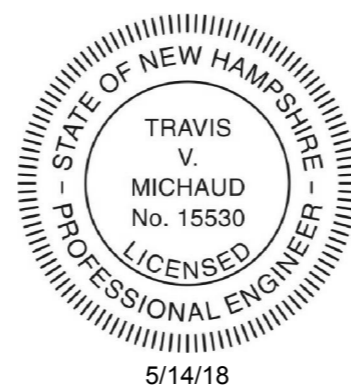


400112 KUA OFFICE

ALLIED STEEL BUILDINGS

FO# 21766
Building 1 of 2



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GENERAL

All materials included in the Metal Building System are in accordance with the manufacturer's standard materials and details unless otherwise specified on the order documents. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 2.1)

DESIGN RESPONSIBILITY

The manufacturer is responsible only for the structural design of the Metal Building System it sells to the purchaser / customer. Neither the manufacturer nor the manufacturer's engineer is the design professional or engineer of record for the construction project. The manufacturer is not responsible for the design of any component or materials not sold by it, or their interface and connection with Metal Building System unless such design responsibility is specifically required by the order documents. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.1)

FOUNDATION DESIGN AND ANCHOR BOLTS

The manufacturer is not responsible for the design, materials, and workmanship of the foundation. The anchor bolt plans prepared by the manufacturer are intended to show only the anchor bolt location, diameter (based on ASTM A36 bolts), and quantity required to connect the Metal Building System to the foundation. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.2). It is the responsibility of the end customer to ensure that adequate provisions are made for specifying bolt embedment, bearing angles, tie rods, and / or associated items embedded in the concrete foundation, as well as foundation design based on the loads imposed by the Metal Building System, or other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.2) U.S. -Anchor bolts shall be accurately set to a tolerance of +/- 1/8 in both elevation and location (AISC Code of Standard Practice for Steel Buildings and Bridges). Canada -Anchor bolts shall be accurately set in accordance with CISC Code of Standard Practice, June 2008, Clause 7.7.1

ADJACENT EXISTING BUILDINGS

The manufacturer does not investigate the influence of the Metal Building System on adjacent existing buildings or structures. The end customer assures that such buildings and structures are adequate to resist snow loads or other conditions as a result of the presence of the Metal Building System. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.2.5)

SHOP-PRIMED STEEL

All structural members of the Metal Building System not fabricated of corrosion resistant material or protected by corrosion resistant coating are painted with one coat of shop primer. All surfaces to receive shop primer are cleaned of loose rust, loose mill scale and other foreign matter by using, as a minimum the hand tool cleaning method SSPC-SP2 (Steel Manual, Structures Painting Council) prior to painting. The coat of shop primer is intended to protect the steel framing for only a short period of exposure to ordinary atmospheric conditions. Shop-primed steel should be placed on blocking to prevent contact with the ground, and so positioned as to minimize water holding pockets, dust, mud and other contamination of the primer film. Repairs of damage to primed surfaces and or removal of foreign material due to improper field storage or site conditions are not the responsibility of the manufacturer. (CISC Code of Standard Practice, June 2008, Clause 6.8; (MBMA 2012 Metal Building Systems Manual, Part IV, Section 4.2.4).

ERECTION-GENERAL

The erector, by entering into contract to erect the building, holds itself out as skilled in the erection of Metal Building Systems and is responsible for complying with all applicable local, federal, and state construction and safety regulations including OSHA regulations as well as any applicable requirements of local, national, or international union rules or practices. (CISC Code of Standard Practice, June 2008, Clause 7.2; (MBMA 2012 Metal Building System Manual, Part IV, Section 6.9).

The erector shall erect the Metal Building System in accordance with the erection drawings, the Erection and Detail Manual (February 2012), and / or the Seam-Lok Technical - Erection manual (May 2012) as furnished by the manufacturer. The aforementioned erection information is intended to illustrate the layout of the framing members, provide the associated connection details, and suggests sequence of erection. It is not intended to specify any particular method of erection to be followed by the erector. The erector remains solely responsible for the safety and appropriateness of all techniques and methods utilized by its crews in the erection of the Metal Building System. The erector is responsible for supplying any safety devices such as scaffolds, runways, nets, et, which may be required to safely erect the Metal Building System. (MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.9) The manufacturer expressly disclaims any responsibility for injury to persons in the course of erection or for damages to the product itself. Field erection of a Pre-Engineered Metal Building, as in all construction projects, involves hazards to persons within the area of the construction and risk of damage to the property itself. Only experienced persons who are skilled and qualified in the erection of Metal Building Systems should be permitted to field-erect a building due to the hazards of this construction activity. The manufacturer is not responsible for the erection of the Metal Building System, the supply of any tools or equipment, or any other field work. The manufacturer provides no field supervision for the erection of the structure nor does the manufacturer perform any intermediate or final inspections of the Metal Building System during or after erection.

The erector shall furnish temporary guys and bracing where needed for squaring, plumbing, and securing the structural framing against loads, such as wind loads acting on the exposed framing as well as loads due to erection equipment and erection operation, but not including loads resulting from the performance of work by others. Bracing furnished by the manufacturer for the Metal Building System cannot be assumed to be adequate during erection. Temporary supports such as temporary guys, braces, false work, cribbing, or other elements required for the erection operation will be determined, erected, and installed by the erector. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 7.10.3; CISC Code of Standard Practices, June, 2008, Clause 1.5; MBMA 2012 Metal Buildings System Manual, Part IV, Section 6.2.1.5).

ERECTION TOLERANCES

U.S. ; Erection tolerances are those set forth in AISC code of standard practice except individual members are considered, plumb, level and aligned if the deviation does not exceed 1:500. (AISC Code of Standard Practice for Steel Buildings and Bridges April 14, 2010 Section 7.13.1; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.8)

Canada; Erection tolerances are those set forth in CISC Code of Standard Practice except individual members are considered plumb, level and aligned if the deviation does not exceed 1:500. (CISC Handbook of Steel Construction, Tenth Edition, Second Revised Printing, Part 1, Clause 29.3; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.8)

BOLT TIGHTENING

The proper tightening and inspection of all fasteners is the responsibility of the erector (Reference RCSC for structural joints using high strength bolts; August 1, 2014). All high strength (ASTM F3125, A325, A490) bolts and nuts must be tightened by the "turn-of-the-nut" method unless otherwise specified by the end customer in the contract documents. Inspection of high strength bolt and nut installation by other than the erector must also be specified in the contract documents and the erector is responsible for ensuring that the installation procedures are compatible prior to the start of erection (CISC Handbook of Steel Construction, Tenth Edition, Second Revised Printing, Part 1, Clause 23.8.2), (MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.9).

Table with 6 columns: MATERIALS, ASTM DESIGNATION, MINIMUM YIELD, MATERIALS, ASTM DESIGNATION, MINIMUM YIELD. Rows include Hot-Rolled Mill Sections, Structural Steel Plates, Structural Steel Bars, Cold Formed Light Gauge Shapes, Cable Bracing, and Rod Bracing.

CORRECTION OF ERRORS AND REPAIRS

The correction of minor misfits by the use of drift pins to draw the components into line, shimming, moderate amounts of reaming, chipping, and cutting, and the replacement of minor shortages of material are a normal part of erection and are not subject to claim. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 7.14; CISC Code of Standard Practice, June 2008, Clause 7.15; MBMA 2012 Metal Building Systems Manual, Part IV, Section 6.10).

DRAWING DISCREPANCIES

In case of discrepancies between the manufacturers steel plans and plans for other trades, the manufacturers steel plans govern. (AISC Code of Standard Practice for Steel Buildings and Bridges, April 14, 2010, Section 3.3; CISC Code of Standard Practice, June 2008, Clause 3.4; MBMA 2012 Metal Building Systems Manual, Part IV, Section 3.1).

