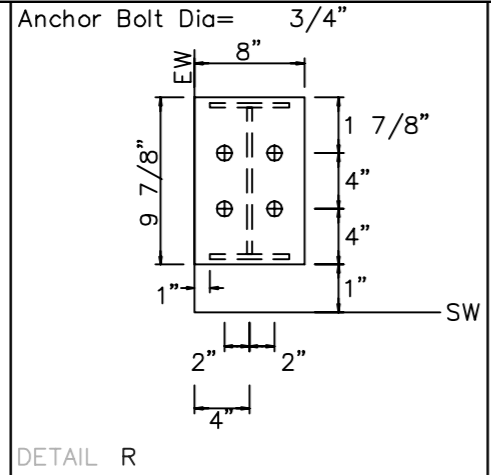
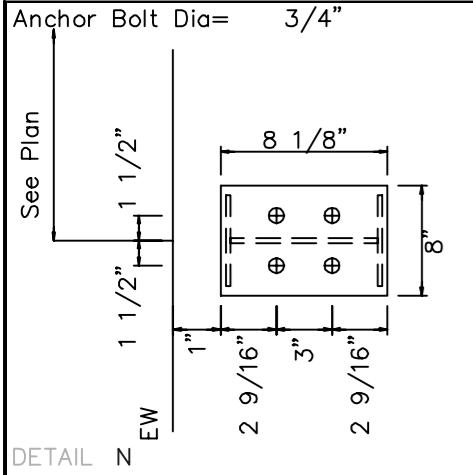
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FOR CONSTRUCTION: FINAL DRAWINGS.

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ADDITIONAL LOADING INFORMATION

Mezzanine Loads:

Dead Load _____ PSF

Collateral Load _____ PSF

Live Load _____ PSF

Crane Information:

Crane Type _____

CMAA Service Class _____

Crane capacity = _____ Kips

Bridge Weight = _____ Kips

Hoist/Trolley Weight = _____ Kips

Wheel Spacing = _____ Ft.

Additional Loads:

1. _____

2. _____

3. _____

ALLIED

400112 KUA OFFICE

50'-0" x 40'-8" x 14'-0"

DATE: 5/ 8/18 REVISION: 0

ENG: TVM DWN: BJC APPD: TVM

F.O. 21767

REVISION HISTORY	
REV.	DESCRIPTION

400112 KUA OFFICE

DRAWING STATUS	REVISION HISTORY						
<p><input checked="" type="checkbox"/> FOR APPROVAL: THESE DRAWINGS, BEING FOR APPROVAL, ARE BY DEFINITION NOT FINAL AND ARE FOR CONCEPTUAL REPRESENTATION ONLY. THEIR PURPOSE IS TO CONFIRM PROPER INTERPRETATION OF THE PROJECT DOCUMENTS. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p><input type="checkbox"/> FOR PERMIT: THESE DRAWINGS, BEING FOR PERMIT, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p> <p><input type="checkbox"/> FOR CONSTRUCTION: THESE DRAWINGS, BEING FOR CONSTRUCTION, ARE BY DEFINITION NOT FINAL. ONLY DRAWINGS ISSUED "FOR CONSTRUCTION" CAN BE CONSIDERED AS COMPLETE.</p>	<table border="1"> <thead> <tr> <th>REV.</th> <th>DESCRIPTION</th> <th>DATE</th> </tr> </thead> <tbody> <tr> <td> </td> <td> </td> <td> </td> </tr> </tbody> </table>	REV.	DESCRIPTION	DATE			
REV.	DESCRIPTION	DATE					

STATE OF NEW HAMPSHIRE

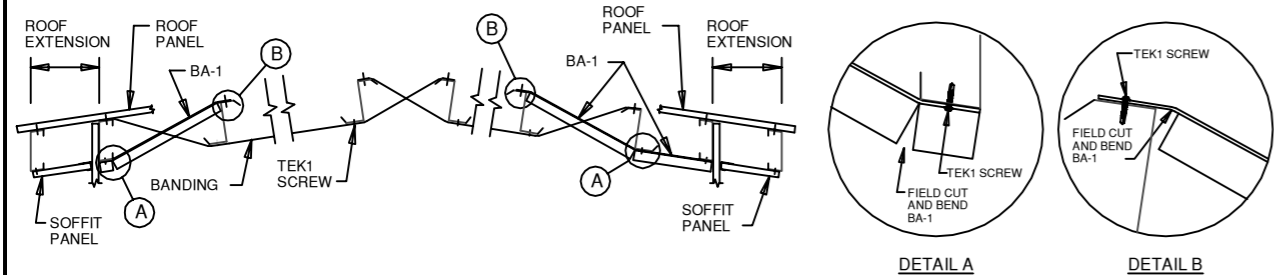
TRAVIS V. MICHAUD
No. 15530

LICENSED PROFESSIONAL ENGINEER

5/14/18

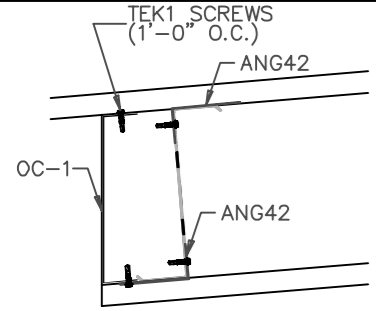
STANDARD PURLIN BRACING DETAIL FOR SCREW-DOWN PANELS

NOTE 1: SPACE BANDING EVENLY ACROSS BAYS



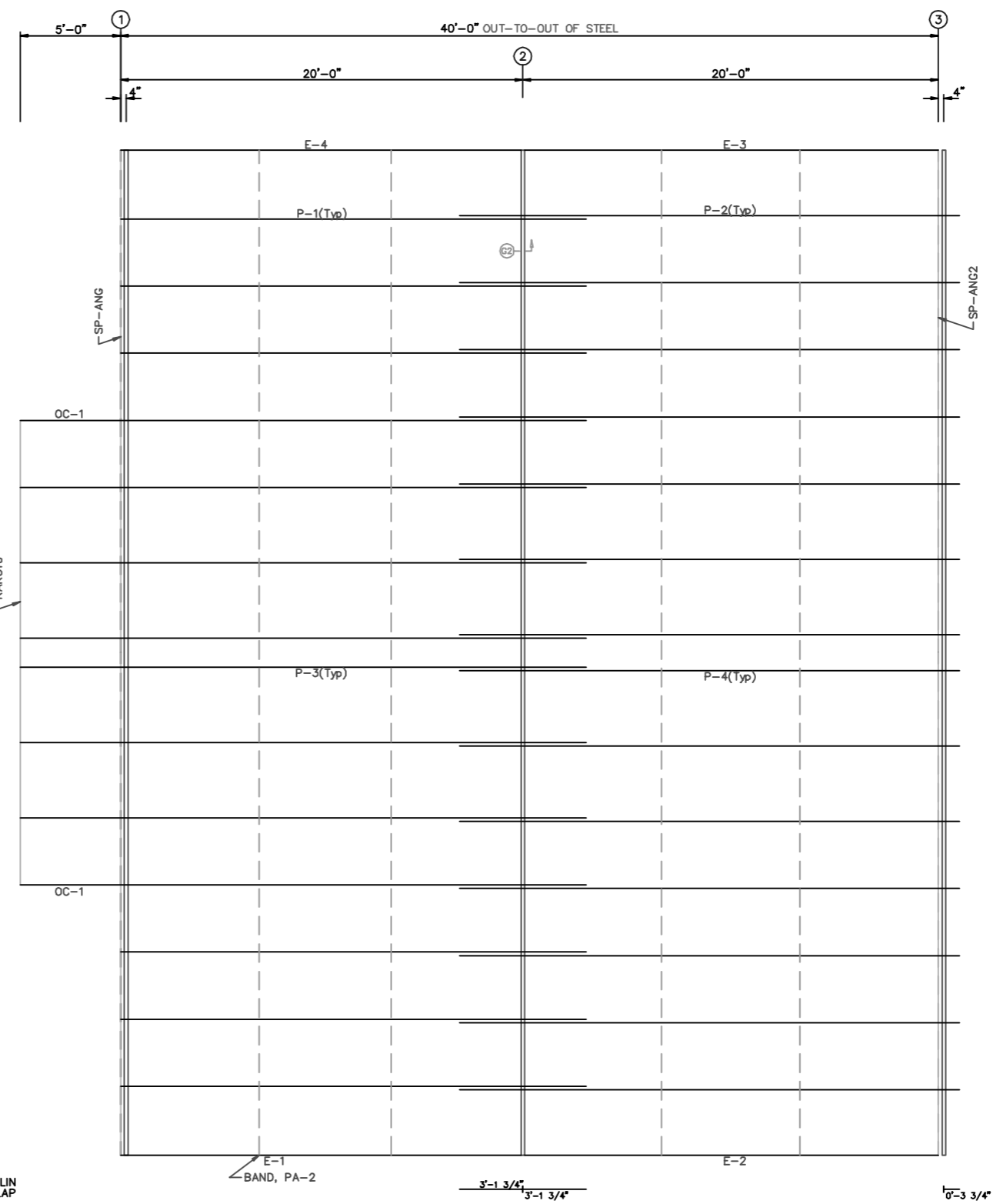
CONDITION 1:
FIRST PURLIN SPACE
GREATER THAN 2'-0"

CONDITION 2:
FIRST PURLIN SPACE LESS
THAN OR EQUAL TO 2'-0"



EXTENSION CAP DETAIL

MEMBER TABLE			
ROOF PLAN			
QUAN	MARK	PART	LENGTH
7	P-1	10X25Z13	28'-1 1/2"
7	P-2	10X25Z10	23'-9 1/2"
7	P-3	10X25Z13	28'-1 1/2"
7	P-4	10X25Z10	23'-9 1/2"
1	E-1	10X35E12	24'-11 1/2"
1	E-2	10X35E12	20'-7 1/2"
1	E-3	10X35E12	20'-7 1/2"
1	E-4	10X35E12	24'-11 1/2"



ROOF FRAMING PLAN



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50'-0" x 40'-8" x 14'-0"
DATE: 5/8/18 REVISION: 0
ENG: TVM DWN: BJC APPD: TVM

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REVISION HISTORY	
REV.	DESCRIPTION

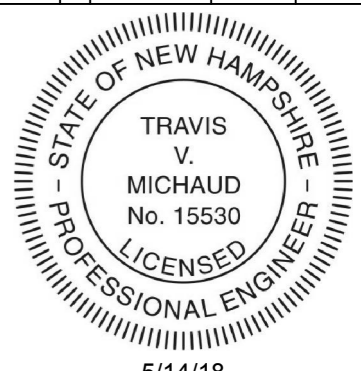
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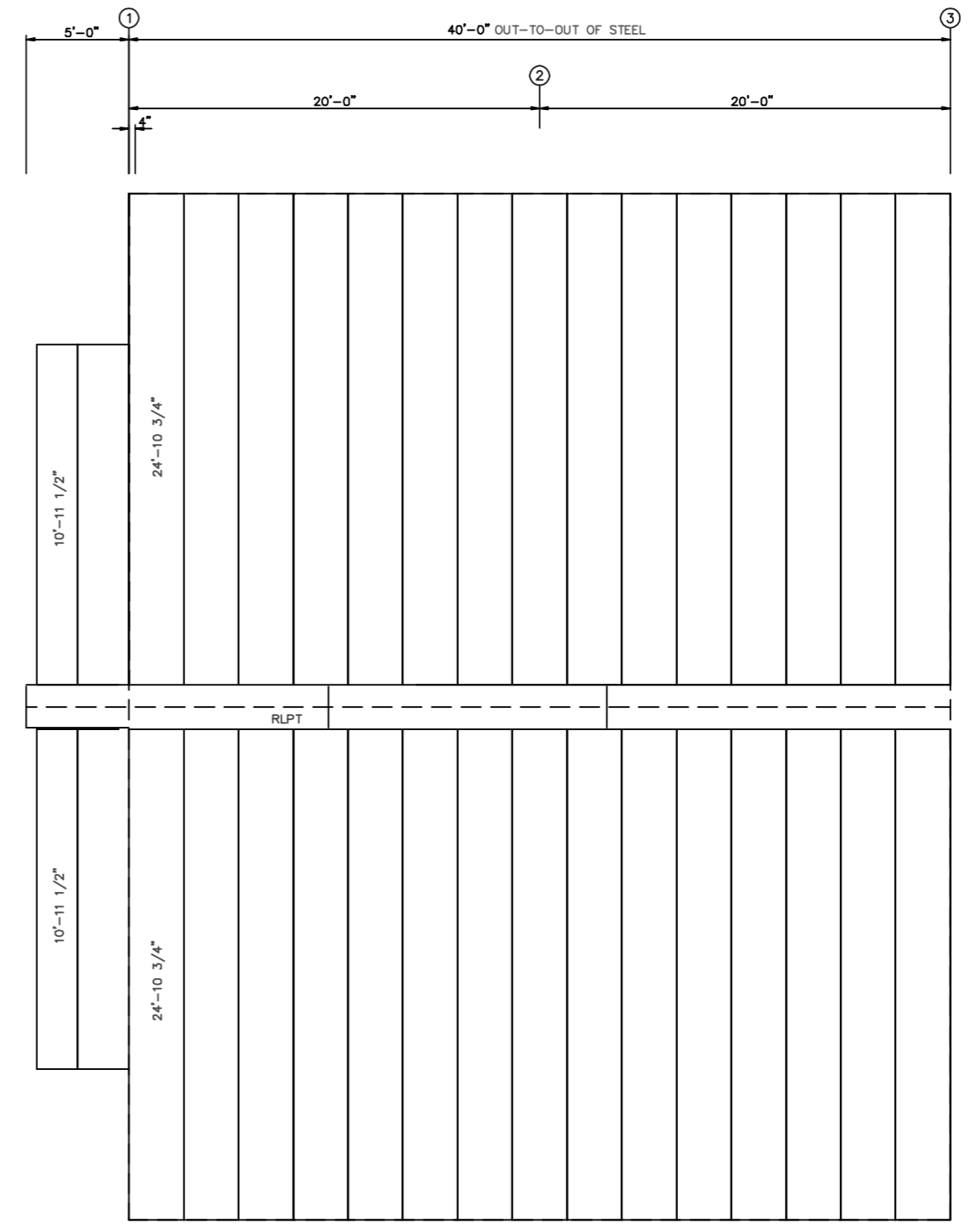
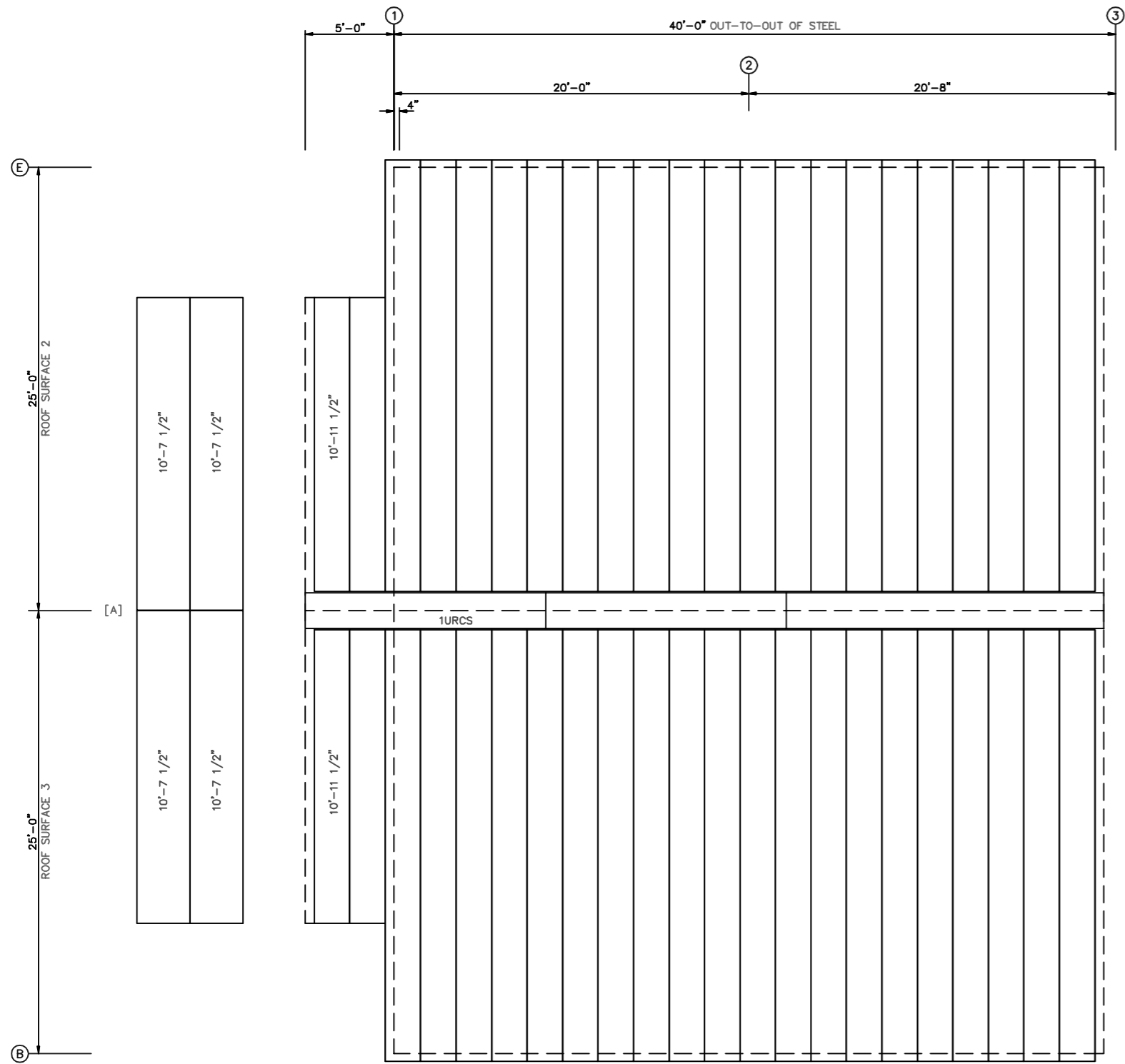
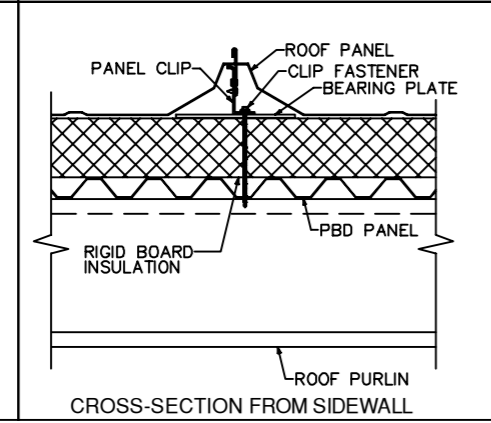
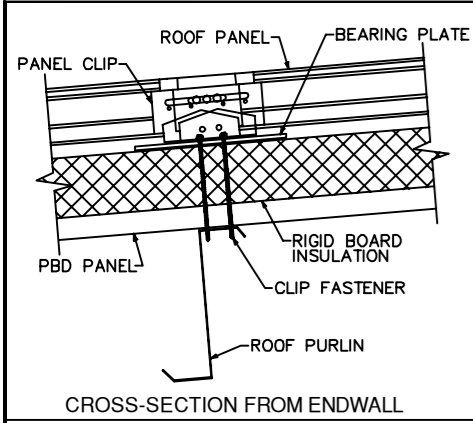
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FOR CONSTRUCTION: FINAL DRAWINGS.



- GENERAL NOTES:
1. Screw Down Roof: Use TEK5WW screws in place of SD150 panel screws at all 10 gage purlins, eave struts, or roof joists.
 2. Standing Seam Roof: Use FST#6 in place of FST#1 clip to purlin screws at all 10 gage purlins, eave struts, or at roof joists.



GENERAL NOTES:
 Panel "Start" and "End" dimensions must be followed for the proper installation of the gable trim(s) provided.

ROOF SHEETING PLAN
 PANELS: 24 Ga. L4 - Galvalume
 [A] SOFFIT PANELS: 26 Ga. RR - TBD

DECKING PLAN
 PANELS: 24 Ga. PBD - Polar White



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50'-0" X 40'-8" X 14'-0"

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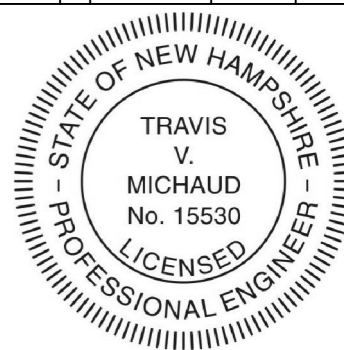
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REV.	DESCRIPTION	DATE
1	CUSTOMER REQUEST	5/11/18

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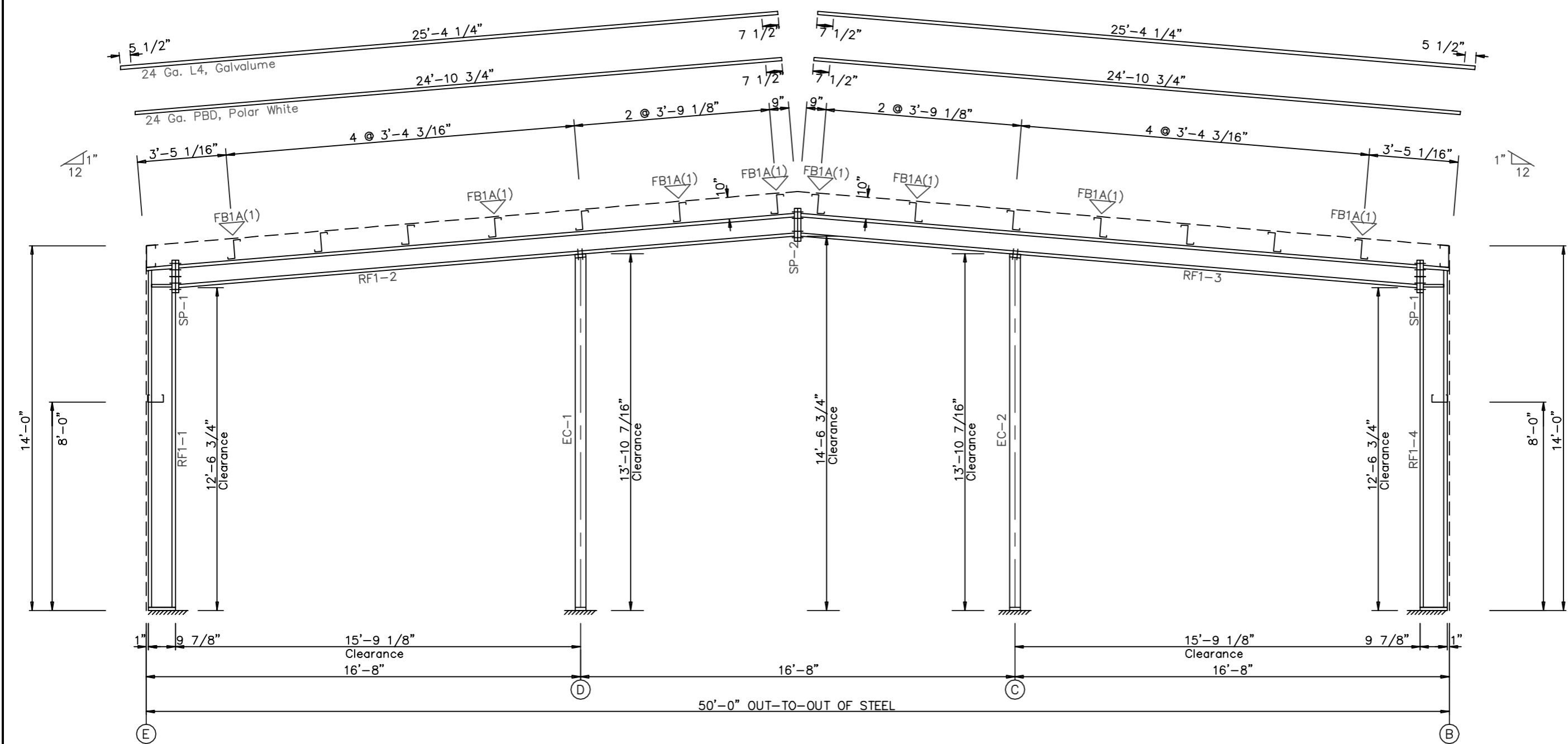


5/14/18

SPLICE BOLT TABLE							CAP PLATE BOLTS				
Mark	Qty		Type	Dia	Length		Mark	Qty	Type	Dia	Length
SP-1	6	6	0	A325	0.500	2.00	EC-1	4	A325	0.500	1.50
SP-2	4	4	0	A325	0.500	1.50	EC-2	4	A325	0.500	1.50

MEMBER SIZE TABLE		
MARK	MEMBER	LENGTH
RF1-1	W10X12	13'-2 1/16"
RF1-2	W8X18	24'-1 5/8"
RF1-3	W8X18	24'-1 5/8"
RF1-4	W10X12	13'-2 1/16"
EC-1	W8X18	13'-10 5/8"
EC-2	W8X18	13'-10 5/8"

FLANGE BRACES: Both Sides(U.N.)
 FBxxA(1)
 A - L15X1/8



BUILDING CROSS SECTION: FRAME LINE 1

- GENERAL NOTES:
- See Detail Sheets for Connection Information.
 - See Shipping List for Flange Brace Lengths.