DELIVERIES

Delivery of any material by the manufacturers carrier, a common carrier, or to purchasers/ customers own leased, chartered, or authorized conveyance shall constitute delivery to builder, and thereafter, such material shall be at builders risk. If builder chooses to use its own, or private carrier, it shall be solely responsible for compliance with all applicable government regulations. All charges shall be borne by the builder. The manufacturers responsibility for damage or loss ceases upon delivery of shipment to carrier. The manufacturer will endeavor to deliver on the required date. The manufacturers truck is not considered as being late if deliveries are between 8am - 12pm (morning) and 12pm - 5pm (afternoon). However, the manufacturer cannot be held responsible for circumstances beyond our control. For deliveries via the manufacturers truck, the manufacturer will only honor claims that were approved by the customer service department at the time of delivery. For deliveries via contract carriers, it is the responsibility of the customer to file claims with the carrier. The manufacturer cannot assume any liability for the claim.

SHORTAGES

The purchaser /customer should make an inspection upon arrival of all building components. The purchaser/customer must note on the freight bill any missing item(s) and notify the manufacturers customer service department immediately; otherwise, the manufacturer cannot be held responsible for any shortages. If any item is damaged, note on the bill of lading and file a claim with the freight agent. Concealed shortages must be reported to the manufacturers customer service department within the following time frames (date from receipt of first delivery), based on the project shipment size, i.e., number of truck loads used in delivery. 1 to 3 loads...2 weeks 4 loads and over...3 weeks The manufacturers responsibility for shortages expires at the end of these time periods.

FABRICATION ERRORS

The purchaser/customer is responsible for contacting the customer service department to advise the manufacturer of fabrication problems and corresponding cost estimates. The manufacturer will be responsible for providing the builder with verbal approval to proceed with appropriate field corrections. This will be done in a timely manner. IF THE BUILDER PROCEEDS WITH CORRECTIVE WORK WITHOUT THE MANUFACTURERS APPROVAL, HE DOES SO AT HIS OWN RISK. The manufacturer shall not be responsible for any claims where the purchaser/customer has not documented the problem, its correction, and reasonable costs for repair, and submitted this documentation for payment within 30 days of the occurrence.

INVOICE PAYMENT

By acceptance of the materials of services set forth in the invoice, the purchaser/customer agrees to pay the invoice amount within the time period specified on the invoice. AT NO TIME IS IT ACCEPTABLE TO DEDUCT A BACK CHARGE OR SHORTAGE FROM AN INVOICE.

SAFETY PROCEDURES

The manufacturer is committed to manufacturing a quality product that can be erected safely. Although good job site practices and a commitment to safety by the erector are beyond the control of the manufacturer, the manufacturer highly recommends the erector provide good, safe working conditions on the job site. The erector should follow all local, state, and federal health and safety regulations at all times. Accident prevention practices should be implemented and each employee should know emergency procedures. The manufacturer also recommends daily meetings to discuss erection safety procedures. For additional information concerning federal health and safety regulations, contact the occupational safety and health administration (osha).

U.S. Department of Labor Occupational Safety and Health Administration 200 Constitution Avenue, N.W. Washington, DC 20210 www.osha.gov

The manufacturer shall not be responsible for personal injury or property damage as a result of failure to follow all applicable safety regulations and material handling and installation recommendations.



400112 KUA OFFICE

85'-0" x 40'-0" x 20'-0"

DATE: 5/ 8/18

ENG: TMM

DWN: BJC

APPD: TMM

REVISION: 0

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REVISION HISTORY

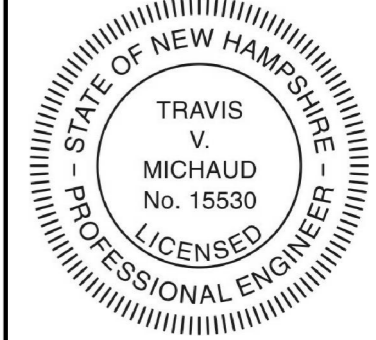
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REV.

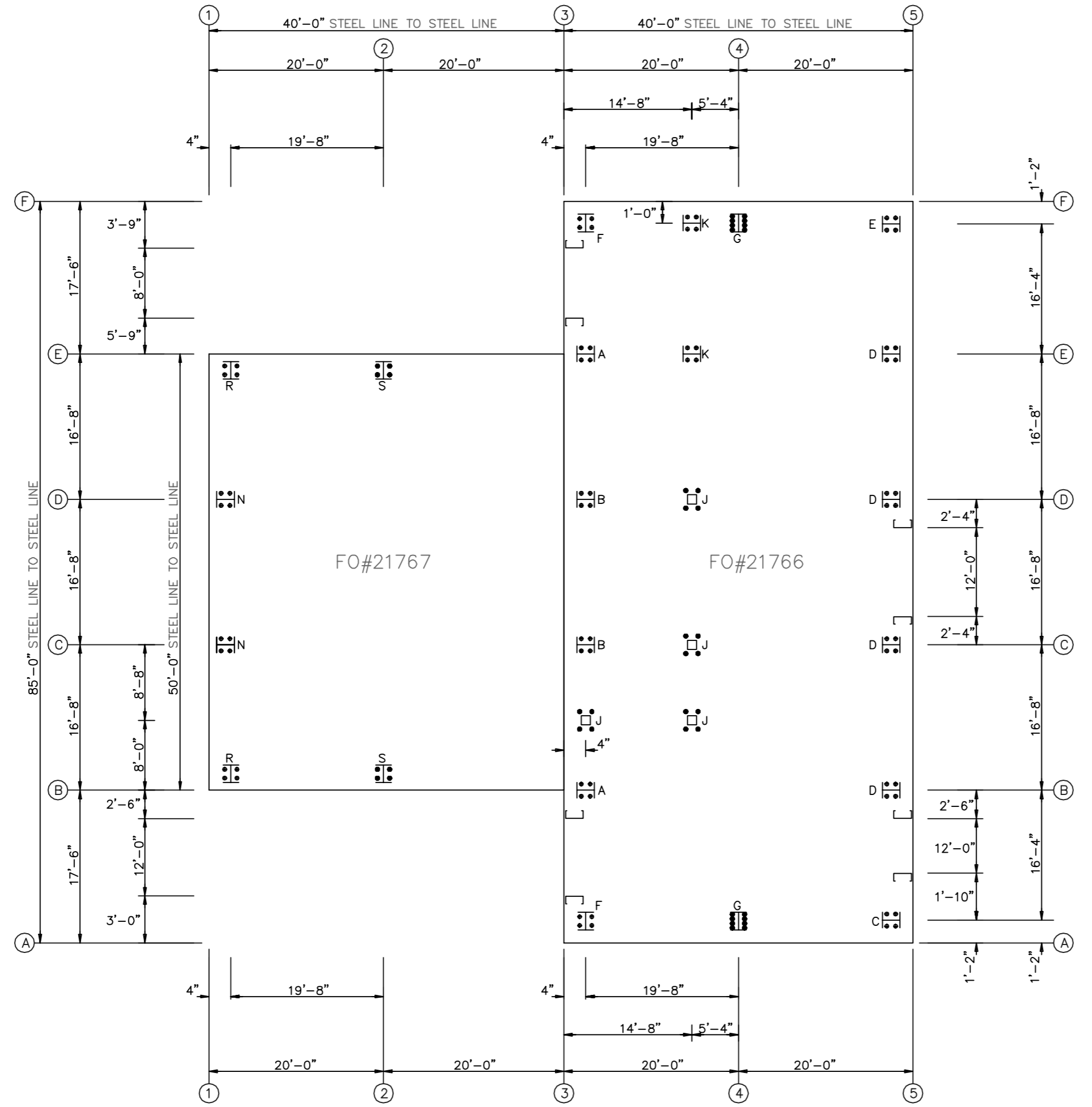
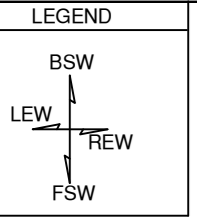
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5/14/18



ANCHOR BOLT PLAN
 NOTE: All Base Plates @ 100'-0" (U.N.)
 Finished Floor @ 100'-0"



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 85'-0" x 40'-0" x 20'-0"
 DATE: 5/8/18 REVISION: 01
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F.O. 21766

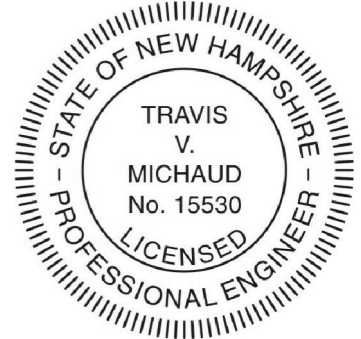
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| REVISION HISTORY | |
|------------------|-----------------------------|
| REV. | DESCRIPTION |
| 01 | UPDATED BUILDING DIMENSIONS |
| | DATE 5-10-18 |

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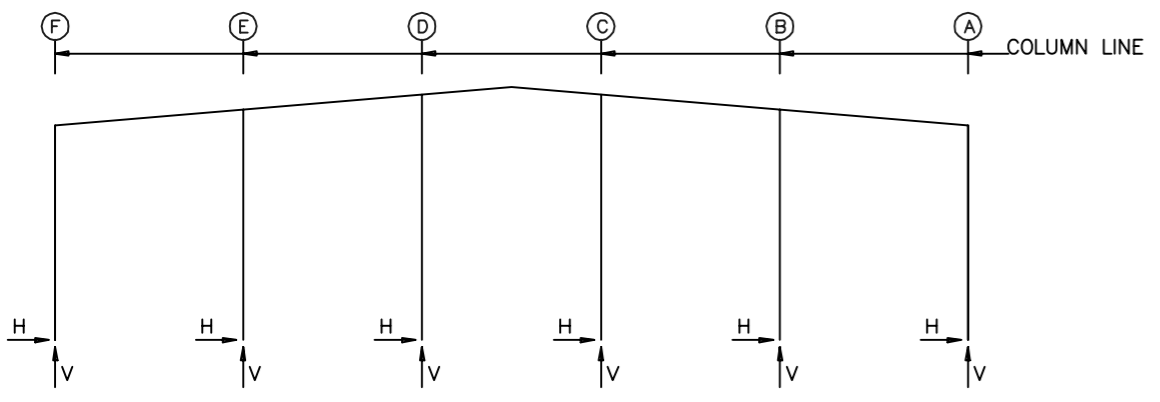
FOR PERMIT:
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FOR CONSTRUCTION:
 FINAL DRAWINGS.



5/14/18

FRAME LINES: 3



RIGID FRAME: BASIC COLUMN REACTIONS (k)

| Frame Line | Column Line | Dead | | Collateral | | Live | | Floor | | Snow | | Snow_Drift | |
|------------|-------------|-------|------|------------|------|-------|------|-------|------|-------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 3 | F | 0.1 | 5.0 | 0.0 | 0.8 | 0.2 | 1.8 | 0.1 | 9.6 | 0.5 | 5.8 | 0.0 | 0.0 |
| 3 | A | -0.1 | 1.0 | 0.0 | 0.5 | -0.2 | 1.8 | -0.1 | 0.1 | -0.5 | 5.8 | 0.0 | 0.0 |
| 3 | E | 0.0 | 9.1 | 0.0 | 2.1 | 0.0 | 5.7 | 0.0 | 16.3 | 0.0 | 17.9 | 0.0 | 3.4 |
| 3 | D | 0.0 | 9.9 | 0.0 | 2.5 | 0.0 | 7.1 | 0.0 | 16.3 | 0.0 | 22.4 | 0.0 | 6.5 |
| 3 | C | 0.0 | 8.2 | 0.0 | 2.3 | 0.0 | 7.2 | 0.0 | 12.5 | 0.0 | 22.4 | 0.0 | 6.5 |
| 3 | B | 0.0 | 2.1 | 0.0 | 1.5 | 0.0 | 5.7 | 0.0 | -0.1 | 0.0 | 17.9 | 0.0 | 3.4 |

| Frame Line | Column Line | Wind_Left1 | | Wind_Right1 | | Wind_Left2 | | Wind_Right2 | | Wind_Long1 | | Wind_Long2 | |
|------------|-------------|------------|------|-------------|------|------------|------|-------------|------|------------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 3 | F | -2.3 | -3.9 | 2.3 | 0.6 | -2.8 | -3.0 | 1.8 | 1.4 | 0.9 | -8.1 | 0.4 | -7.2 |
| 3 | A | -2.4 | 0.6 | 2.3 | -3.9 | -1.8 | 1.5 | 2.8 | -3.0 | -0.9 | -8.1 | -0.4 | -7.2 |
| 3 | E | 0.0 | -3.2 | 0.0 | -6.4 | 0.0 | -1.9 | 0.0 | -5.2 | 0.0 | -4.6 | 0.0 | -3.7 |
| 3 | D | 0.0 | -6.6 | 0.0 | -5.2 | 0.0 | -5.4 | 0.0 | -4.0 | 0.0 | -5.7 | 0.0 | -3.3 |
| 3 | C | 0.0 | -5.2 | 0.0 | -6.6 | 0.0 | -4.0 | 0.0 | -5.4 | 0.0 | -5.7 | 0.0 | -3.3 |
| 3 | B | 0.0 | -6.4 | 0.0 | -3.2 | 0.0 | -5.2 | 0.0 | -1.9 | 0.0 | -4.6 | 0.0 | -3.7 |

| Frame Line | Column Line | Seismic_Left | | Seismic_Right | | Seismic_Long | | LWIND1_L2E | | LWIND1_R2E | | LWIND2_L2E | |
|------------|-------------|--------------|------|---------------|------|--------------|------|------------|------|------------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 3 | F | -4.9 | -4.3 | 4.9 | 4.3 | 0.0 | -9.3 | 0.0 | -0.5 | 0.0 | -0.1 | 0.0 | -0.5 |
| 3 | A | -3.9 | 5.3 | 3.9 | -5.3 | 0.0 | -7.5 | 0.0 | -0.1 | 0.0 | -0.5 | 0.0 | -0.1 |
| 3 | E | 0.0 | 5.4 | 0.0 | -5.4 | 0.0 | 0.0 | 0.0 | -0.2 | 0.0 | 0.1 | 0.0 | -0.2 |
| 3 | D | 0.0 | -1.6 | 0.0 | 1.6 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.0 | 0.1 |
| 3 | C | 0.0 | 1.4 | 0.0 | -1.4 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 3 | B | 0.0 | -6.1 | 0.0 | 6.1 | 0.0 | 0.0 | 0.0 | 0.1 | 0.0 | -0.2 | 0.0 | 0.1 |

| Frame Line | Column Line | LWIND2_R2E | | F1UNB_SL_L | | F1UNB_SL_R | |
|------------|-------------|------------|------|------------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 3 | F | 0.0 | -0.1 | 0.3 | 5.3 | 0.3 | 2.0 |
| 3 | A | 0.0 | -0.5 | -0.4 | 2.1 | -0.3 | 5.3 |
| 3 | E | 0.0 | 0.1 | 0.0 | 16.1 | 0.0 | 3.1 |
| 3 | D | 0.0 | 0.0 | 0.0 | 15.0 | 0.0 | 4.4 |
| 3 | C | 0.0 | 0.0 | 0.0 | 4.5 | 0.0 | 15.0 |
| 3 | B | 0.0 | -0.2 | 0.0 | 3.1 | 0.0 | 16.2 |