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 50'-0" x 40'-8" x 14'-0"
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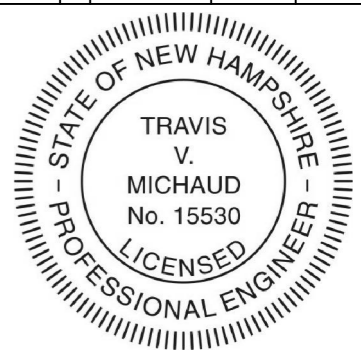
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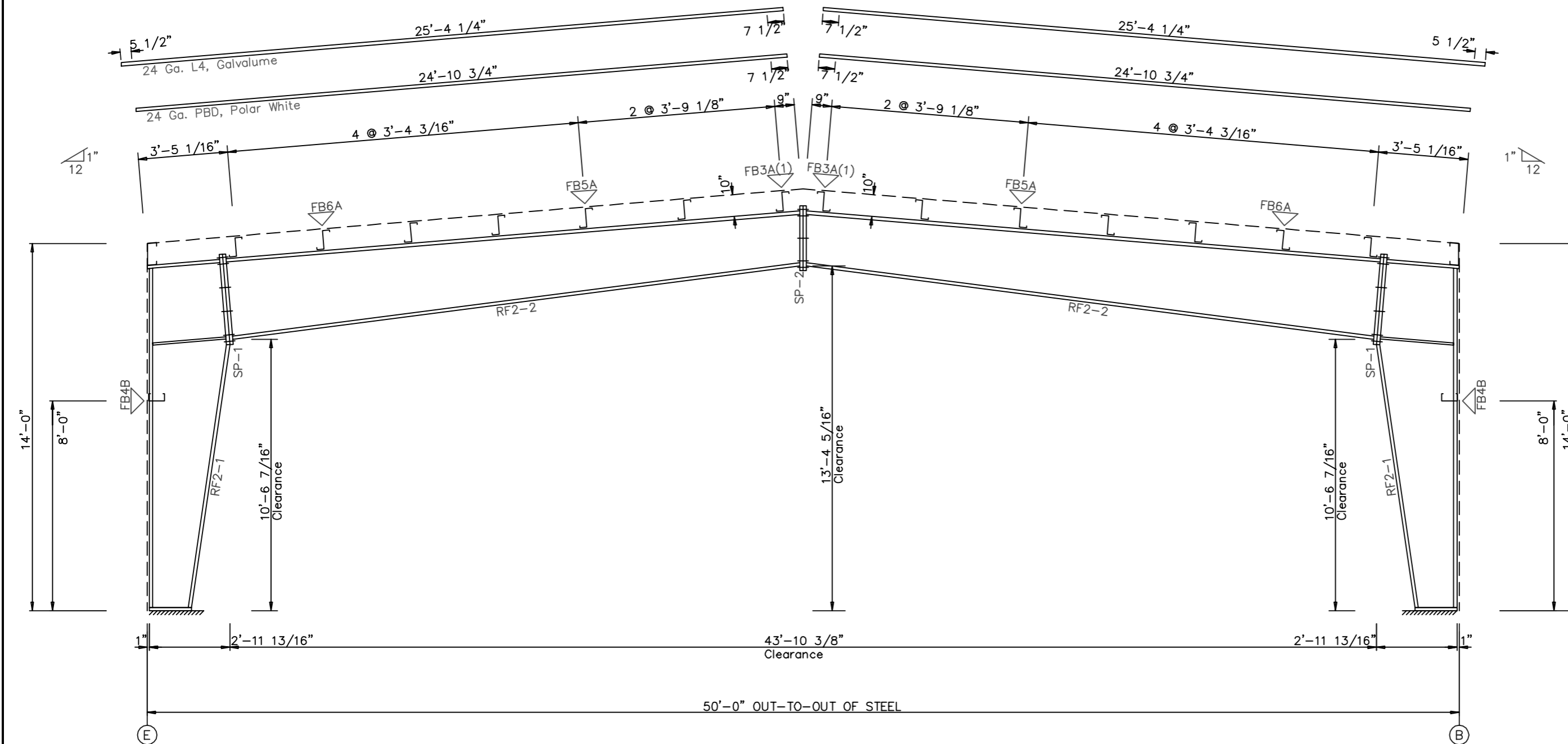


5/14/18

SPLICE BOLT TABLE						
Mark	Qty		Int	Type	Dia	Length
	Top	Bot				
SP-1	4	4	4	A325	1.000	2.75
SP-2	4	4	2	A325	0.750	2.25

MEMBER TABLE								
Mark	Web Depth		Web Thick	Plate Length	Outside Flange		Inside Flange	
	Start	End			W x Thk	Length	W x Thk	Length
RF2-1	16.0	35.0	0.250	160.0	6 x 5/16"	x 157.3	6 x 1/2"	x 123.6
RF2-2	34.0	30.1	0.313	85.6	6 x 1/4"	x 33.3	6 x 5/16"	x 85.7
	30.1	22.0	0.219	180.0	6 x 1/4"	x 145.6	6 x 1/4"	x 178.3

✓ FLANGE BRACES: Both Sides(U.N.)
 FBxxB(1)
 B - L20X3/16
 A - L15X1/8



BUILDING CROSS SECTION: FRAME LINE 2

GENERAL NOTES:
 1. See Detail Sheets for Connection Information.
 2. See Shipping List for Flange Brace Lengths.



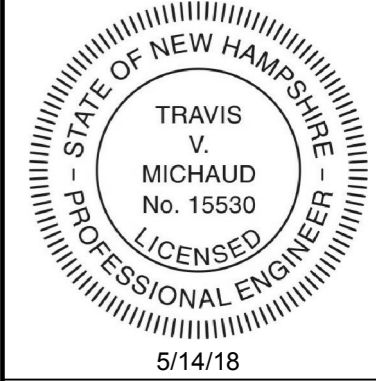
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 ENG: TVM DWN: BJC APPD: TVM

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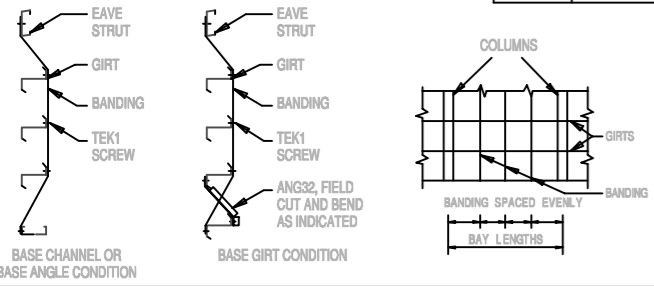
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STANDARD GIRT BRACING DETAIL
FOR SIDEWALL GIRTS

BANDING REQUIREMENTS	
BAY WIDTH	RUNS REQ'D
AS SHOWN	AS SHOWN



MEMBER TABLE			
FRAME LINE B			
QUAN	MARK	PART	LENGTH
1	G-3	8X35C13	19'-2 1/2"
1	G-4	8X35C13	20'-4 1/2"

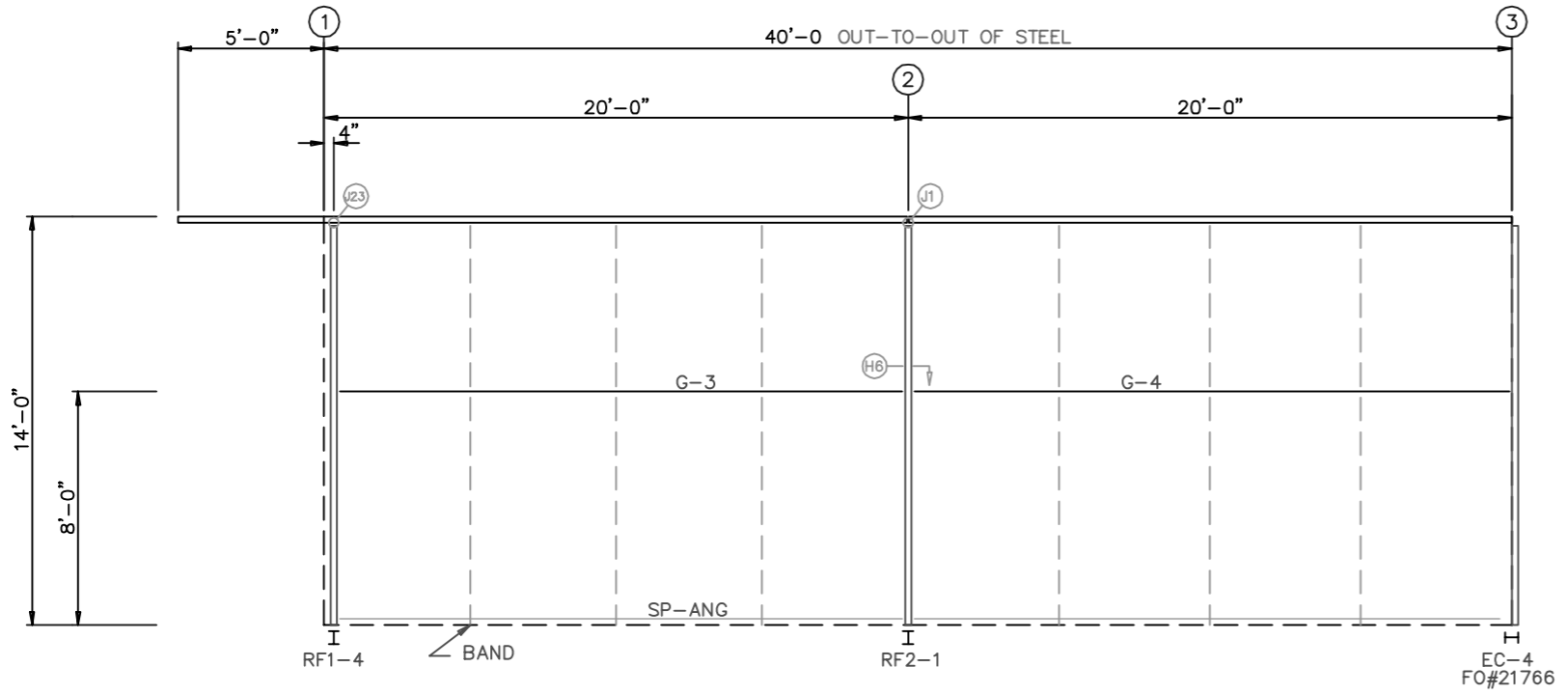
NOTE(S):
1.) WALL PANEL MUST BE FASTENED TO SUPPORT WITH MBCI FASTENING PATTERN FP2.



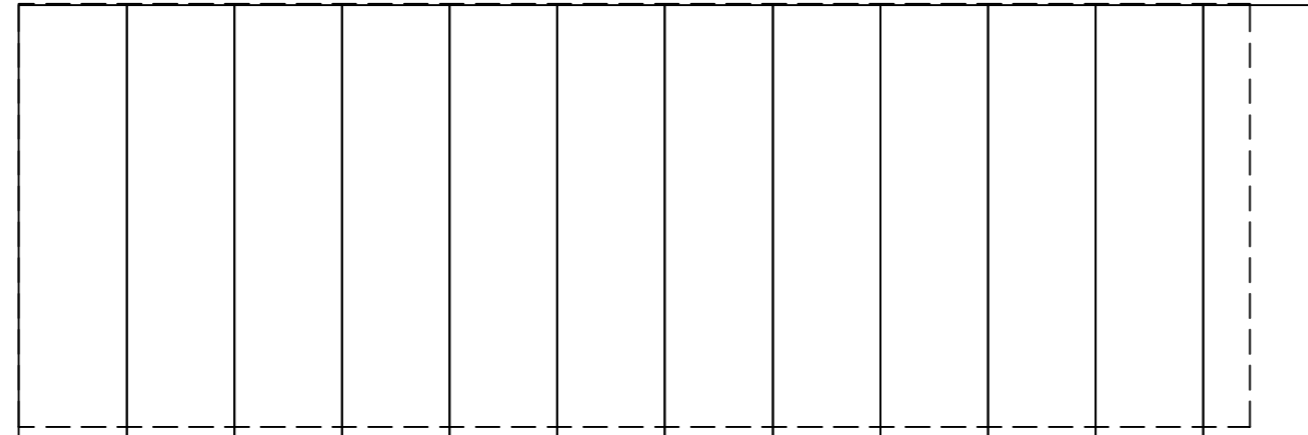
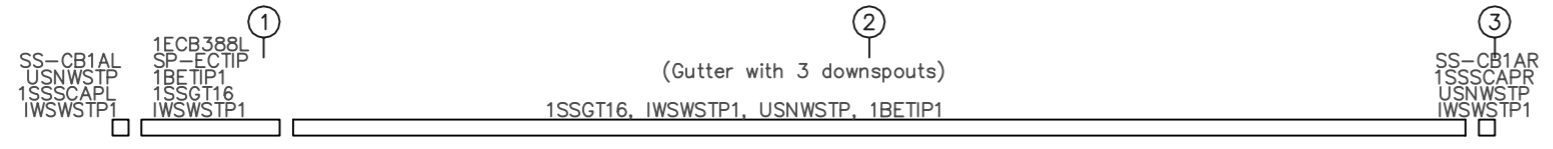
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SIDEWALL FRAMING: FRAME LINE B



SIDEWALL SHEETING & TRIM: FRAME LINE B

PANELS: 3" CF MESA 26Gα EXTERIOR & 26Gα INTERIOR -
EXT: TBD & INT: TBD

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

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REVISION HISTORY

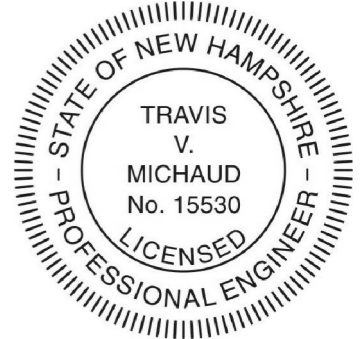
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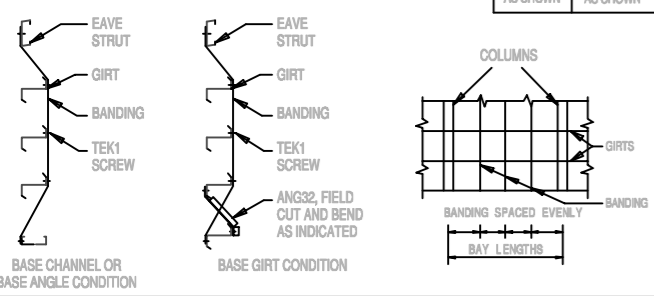
5/14/18

TRIM COLORS

EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNSPOUTS = TBD
RAKE TRIM = TBD	

* LINER TRIM = Liner panel color
* SOFFIT TRIM = Soffit panel color
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

STANDARD GIRT BRACING DETAIL FOR SIDEWALL GIRTS



MEMBER TABLE
FRAME LINE E

QUAN	MARK	PART	LENGTH
1	G-3	8X35C13	19'-2 1/2"
1	G-4	8X35C13	20'-4 1/2"