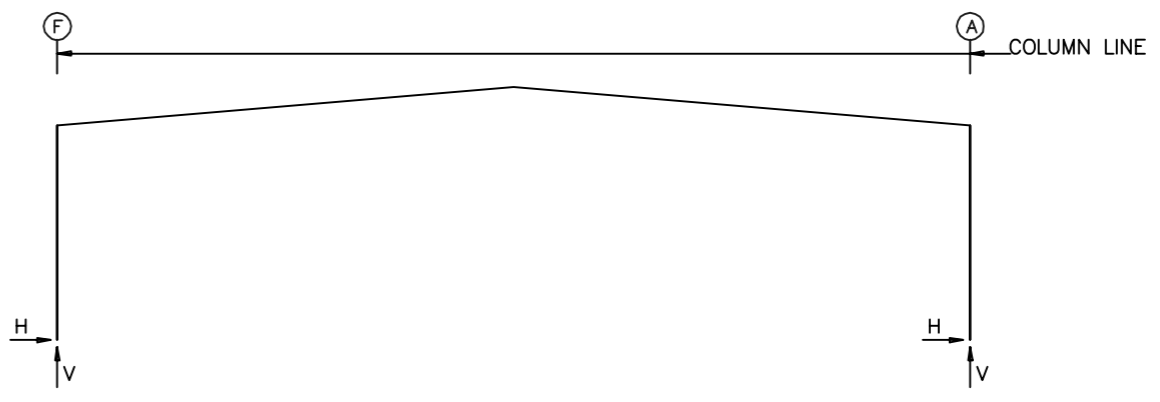
| Frame Line | Column Line | Dead | | Collateral | | Live | | Snow | | Wind_Left1 | | Wind_Right1 | |
|------------|-------------|-------|------|------------|------|-------|------|-------|------|------------|-------|-------------|-------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 4 | F | 6.3 | 9.2 | 4.3 | 5.3 | 17.1 | 21.2 | 53.8 | 66.9 | -14.3 | -17.1 | -8.1 | -12.4 |
| 4 | A | -6.3 | 9.2 | -4.3 | 5.3 | -17.1 | 21.3 | -53.8 | 66.9 | 8.1 | -12.4 | 14.3 | -17.1 |

| Frame Line | Column Line | Wind_Left2 | | Wind_Right2 | | Wind_Long1 | | Wind_Long2 | | Seismic_Left | | Seismic_Right | |
|------------|-------------|------------|------|-------------|------|------------|-------|------------|-------|--------------|------|---------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 4 | F | -10.0 | -9.6 | -3.8 | -4.9 | -11.4 | -24.2 | -7.2 | -16.7 | -2.9 | -1.2 | 2.9 | 1.2 |
| 4 | A | 3.8 | -4.9 | 10.0 | -9.6 | 11.4 | -24.2 | 7.2 | -16.7 | -2.9 | 1.2 | 2.9 | -1.2 |

| Frame Line | Column Line | Seismic_Long | | LWIND1_L2E | | LWIND1_R2E | | LWIND2_L2E | | LWIND2_R2E | | F2UNB_SL_L | |
|------------|-------------|--------------|------|------------|------|------------|------|------------|------|------------|------|------------|------|
| | | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert | Horiz | Vert |
| 4 | F | 0.0 | -9.3 | 0.0 | -1.4 | -0.2 | -0.1 | 0.0 | -1.4 | -0.2 | -0.1 | 44.0 | 67.4 |
| 4 | A | 0.0 | -7.5 | 0.2 | -0.1 | 0.0 | -1.4 | 0.2 | -0.1 | 0.0 | -1.4 | -44.3 | 37.4 |

| Frame Line | Column Line | F2UNB_SL_R | |
|------------|-------------|------------|------|
| | | Horiz | Vert |
| 4 | F | 44.3 | 37.4 |
| 4 | A | -44.0 | 67.4 |

FRAME LINES: 4



RIGID FRAME: ANCHOR BOLTS & BASE PLATES

| Frm Line | Col Line | Anc. Bolt Qty | Bolt Dia | Base Plate (in) | | | Grout (in) |
|----------|----------|---------------|----------|-----------------|--------|-------|------------|
| | | | | Width | Length | Thick | |
| 3 | F | 4 | 0.750 | 8.000 | 11.50 | 0.500 | 0.0 |
| 3 | A | 4 | 0.750 | 8.000 | 11.50 | 0.500 | 0.0 |
| 3 | E | 4 | 0.750 | 8.000 | 8.140 | 0.500 | 0.0 |
| 3 | D | 4 | 0.750 | 8.000 | 8.060 | 0.500 | 0.0 |
| 3 | C | 4 | 0.750 | 8.000 | 8.060 | 0.500 | 0.0 |
| 3 | B | 4 | 0.750 | 8.000 | 8.140 | 0.500 | 0.0 |

RIGID FRAME: ANCHOR BOLTS & BASE PLATES

| Frm Line | Col Line | Anc. Bolt Qty | Bolt Dia | Base Plate (in) | | | Grout (in) |
|----------|----------|---------------|----------|-----------------|--------|-------|------------|
| | | | | Width | Length | Thick | |
| 4 | F | 8 | 1.000 | 8.000 | 18.88 | 1.000 | 0.0 |
| 4 | A | 8 | 1.000 | 8.000 | 18.88 | 1.000 | 0.0 |

FLOOR COLUMN REACTIONS

| Frame Line | Col Line | Dead Vert (k) | Coll Vert (k) | Live Vert (k) | Anc. Bolt Qty | Bolt Dia | Base Width | Base Length | Base Thick | Grout (in) |
|------------|----------|---------------|---------------|---------------|---------------|----------|------------|-------------|------------|------------|
| @1.3 | @59.5 | 1.8 | 0.2 | 4.3 | 4 | 1.000 | 10.00 | 10.00 | 1.000 | 0.0 |
| @14.7 | @0.6 | 3.0 | 0.3 | 7.2 | 4 | 0.750 | 8.000 | 8.125 | 0.500 | 0.0 |
| @14.7 | E | 6.2 | 0.6 | 14.5 | 4 | 0.750 | 8.000 | 8.125 | 0.500 | 0.0 |
| @14.7 | D | 6.2 | 0.6 | 14.7 | 4 | 1.000 | 10.00 | 10.00 | 1.000 | 0.0 |
| @14.7 | C | 4.8 | 0.4 | 11.2 | 4 | 1.000 | 10.00 | 10.00 | 1.000 | 0.0 |
| @14.7 | @59.5 | 1.7 | 0.2 | 3.9 | 4 | 1.000 | 10.00 | 10.00 | 1.000 | 0.0 |



400112 KUA OFFICE
85'-0" x 40'-0" x 20'-0"

DATE: 5/8/18 REVISION: 0
ENG: TVM DWN: BJC APPD: TVM

F.O. 21766

400112 KUA OFFICE

REVISION HISTORY

| REV. | DESCRIPTION | DATE |
|------|-------------|------|
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DRAWING STATUS

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FOR CONSTRUCTION: FINAL DRAWINGS.