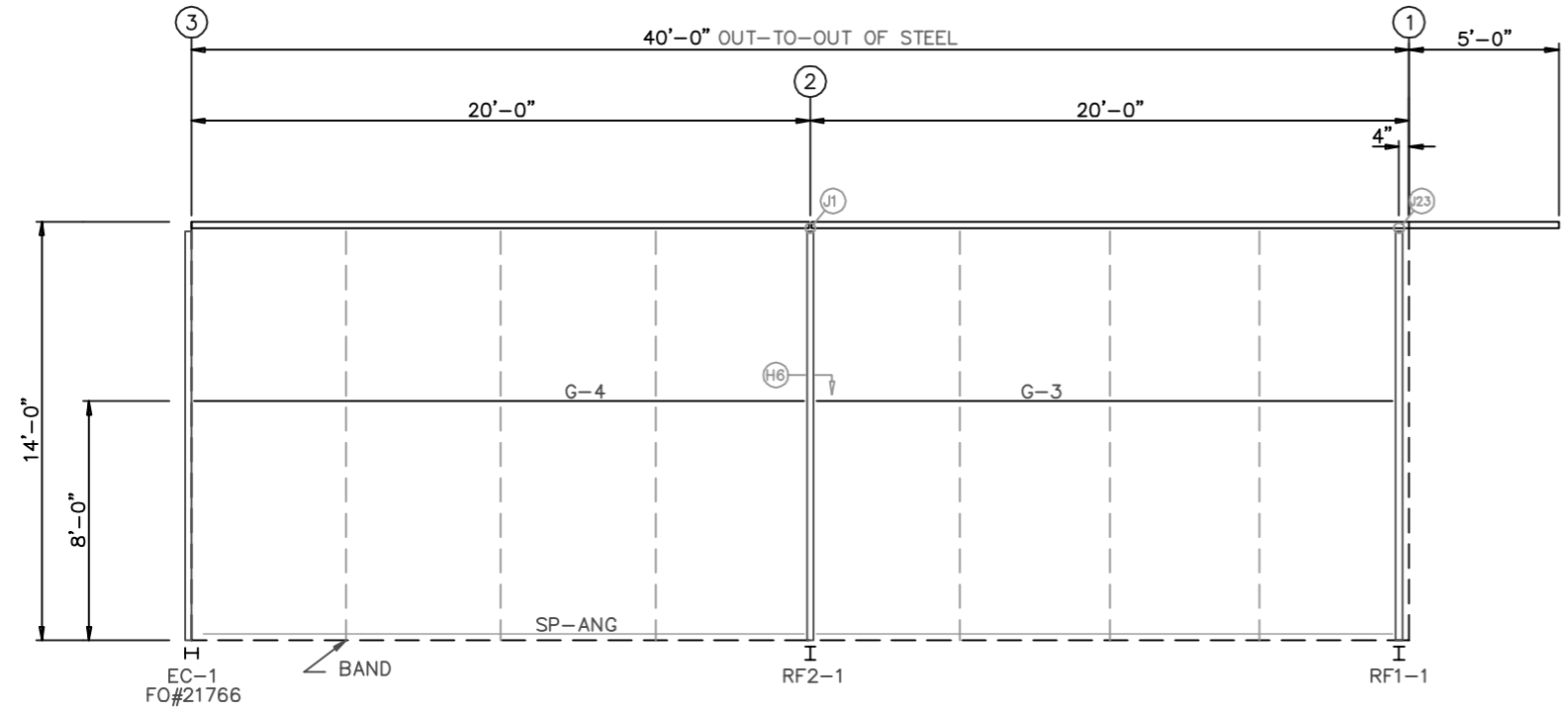
NOTE(S):
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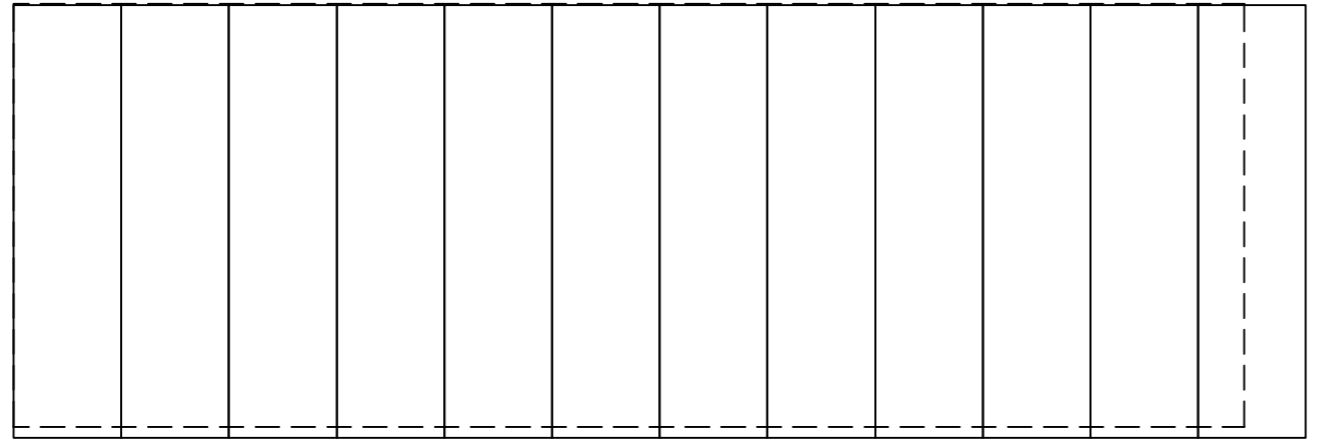
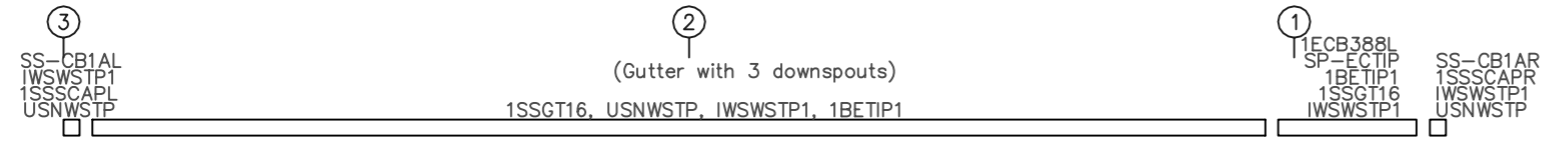
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SIDEWALL FRAMING: FRAME LINE E



SIDEWALL SHEETING & TRIM: FRAME LINE E

PANELS: 3" CF MESA 26G_a EXTERIOR & 26G_a INTERIOR -
EXT: TBD & INT: TBD

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

TRIM COLORS

EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNPOUTS = TBD
RAKE TRIM = TBD	

* LINER TRIM = Liner panel color
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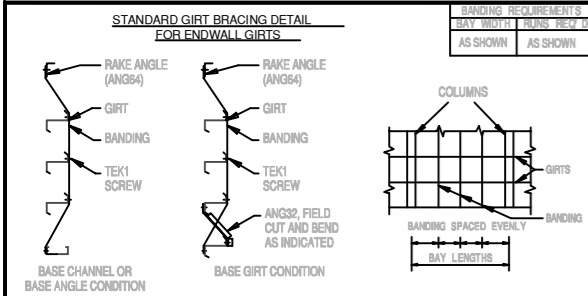
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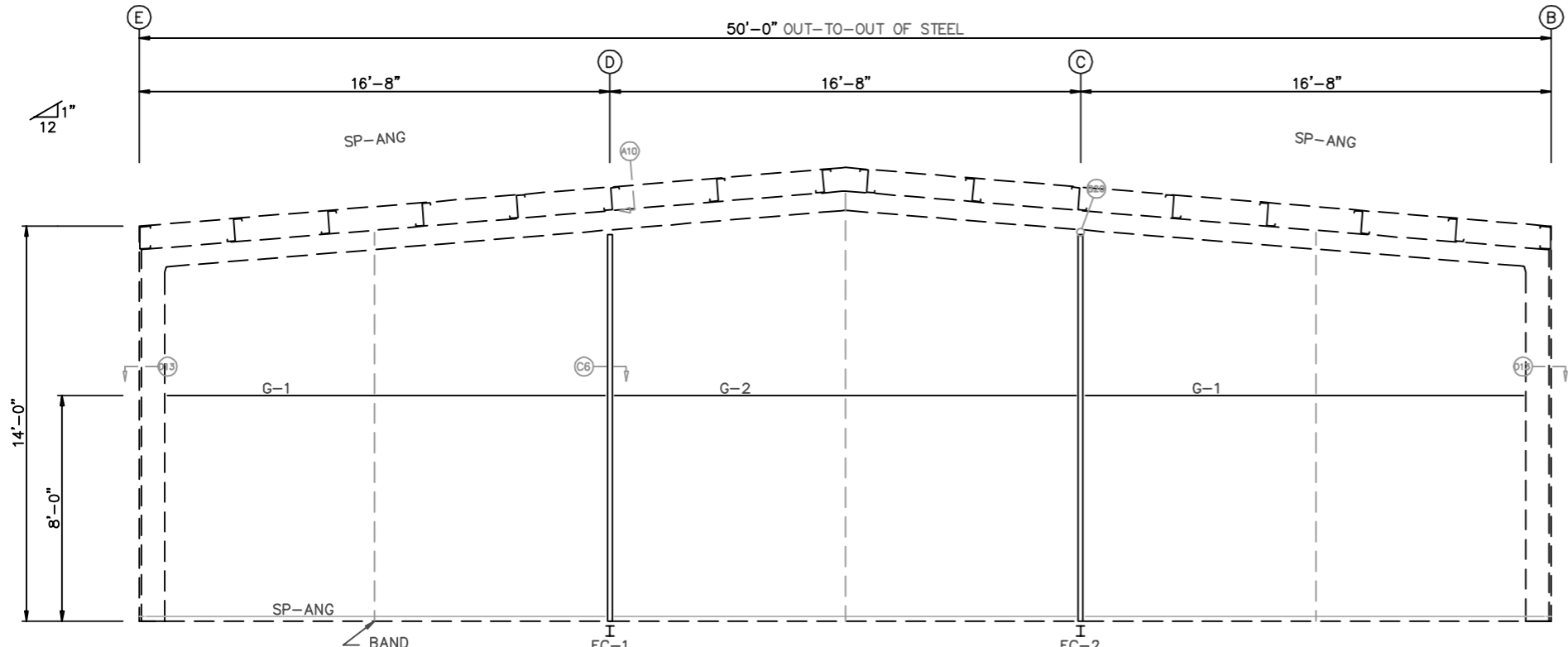
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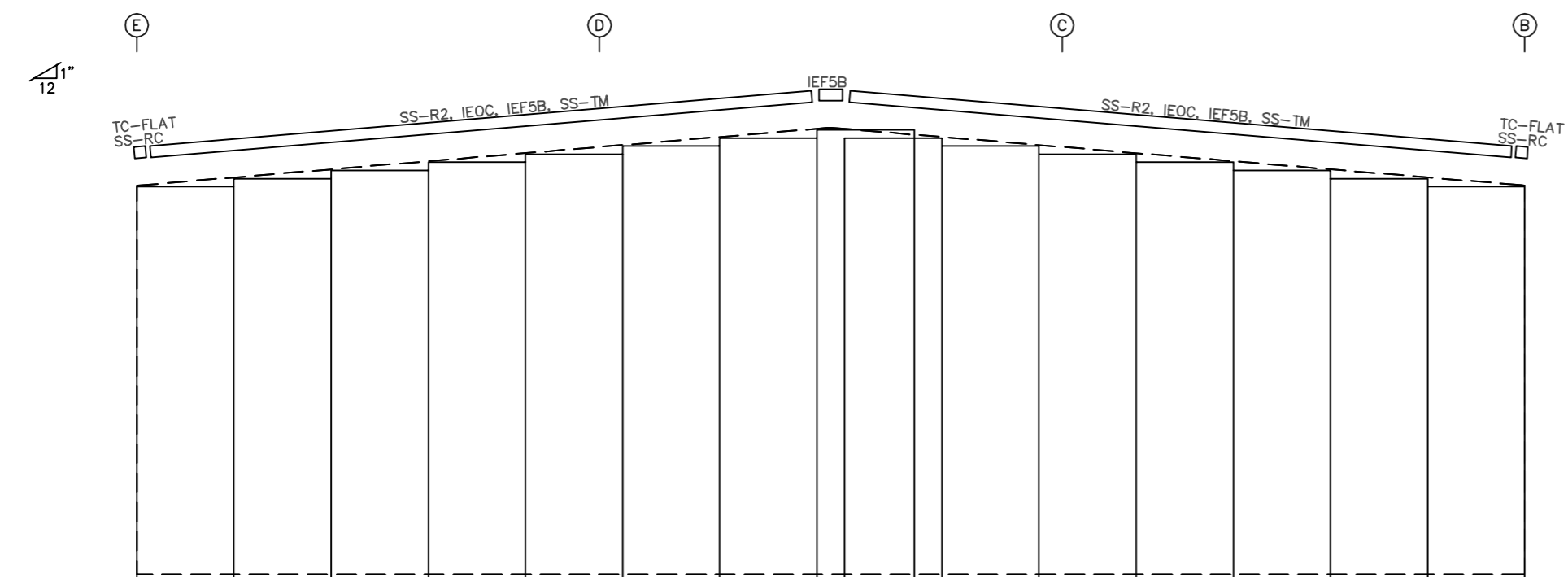


MEMBER TABLE
FRAME LINE 1

QUAN	MARK	PART	LENGTH
2	G-1	8X35C13	15'-6"
1	G-2	8X35C13	16'-2 1/4"



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1
PANELS: 3" CF MESA 26Ga EXTERIOR & 26Ga INTERIOR -
EXT: TBD & INT: TBD

NOTE(S):
1.) WALL PANEL MUST BE FASTENED TO SUPPORT WITH MBCI FASTENING PATTERN FP2.