5/14/18

BASIC COLUMN REACTIONS (k)

| Frm Line | Col Line | Wind Press | Wind Suct |
|----------|----------|------------|-----------|
| 3 | F | -2.9 | 3.2 |
| 3 | D | -3.0 | 3.3 |
| 3 | C | -3.0 | 3.3 |
| 3 | B | -2.9 | 3.2 |

| Frm Line | Col Line | Dead Vert | Collat Vert | Live Vert | Snow Vert | Wind_Left1 Horz | Wind_Left1 Vert | Wind_Right1 Horz | Wind_Right1 Vert | Wind_Left2 Horz | Wind_Left2 Vert | Wind_Right2 Horz | Wind_Right2 Vert |
|----------|----------|-----------|-------------|-----------|-----------|-----------------|-----------------|------------------|------------------|-----------------|-----------------|------------------|------------------|
| 5 | A | 0.9 | 0.4 | 1.7 | 5.3 | 0.0 | -1.8 | 0.0 | -1.2 | 0.0 | -1.8 | 0.0 | -1.2 |
| 5 | B | 1.5 | 0.9 | 3.5 | 10.9 | 1.5 | -5.7 | 0.0 | -0.2 | 1.5 | -5.7 | 0.0 | -0.2 |
| 5 | C | 1.5 | 0.9 | 3.5 | 11.1 | 0.0 | -1.8 | 1.5 | -4.2 | 0.0 | -1.8 | 1.5 | -4.2 |
| 5 | D | 1.5 | 0.9 | 3.5 | 11.1 | 0.0 | -2.2 | 0.0 | -3.7 | 0.0 | -2.2 | 0.0 | -3.7 |
| 5 | E | 1.5 | 0.9 | 3.5 | 10.9 | 0.0 | -2.2 | 0.0 | -3.8 | 0.0 | -2.2 | 0.0 | -3.8 |
| 5 | F | 0.7 | 0.4 | 1.7 | 5.3 | 0.0 | -1.2 | 0.0 | -1.8 | 0.0 | -1.2 | 0.0 | -1.8 |

| Frm Line | Col Line | Wind_Press Horz | Wind_Suct Vert | Wind Long1 Vert | Wind Long2 Vert | Seis_Left Horz | Seis_Right Vert | Seis_Long Horz | Seis_Long Vert | E2UNB_SL_L Horz | E2UNB_SL_L Vert |
|----------|----------|-----------------|----------------|-----------------|-----------------|----------------|-----------------|----------------|----------------|-----------------|-----------------|
| 5 | A | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 5.2 |
| 5 | B | -2.8 | 0.0 | 3.1 | 0.0 | -3.0 | -1.8 | 2.8 | -3.6 | 0.0 | 3.8 |
| 5 | C | -3.0 | 0.0 | 3.3 | 0.0 | -3.0 | -1.7 | 0.0 | 3.5 | 2.8 | -3.8 |
| 5 | D | -3.0 | 0.0 | 3.3 | 0.0 | -3.0 | -1.7 | 0.0 | 0.1 | 0.0 | -0.1 |
| 5 | E | -2.8 | 0.0 | 3.1 | 0.0 | -3.0 | -1.8 | 0.0 | -0.1 | 0.0 | 0.0 |
| 5 | F | -6.8 | -6.0 | 0.0 | 6.0 | -1.4 | -0.8 | 0.0 | 0.0 | -10.4 | -9.1 |

| Frm Line | Col Line | E2UNB_SL_R Horz | E2UNB_SL_R Vert | -LWIND1_L Horz | -LWIND1_L Vert | -LWIND1_R Horz | -LWIND1_R Vert | -LWIND2_L Horz | -LWIND2_L Vert | -LWIND2_R Horz | -LWIND2_R Vert |
|----------|----------|-----------------|-----------------|----------------|----------------|----------------|----------------|----------------|----------------|----------------|----------------|
| 5 | A | 0.0 | 1.6 | 0.0 | -0.5 | 0.0 | 0.0 | 0.0 | -0.5 | 0.0 | 0.0 |
| 5 | B | 0.0 | 3.3 | 0.0 | -0.1 | 0.1 | -0.1 | 0.0 | -0.1 | 0.1 | -0.1 |
| 5 | C | 0.1 | 4.0 | 0.1 | -0.1 | 0.0 | 0.1 | 0.1 | -0.1 | 0.0 | 0.1 |
| 5 | D | 0.0 | 14.6 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| 5 | E | 0.0 | 14.1 | 0.0 | 0.0 | 0.0 | -0.1 | 0.0 | 0.0 | 0.0 | -0.1 |
| 5 | F | 0.0 | 5.2 | 0.0 | 0.0 | 0.0 | -0.5 | 0.0 | 0.0 | 0.0 | -0.5 |

ANCHOR BOLT SUMMARY

| Qty | Locate | Dia (in) | Type |
|-----|---------|----------|------|
| 40 | Endwall | 3/4" | |
| 8 | Frame | 3/4" | |
| 16 | Frame | 1" | |
| 16 | Floor | 1" | |
| 8 | Floor | 3/4" | |

BUILDING BRACING REACTIONS

| Wall Loc | Col Line | Wind Reactions(k) | Seismic Reactions(k) | Panel Shear (lb/ft) | Note |
|----------|----------|-------------------|---------------------------|---------------------|------|
| L_EW | 3 | | | | (h) |
| F_SW | A | 3.4 | 6.8 * | 8.4 * | |
| R_EW | 5 | B,C | Bracing, see EW reactions | | |
| B_SW | F | 5.4 | 6.8 * | 10.4 * | |

(h) Rigid frame at endwall

*See RF reactions table for vertical and horizontal reactions in plane of the rigid frame.

FLOOR BRACING REACTIONS

| Orient | Offset | Location | | Reactions(k) | |
|--------|--------|----------|-----|--------------|------|
| | | Start | End | Seismic | Horz |
| Tran | 4 | F | E | 4.2 | 3.1 |

ANCHOR BOLTS & BASE PLATES

| Frm Line | Col Line | Anc_Bolt Qty | Anc_Bolt Dia | Base_Plate Width | Base_Plate Length | Base_Plate Thick | Grout (in) |
|----------|----------|--------------|--------------|------------------|-------------------|------------------|------------|
| 5 | A | 4 | 0.750 | 6.000 | 8.125 | 0.375 | 0.0 |
| 5 | B | 4 | 0.750 | 6.000 | 8.125 | 0.375 | 0.0 |
| 5 | C | 4 | 0.750 | 6.000 | 8.125 | 0.375 | 0.0 |
| 5 | D | 4 | 0.750 | 6.000 | 8.125 | 0.375 | 0.0 |
| 5 | E | 4 | 0.750 | 6.000 | 8.125 | 0.375 | 0.0 |
| 5 | F | 4 | 0.750 | 6.000 | 7.875 | 0.375 | 0.0 |

DESIGN INFORMATION

- All loading conditions are examined and only the maximum / minimum H or V and the corresponding H or V are reported.
- Positive reactions are shown in the sketch. Foundation loads are in opposite directions.
- Bracing reactions are in the plane of the brace with the H pointing away from the braced bay. The vertical reaction is downward.
- Building reactions are based on the following building data:

| | | |
|---|--|-------------------------------|
| ----- DESIGN CRITERIA ----- | ----- SEISMIC CRITERIA ----- | ----- DEFLECTION LIMITS ----- |
| Width (ft) = 85 | Seismic Importance = 1.00 | ENDWALL COLUMN L/ 120 |
| Length (ft) = 40 | Occupancy Category = II - Normal | ENDWALL RAFTER (Live) L/ 180 |
| Eave Height (ft) = 20 | | ENDWALL RAFTER (Wind) L/ 180 |
| Roof Slope (rise/12) = 1.0: 12 | Mapped Spectral Response Accelerations | WALL GIRTS L/ 90 |
| Building Code = IBC 09 | Ss = 0.2960 | PURLIN (LIVE) L/ 150 |
| Local Code (State/Prov) = IBC 09 | S1 = 0.0820 | PURLIN (WIND) L/ 150 |
| Dead Load (psf) = 4.840 | ---Spectral Response Coefficients--- | WALL PANEL L/ 90 |
| Collateral Load (psf) = 5.00 | Sds = 0.3085 | ROOF PANEL (Live) L/ 120 |
| Roof Live Load (psf) = 20.00 | Sd1 = 0.1312 | ROOF PANEL (Wind) L/ 120 |
| Frame Live Load (psf) = 20.00 | Site Class = D | Main Frame (Horiz) L/ 60 |
| Snow: Ground Snow Load (psf) = 90.00 | Seismic Design Category = B | Main Frame (Vert) L/ 180 |
| Snow Importance = 1.00 | | WIND BRACING L/ 60 |
| Thermal Coefficient = 1.00 | -----Base Shear----- | Main Frame (Crane) L/ 100 |
| Snow Exposure Factor = 1.00 | Expanded Formula = 0.667*le*Fa*Ss*W/R | Main Frame (Seismic) L/ 50 |
| Slippery Roof = N | Longitudinal Base Shear = 18.82 | SEISMIC BRACING L/ 50 |
| Roof Snow Load (psf) = 63 | Transverse Base Shear = 17.51 | PARTITION COLUMN L/ 120 |
| Wind: Ultimate Wind Speed (mph) = 100 mph | ---Seismic Response Coefficients--- | PARTITION GIRT L/ 120 |
| Occupancy Category = II - Normal | Frame = 0.103 | PARTITION PANEL L/ 120 |
| Importance - Wind = 1.00 | FSW = 0.103 | |
| Wind Exposure = C | BSW = 0.103 | |
| Enclosure Classification = C | ---Response Modification Factors--- | |
| ---Internal Pressure Coefficients--- | Frame = 3 | |
| Pressure = 0.18 | FSW = 3 | |
| Suction = -0.18 | BSW = 3 | |
| -----Components & Cladding----- | | |
| Design Pressure: Pressure (psf) = 21.15 | | |
| Suction (psf) = -28.15 | | |
| Equivalent Lateral Brace Force Procedure. | | |
| Steel systems not specifically detailed for seismic resistance. | | |



400112 KUA OFFICE
85'-0" x 40'-0" x 20'-0"

DATE: 5/8/18 REVISION: 01
ENG: TVM DWN: BJC APPD: TVM

F.O. 21766

REVISION HISTORY

| REV. | DESCRIPTION | DATE |
|------|--------------------------|---------|
| 01 | UPDATED BRACING LOCATION | 5-14-18 |

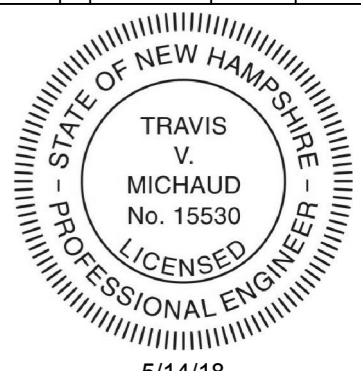
400112 KUA OFFICE

DRAWING STATUS

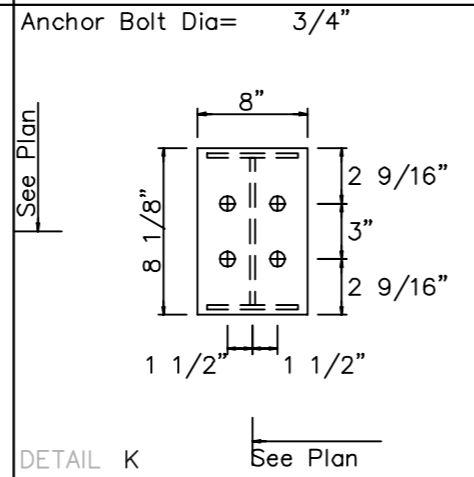
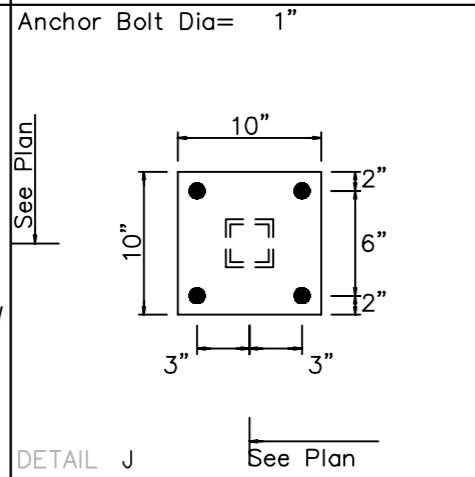
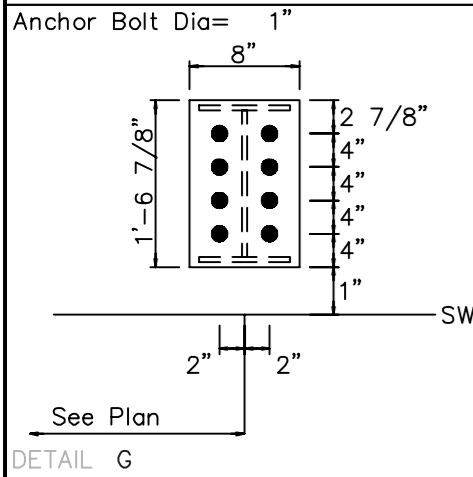
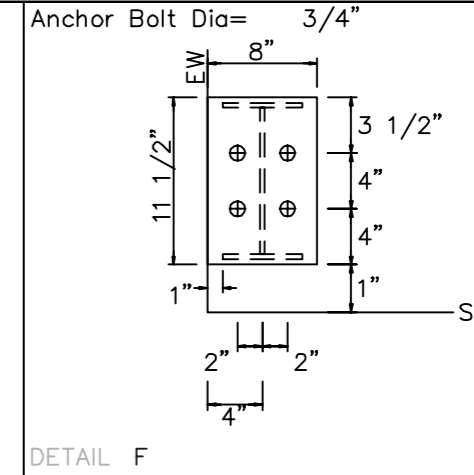
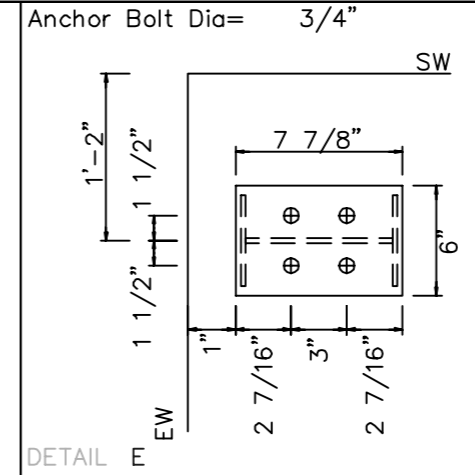
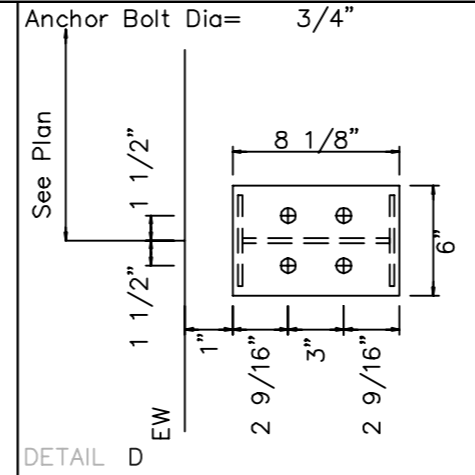
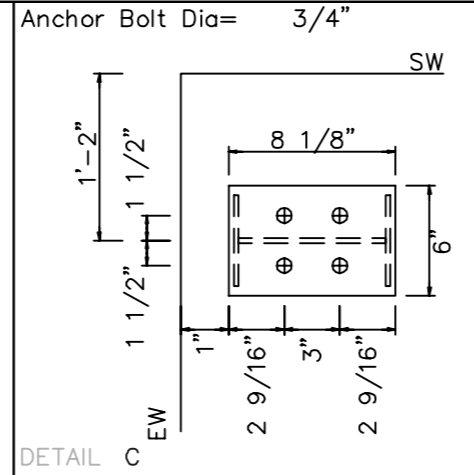
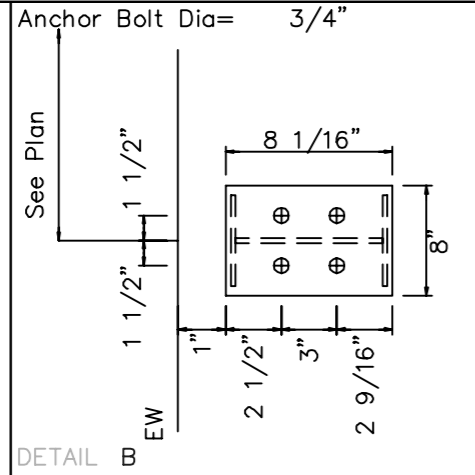
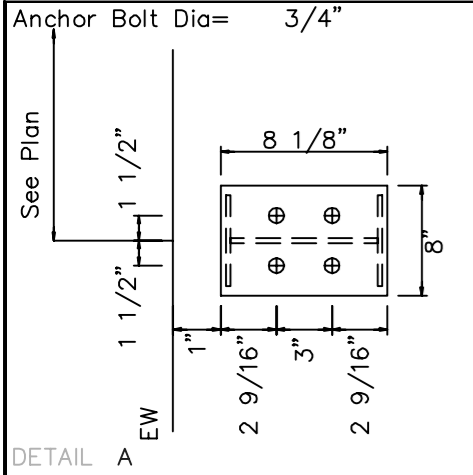
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ADDITIONAL LOADING INFORMATION

| | | | |
|----------------------------|----------|----------------------------|-----------|
| <u>Mezzanine #1 Loads:</u> | | <u>Mezzanine #2 Loads:</u> | |
| Dead Load | 50.0 PSF | Dead Load | _____ PSF |
| Collateral Load | 5 PSF | Collateral Load | _____ PSF |
| Live Load | 125 PSF | Live Load | _____ PSF |

Crane Information:

Crane Type _____

CMAA Service Class _____

Crane capacity = _____ Kips

Bridge Weight = _____ Kips

Hoist/Trolley Weight = _____ Kips

Wheel Spacing = _____ Ft.

Additional Loads:

- _____
- _____
- _____

ALLIED

400112 KUA OFFICE

85'-0" x 40'-0" x 20'-0"

DATE: 5/8/18 REVISION: 0

ENG: TVM DWN: BJC APPD: TVM

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STATE OF NEW HAMPSHIRE

TRAVIS V. MICHAUD
No. 15530

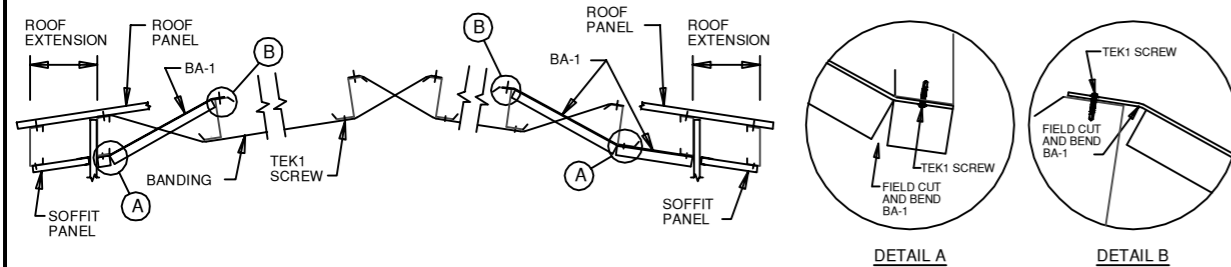
LICENSED PROFESSIONAL ENGINEER

5/14/18

PAGE 5 OF 22

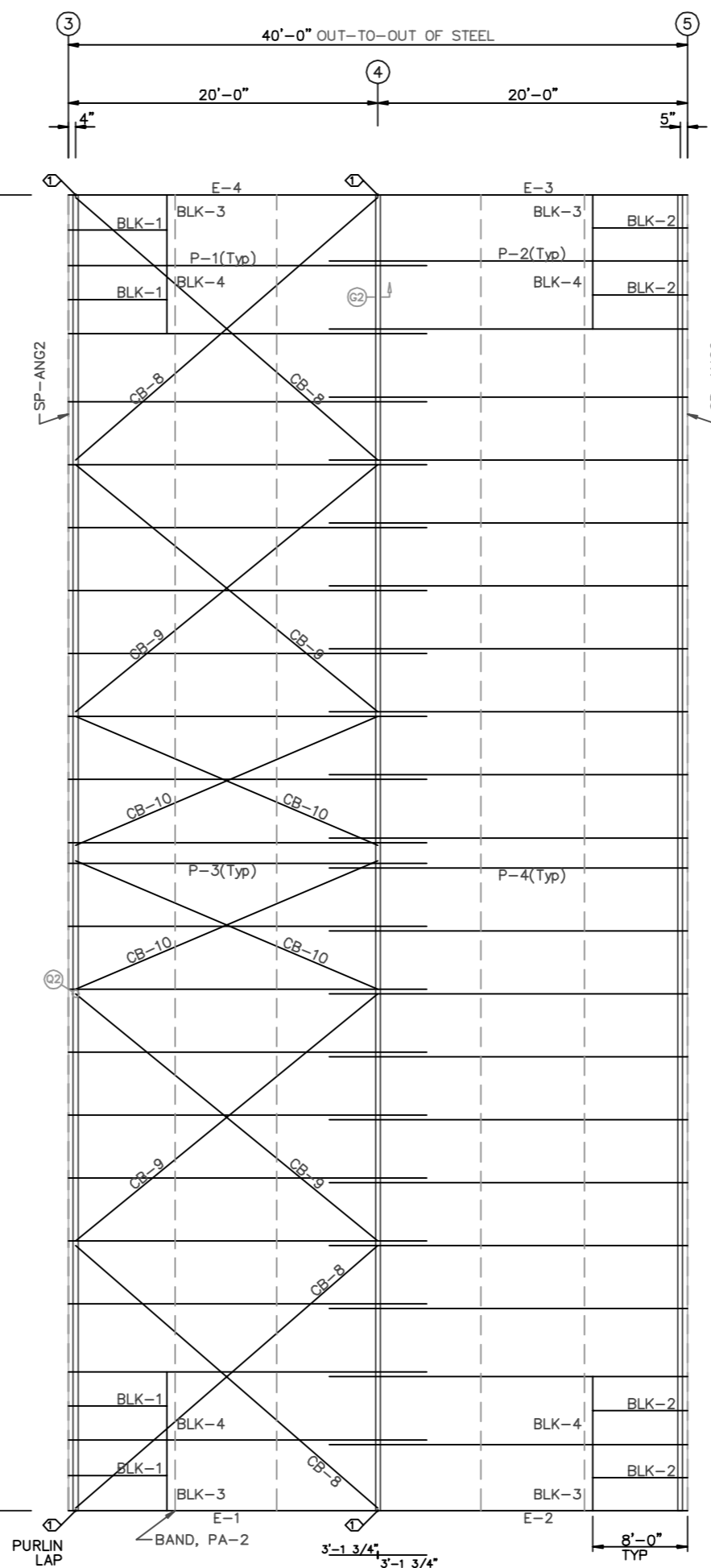
STANDARD PURLIN BRACING DETAIL FOR SCREW-DOWN PANELS

NOTE 1: SPACE BANDING EVENLY ACROSS BAYS



CONDITION 1:
FIRST PURLIN SPACE
GREATER THAN 2'-0"