TRIM COLORS

EAVE TRIM = TBD	CORNER TRIM = TBD
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RAKE TRIM = TBD	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	

* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. See detail C7A for field coping of coldform endwall column flange braces.
3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

ALLIED

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STATE OF NEW HAMPSHIRE

TRAVIS V. MICHAUD
No. 15530

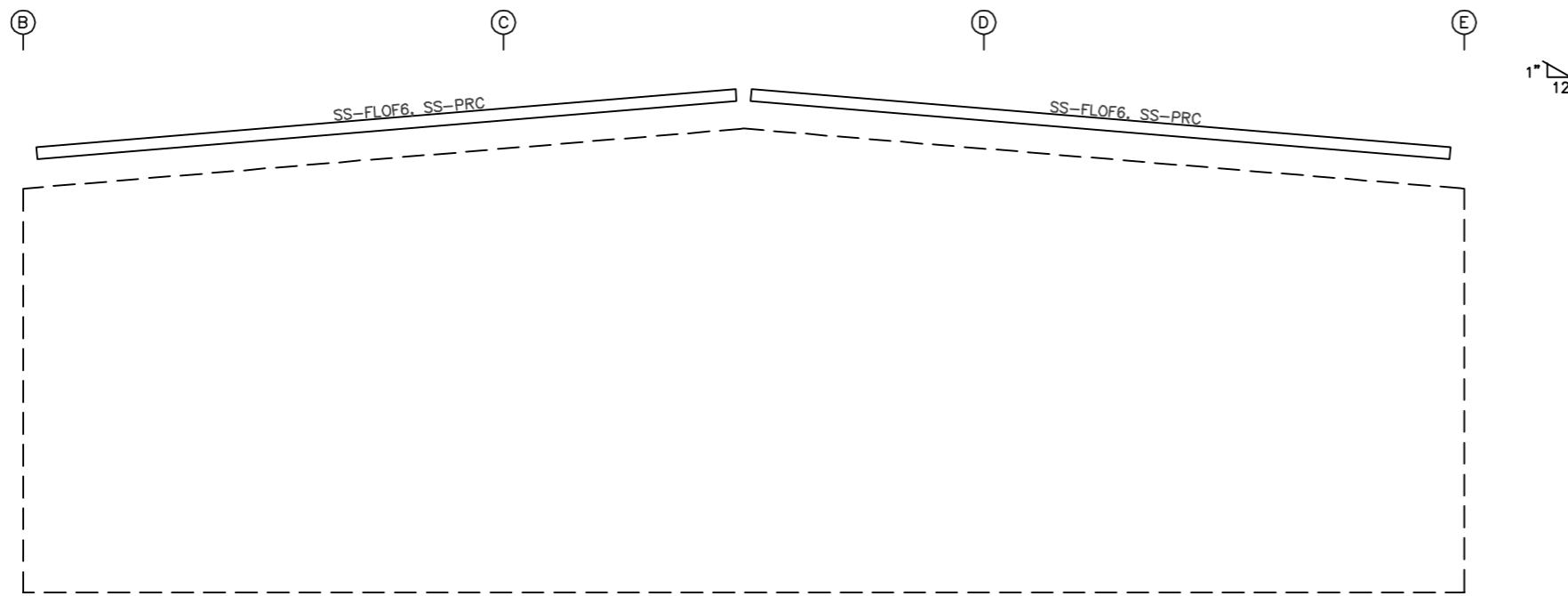
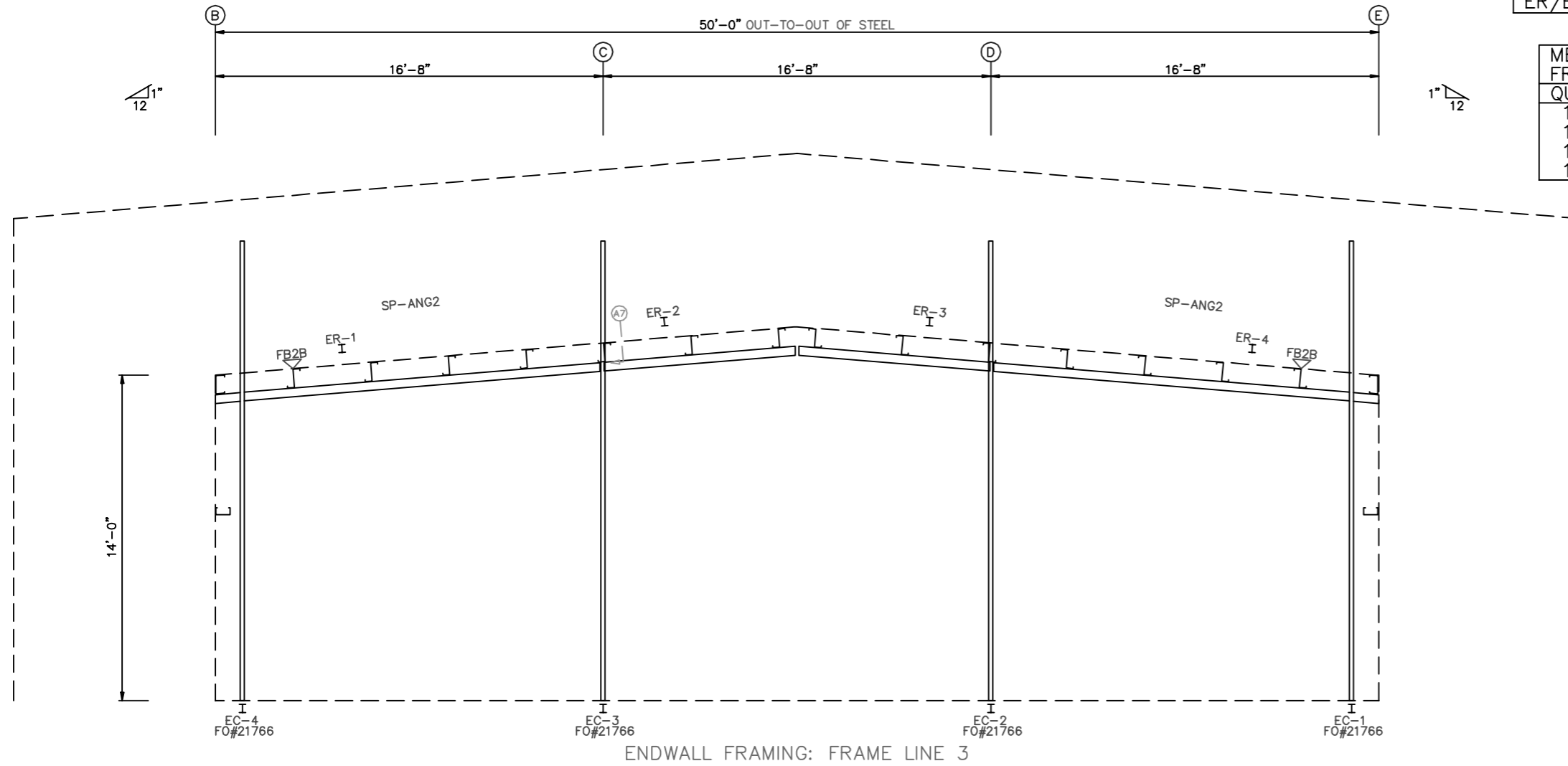
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5/14/18

BOLT TABLE FRAME LINE 3				
LOCATION	QUAN	TYPE	DIA	LENGTH
ER-2/ER-3	8	A325	3/4"	2"
ER/EC	2	A325	1"	2"

MEMBER TABLE FRAME LINE 3			
QUAN	MARK	PART	LENGTH
1	ER-1	W12X16	16'-8 9/16"
1	ER-2	W12X16	8'-5 1/4"
1	ER-3	W12X16	8'-5 1/4"
1	ER-4	W12X16	16'-8 9/16"

FLANGE BRACE TABLE FRAME LINE 3		
VID	MARK	LENGTH
1	FB2B	1'-6"



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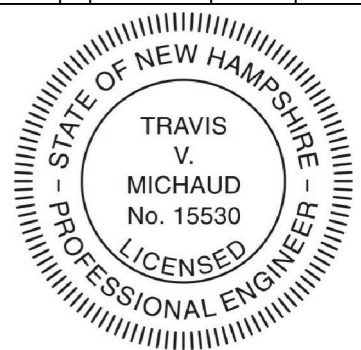
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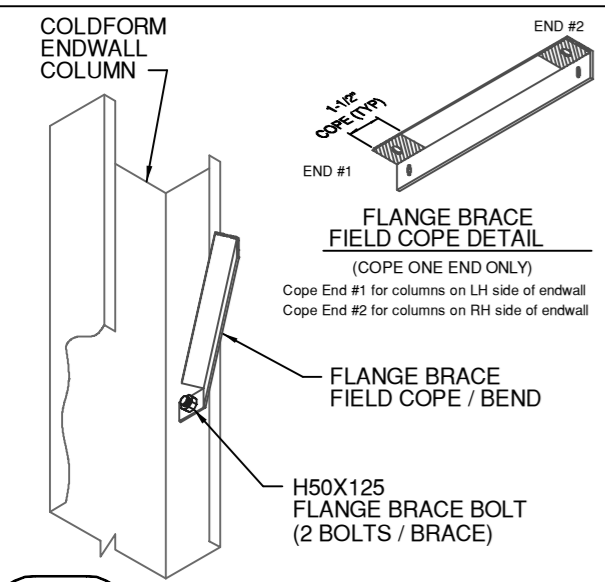


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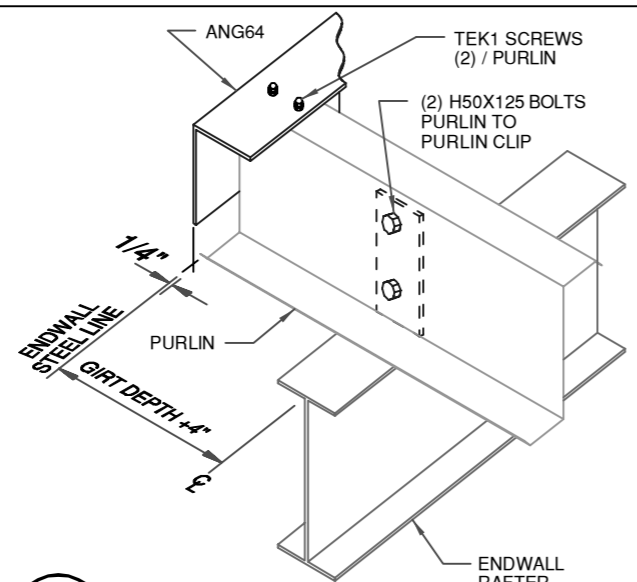
TRIM COLORS	
EAVE TRIM = TBD	CORNER TRIM = TBD
BASE TRIM = TBD	GUTTER = TBD
DOOR TRIM = TBD	DOWNSPOUTS = TBD
RAKE TRIM = TBD	
* LINER TRIM = Liner panel color	
* SOFFIT TRIM = Soffit panel color	
* ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER.	

GENERAL NOTES:

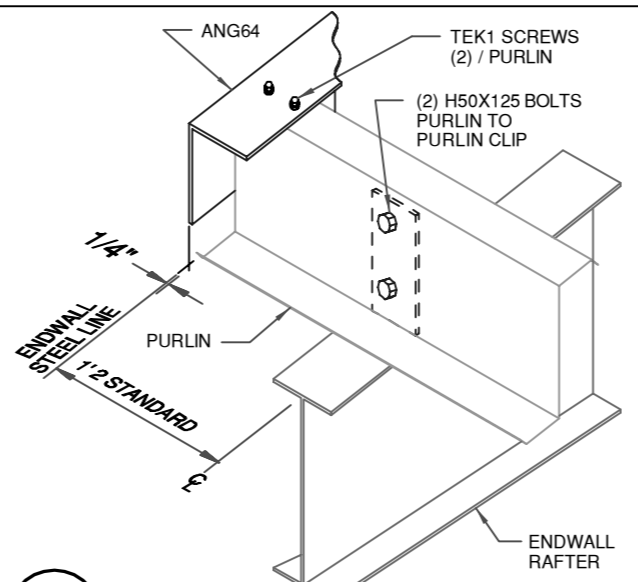
1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. See detail C7A for field coping of coldform endwall column flange braces.
3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).



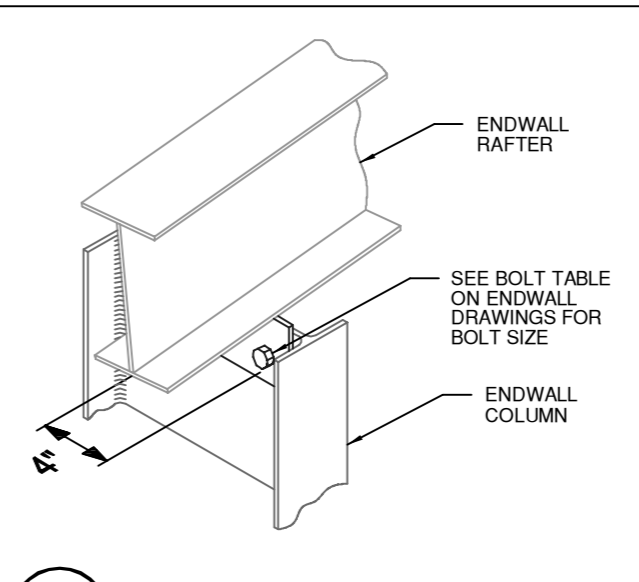
C7A FLANGE BRACE TO BYPASS COLDFORM ENDWALL COLUMN



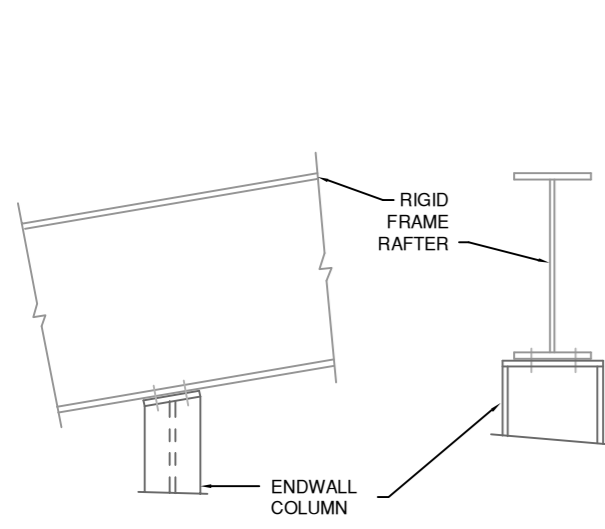
A7 PURLIN TO ENDWALL RAFTER CONNECTION



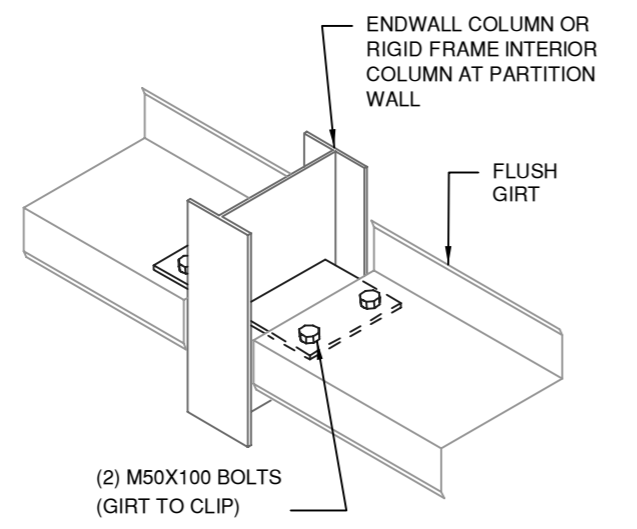
A10 PURLIN TO RIGID FRAME ENDWALL RAFTER CONNECTION



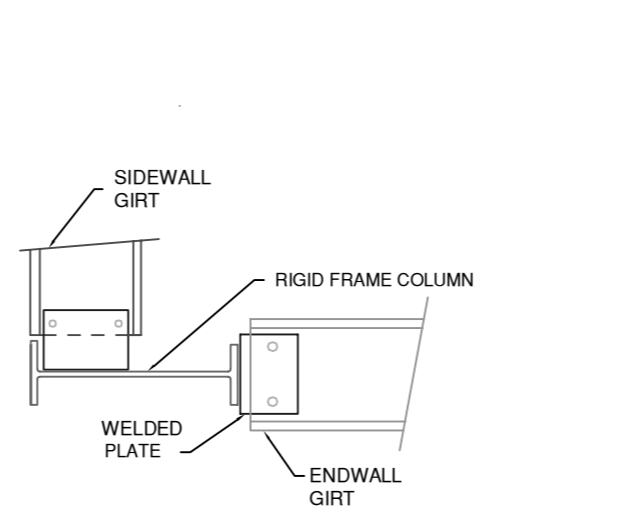
B6 ENDWALL COLUMN TO RAFTER CONNECTION



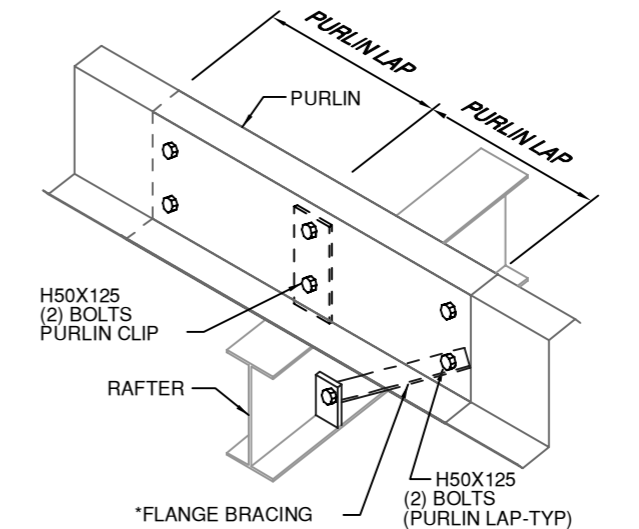
B20 COLUMN TO RIGID FRAME RAFTER



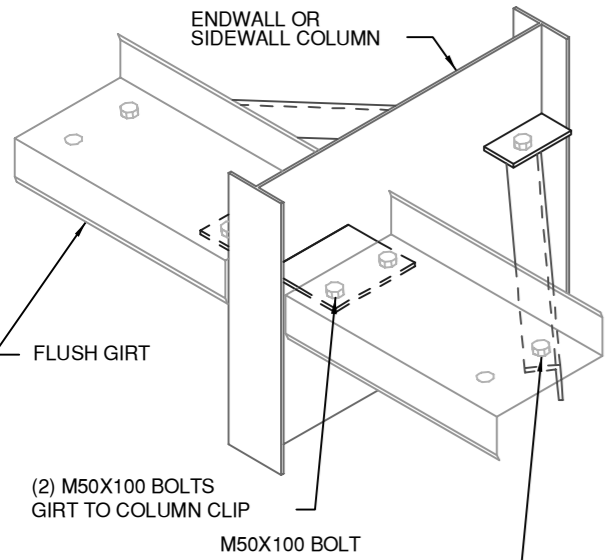
C6 FLUSH GIRT TO COLUMN



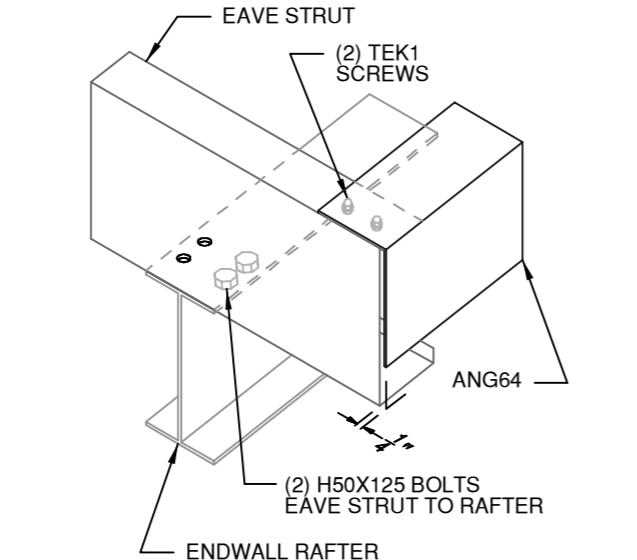
D13 CORNER COLUMN TO WALL GIRT



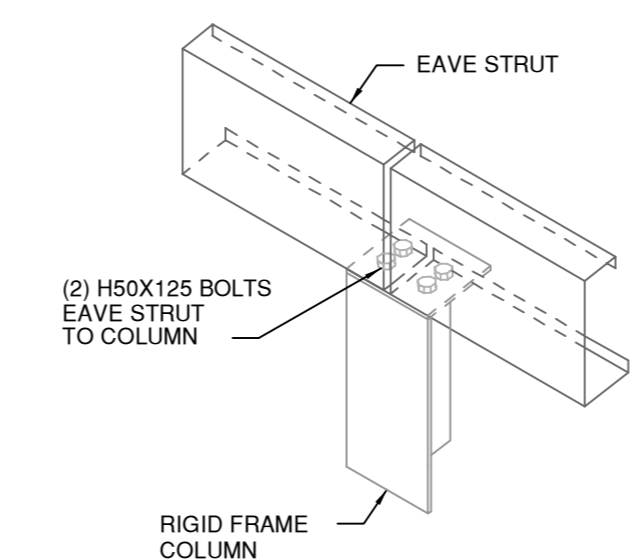
G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



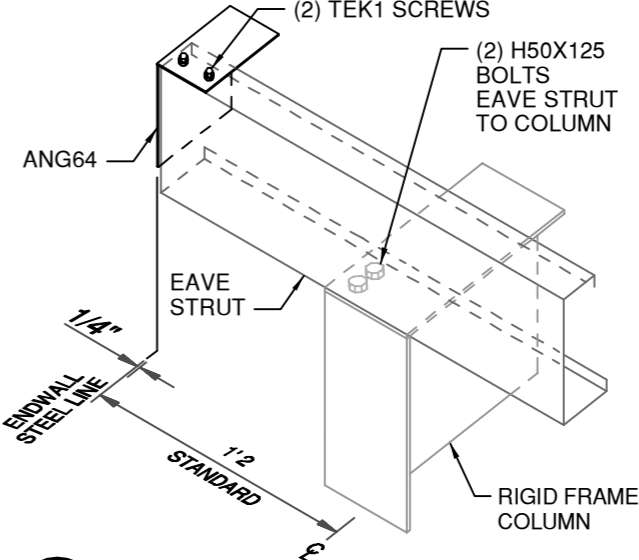
H6 GIRT TO COLUMN - FLUSH GIRTS



I8 EAVE STRUT TO ENDWALL RAFTER LOW EAVE



J1 EAVE STRUT TO RIGID FRAME FLUSH GIRT CONDITION



J23 EAVE STRUT TO RIGID FRAME IN ENDWALL FLUSH GIRT CONDITION



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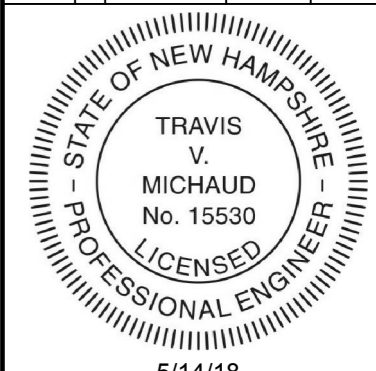
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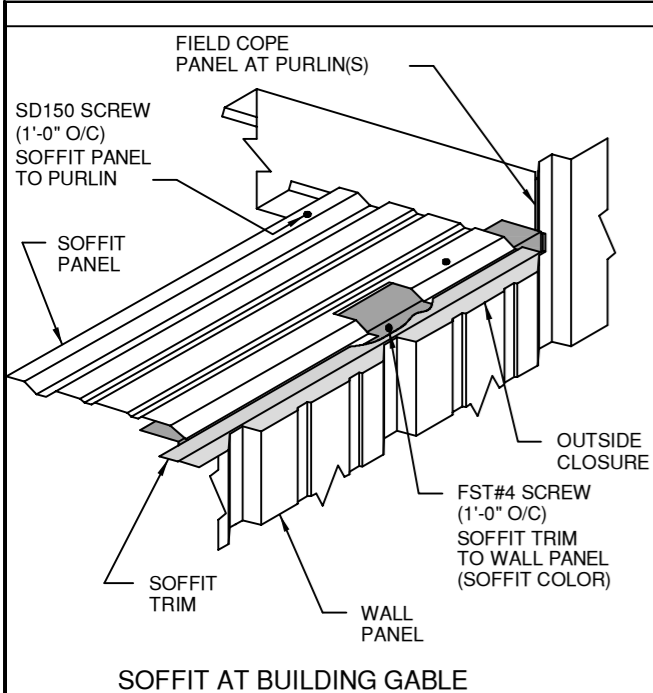
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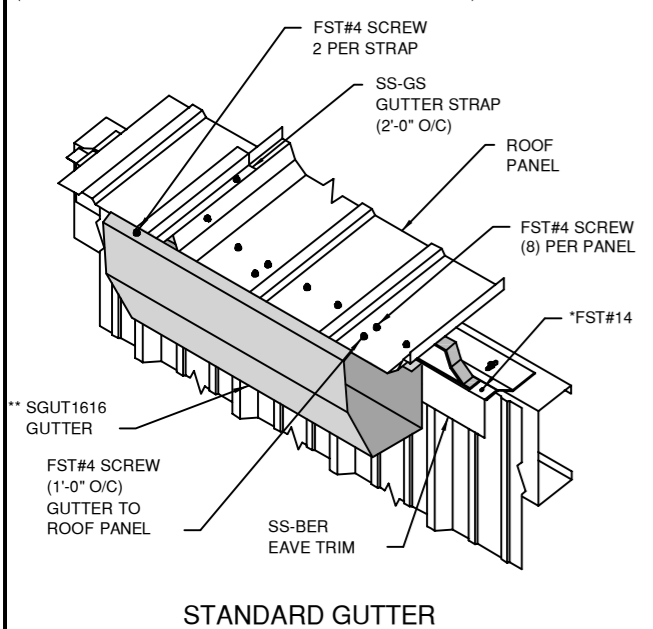
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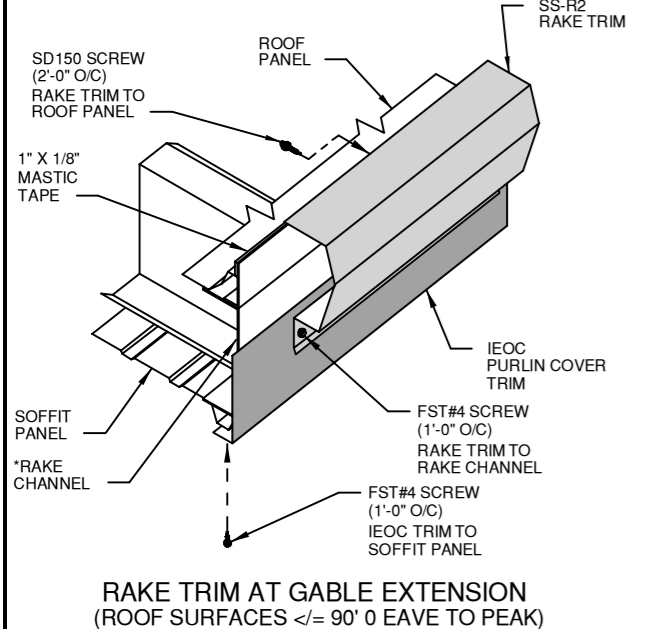
SOFFIT AT BUILDING GABLE

* 3 PER TRIM - TRIM TO EAVE PLATE - INSTALL MASTIC OVER FST#14. (FST#14 HOLDS TRIM UNTIL ROOF PANEL SCREWS ARE INSTALLED).

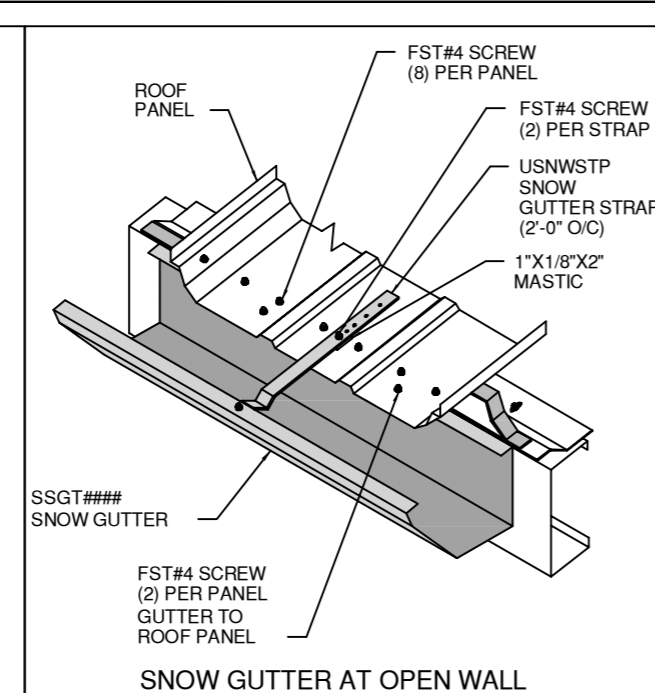


STANDARD GUTTER

* ATTACH RAKE CHANNEL TO PURLINS WITH (2) TEK1 SCREWS AT BOTH THE TOP AND BOTTOM LEGS.

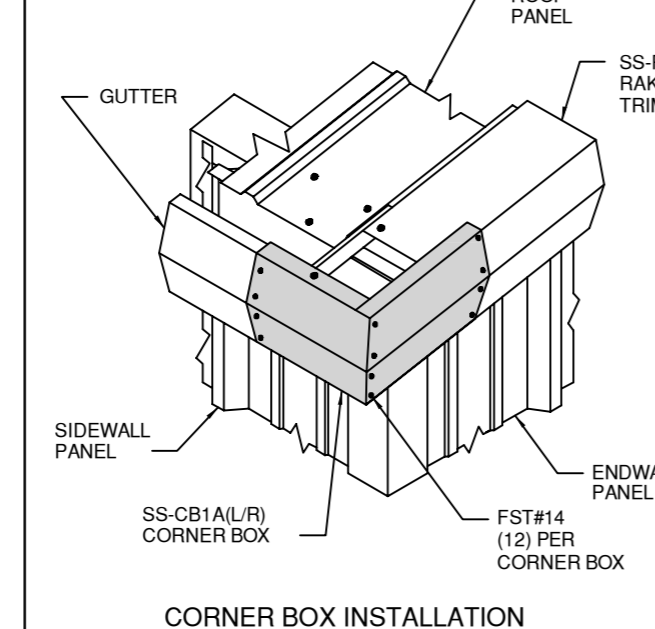


RAKE TRIM AT GABLE EXTENSION (ROOF SURFACES $\leq 90^\circ$ EAVE TO PEAK)



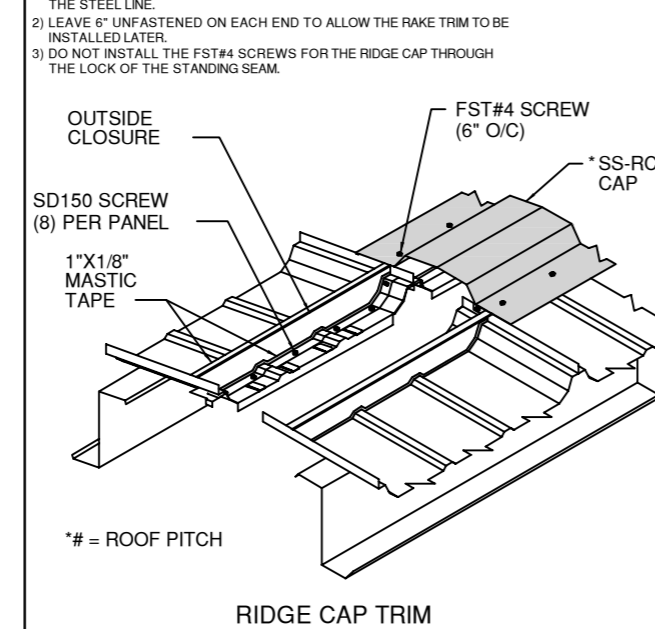
SNOW GUTTER AT OPEN WALL

(FIELD ASSEMBLY OF CORNER BOX REQUIRED)

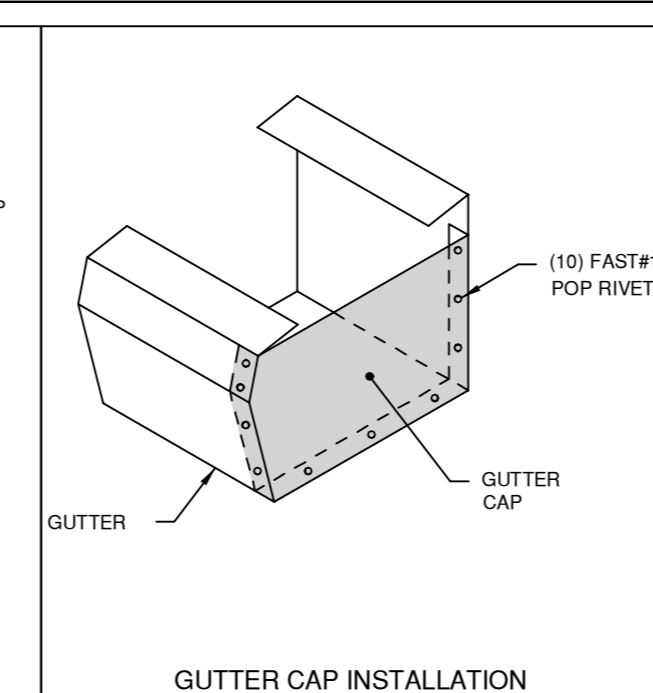


CORNER BOX INSTALLATION

NOTE(S):
 1) INSTALL THE RIDGE FLASHING STARTING AND ENDING 2-1/2" OUTSIDE THE STEEL LINE.
 2) LEAVE 6" UNFASTENED ON EACH END TO ALLOW THE RAKE TRIM TO BE INSTALLED LATER.
 3) DO NOT INSTALL THE FST#4 SCREWS FOR THE RIDGE CAP THROUGH THE LOCK OF THE STANDING SEAM.

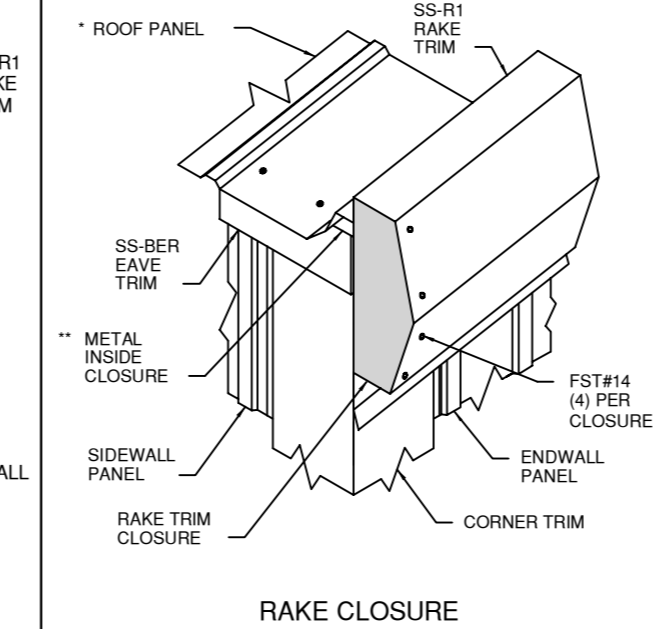


RIDGE CAP TRIM



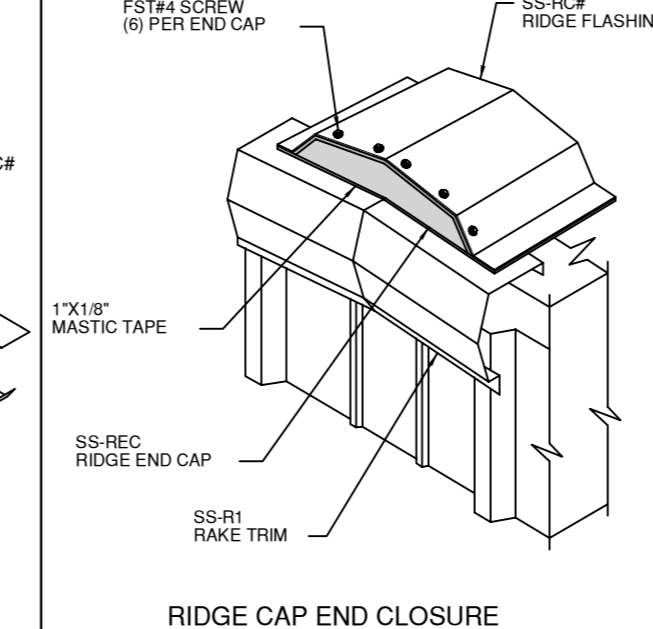
GUTTER CAP INSTALLATION

* CAN BE SEAM-LOK OR VERTICAL-LOK
 ** SEAM-LOK ROOF ONLY

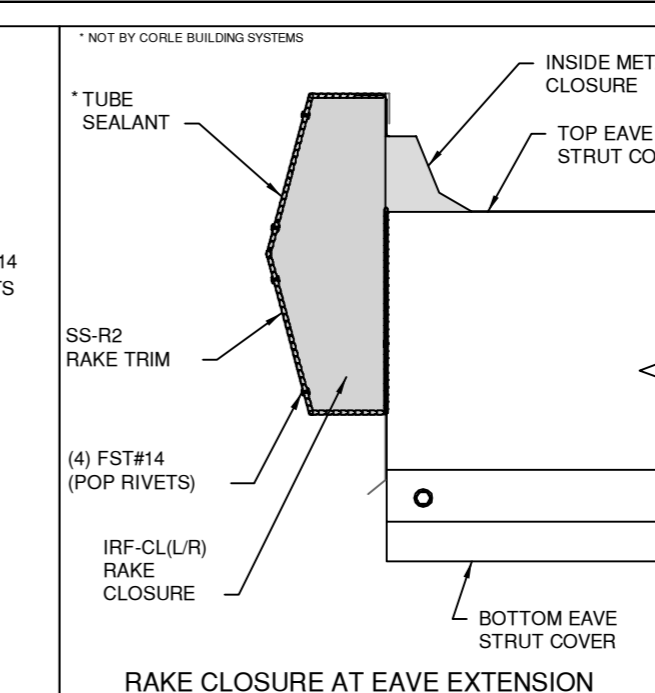


RAKE CLOSURE

* ATTACH EAVE TRIM TO EAVE PLATE WITH (3) FST#14/PIECE UNTIL ROOF PANEL TO EAVE PLATE SCREWS ARE INSTALLED.

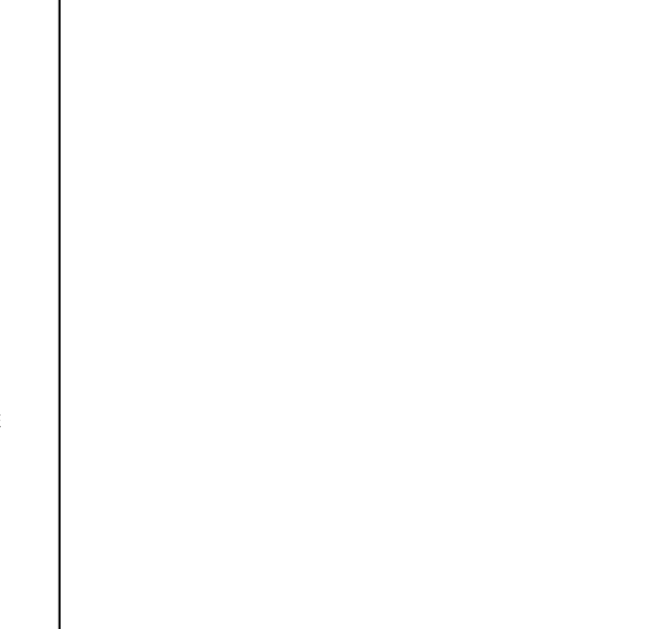


RIDGE CAP END CLOSURE



RAKE CLOSURE AT EAVE EXTENSION

* NOT BY CORLE BUILDING SYSTEMS



GABLE OR SINGLE SLOPE-LOW EAVE OPEN WALL OR MASONRY CONDITION



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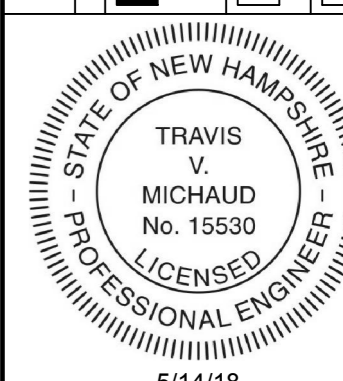
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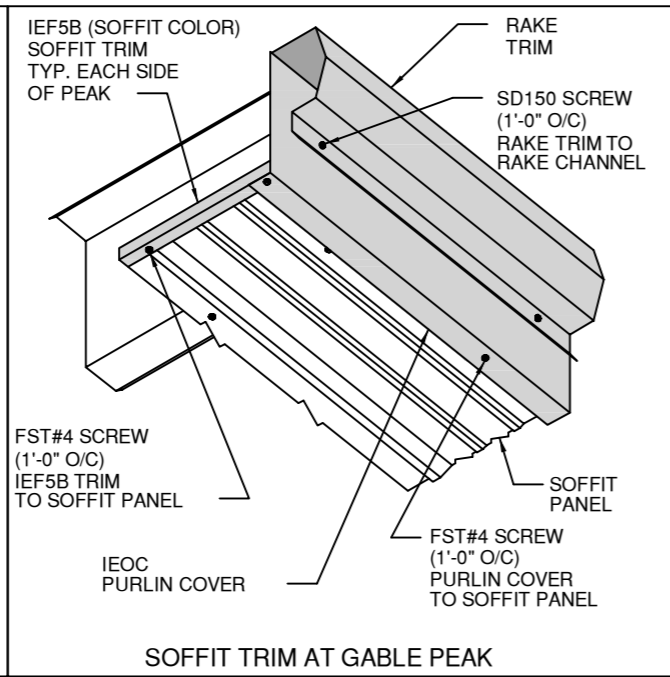
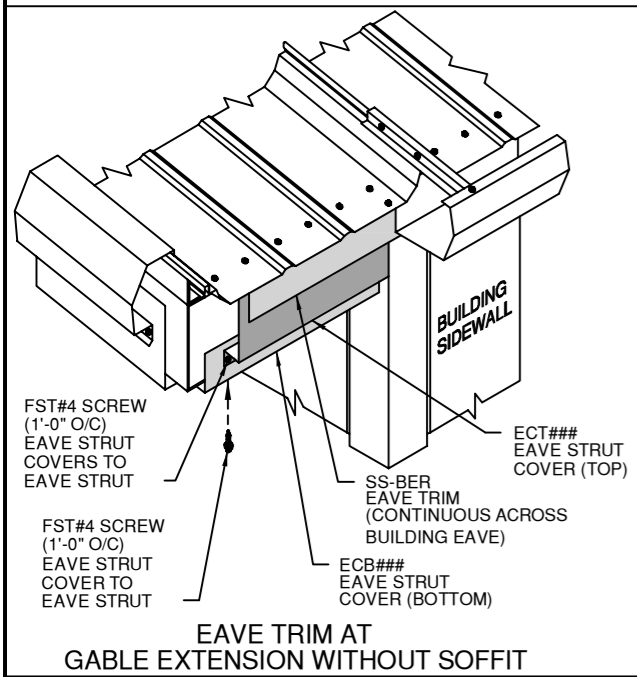
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EAVE TRIM AT GABLE EXTENSION WITHOUT SOFFIT

SOFFIT TRIM AT GABLE PEAK



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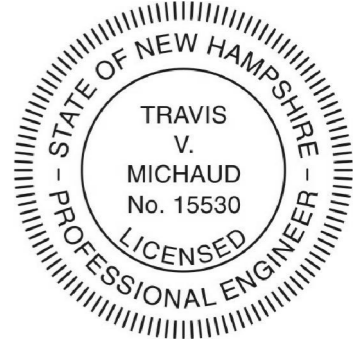
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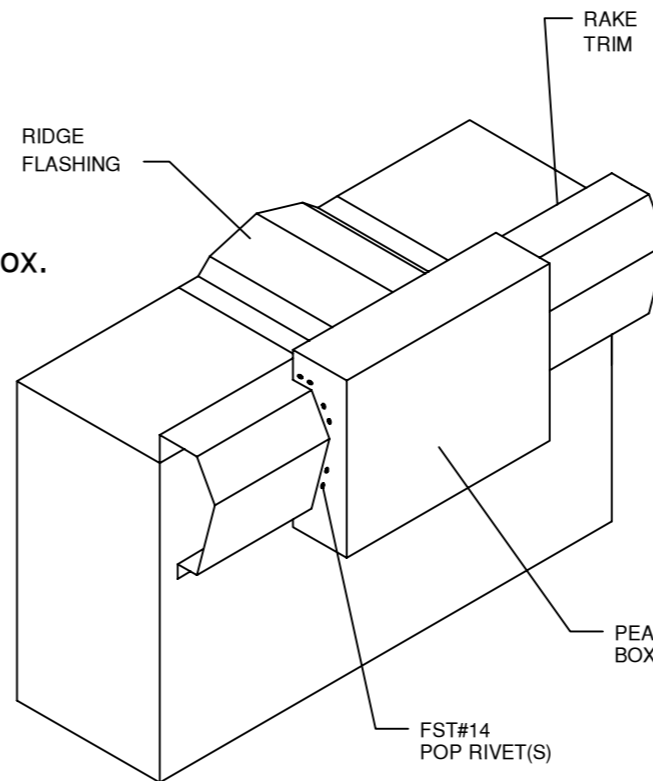
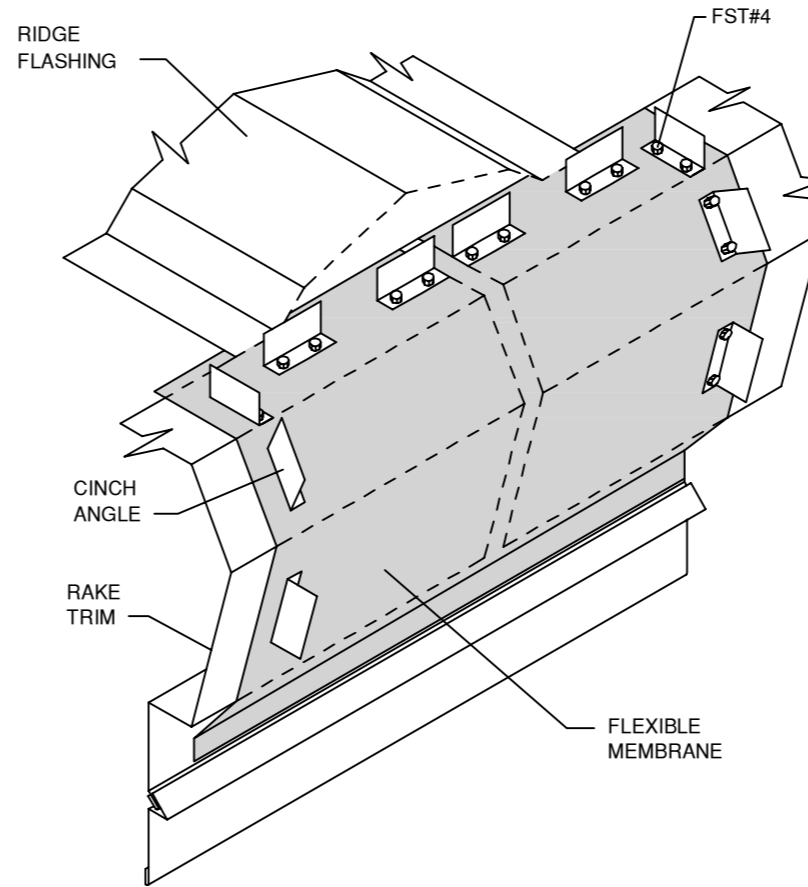
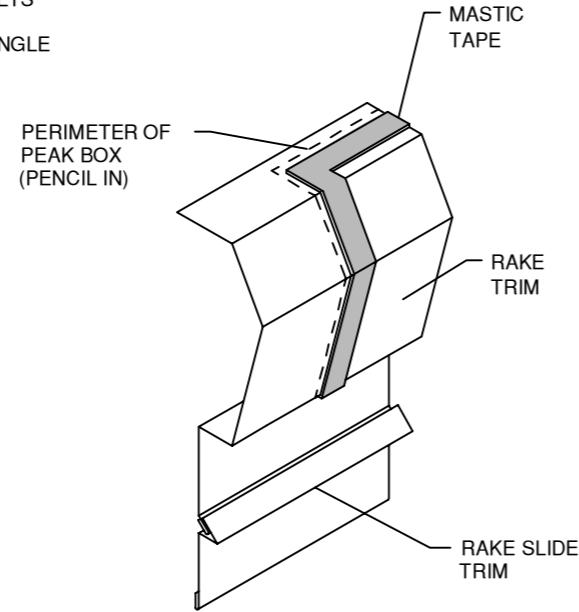
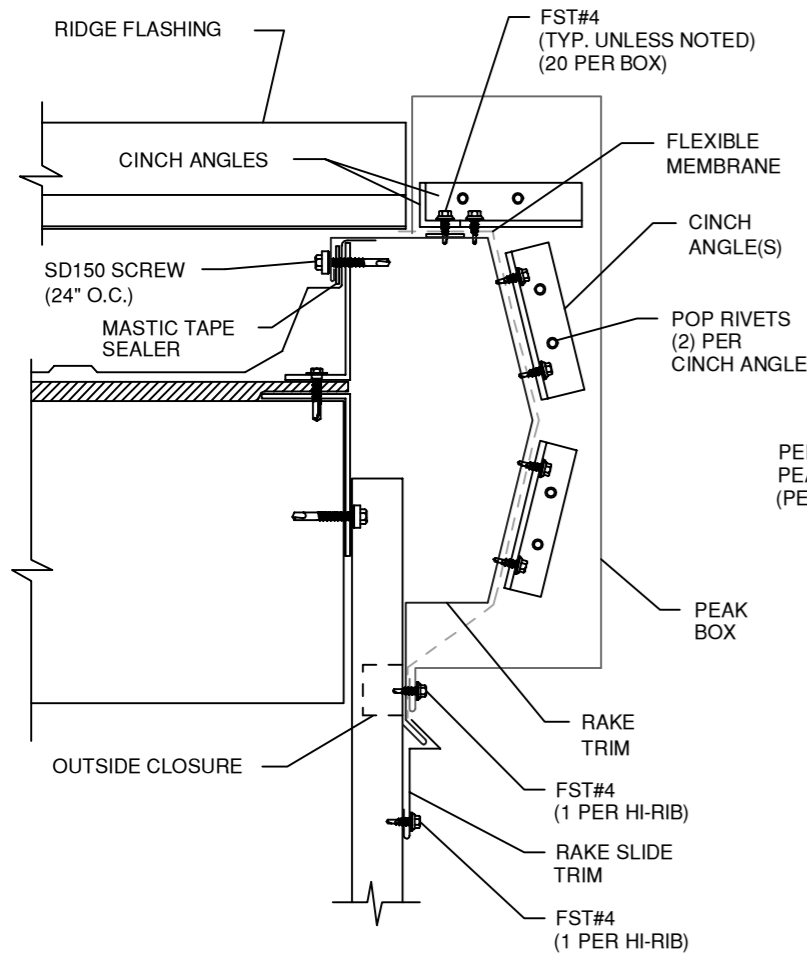
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TRIM DETAILS PEAK BOX INSTALLATION SEAM-LOK / VERTICAL-LOK



PEAK BOX INSTALLATION

1. Install rake trim on each side of ridge to within 1" of centerline of building.
2. Install ridge flashing so that it is on the top leg of rake trim, 2" back from outside edge.
3. Temporarily set peak box in place and mark perimeter of box on rake trim. Remove peak box.
4. Just inside mark, install tape sealer continuously across the top of rake trim, then down the face of the rake trim on both sides of ridge.
5. Place flexible membrane over tape sealer and hold in place with cinch angles. Cinch angles should be attached with fastener #4. To prevent leaks, flexible membrane should be tight against rake trim with no wrinkles at the sealed edges.
6. Hook top of peak box over cinch angles installed on top of rake trim and attach bottom of peak box to endwall with fastener #4.
7. Install FST#14 (pop rivets). (2) per cinch angle.
8. Seal the rake and ridge cap intersections with urethane sealant.



ALLIED

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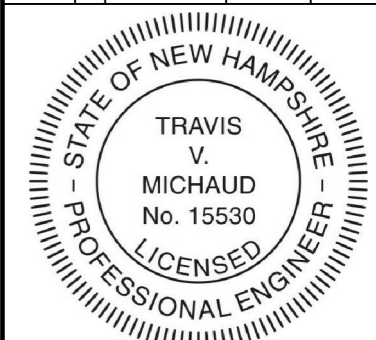
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