CONDITION 2:
FIRST PURLIN SPACE LESS
THAN OR EQUAL TO 2'-0"



| MEMBER TABLE | | | |
|--------------|-------|----------|--------------|
| ROOF PLAN | | | |
| QUAN | MARK | PART | LENGTH |
| 10 | P-1 | 10X25Z12 | 23'-1 1/2" |
| 10 | P-2 | 10X25Z12 | 23'-1 1/2" |
| 10 | P-3 | 10X25Z12 | 23'-1 1/2" |
| 10 | P-4 | 10X25Z12 | 23'-1 1/2" |
| 1 | E-1 | 10X35E10 | 19'-11 1/2" |
| 1 | E-2 | 10X35E10 | 19'-11 1/2" |
| 1 | E-3 | 10X35E10 | 19'-11 1/2" |
| 1 | E-4 | 10X35E10 | 19'-11 1/2" |
| 4 | CB-8 | CABLE500 | 23'-1 13/16" |
| 4 | CB-9 | CABLE500 | 22'-6 1/4" |
| 4 | CB-10 | CABLE500 | 18'-9 3/8" |
| 4 | BLK-1 | 10X25Z16 | 8'-0" |
| 4 | BLK-2 | 10X25Z16 | 8'-0" |
| 4 | BLK-3 | 8X35C16 | 3'-11 13/16" |
| 4 | BLK-4 | 8X35C16 | 4'-5" |

| SPECIAL BOLTS | | | | | |
|---------------|------|------|------|--------|------|
| ROOF PLAN | | | | | |
| Q ID | QUAN | TYPE | DIA | LENGTH | WASH |
| 1 | 4 | A325 | 1/2" | 1 1/4" | 0 |



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85'-0" x 40'-0" x 20'-0"
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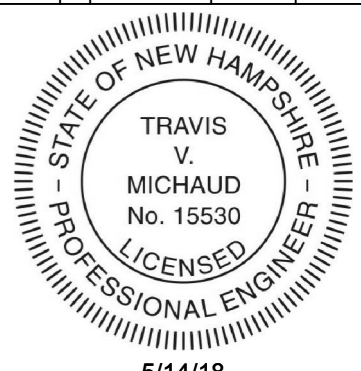
| REVISION HISTORY | |
|------------------|-------------|
| REV. | DESCRIPTION |
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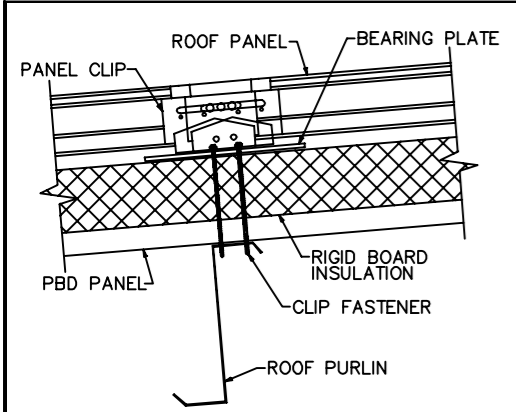


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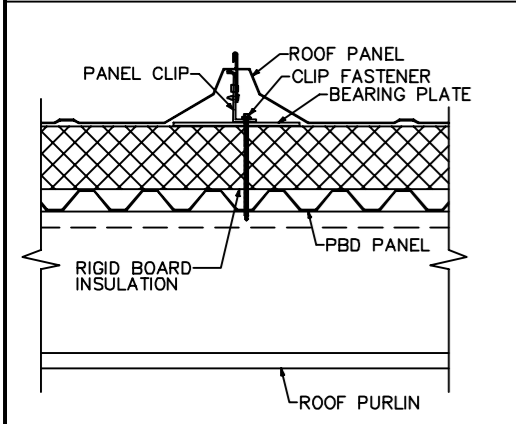
- GENERAL NOTES:**
1. Screw Down Roof: Use TEK5WW screws in place of SD150 panel screws at all 10 gage purlins, eave struts, or roof joists.
 2. Standing Seam Roof: Use FST#6 in place of FST#1 clip to purlin screws at all 10 gage purlins, eave struts, or at roof joists.

ROOF FRAMING PLAN

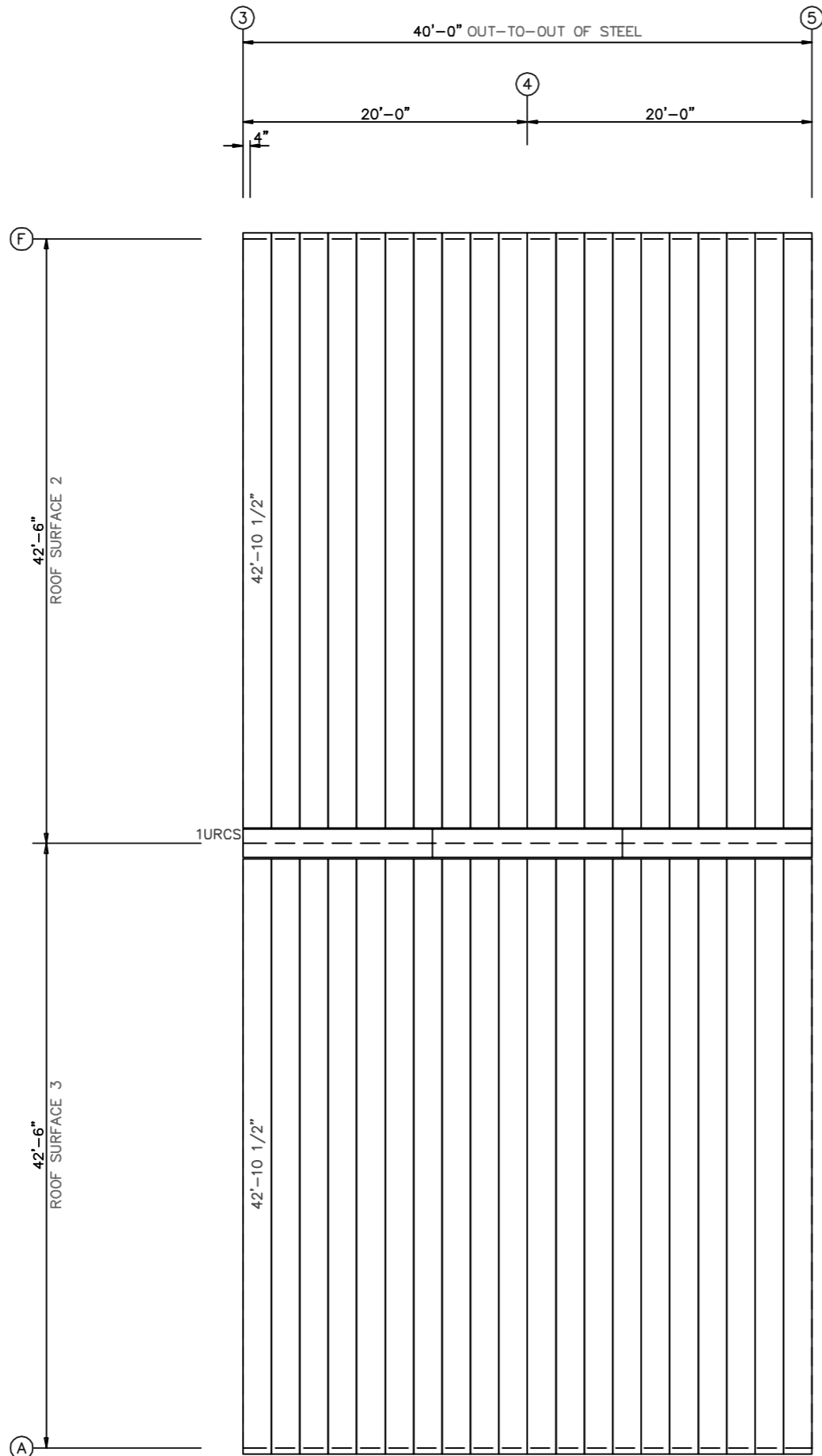
NOTE(S):
1. ATTACH BLOCKING USING JC CLIPS WITH (8) TEK1 OR TEK5 SCREWS PER CLIP.



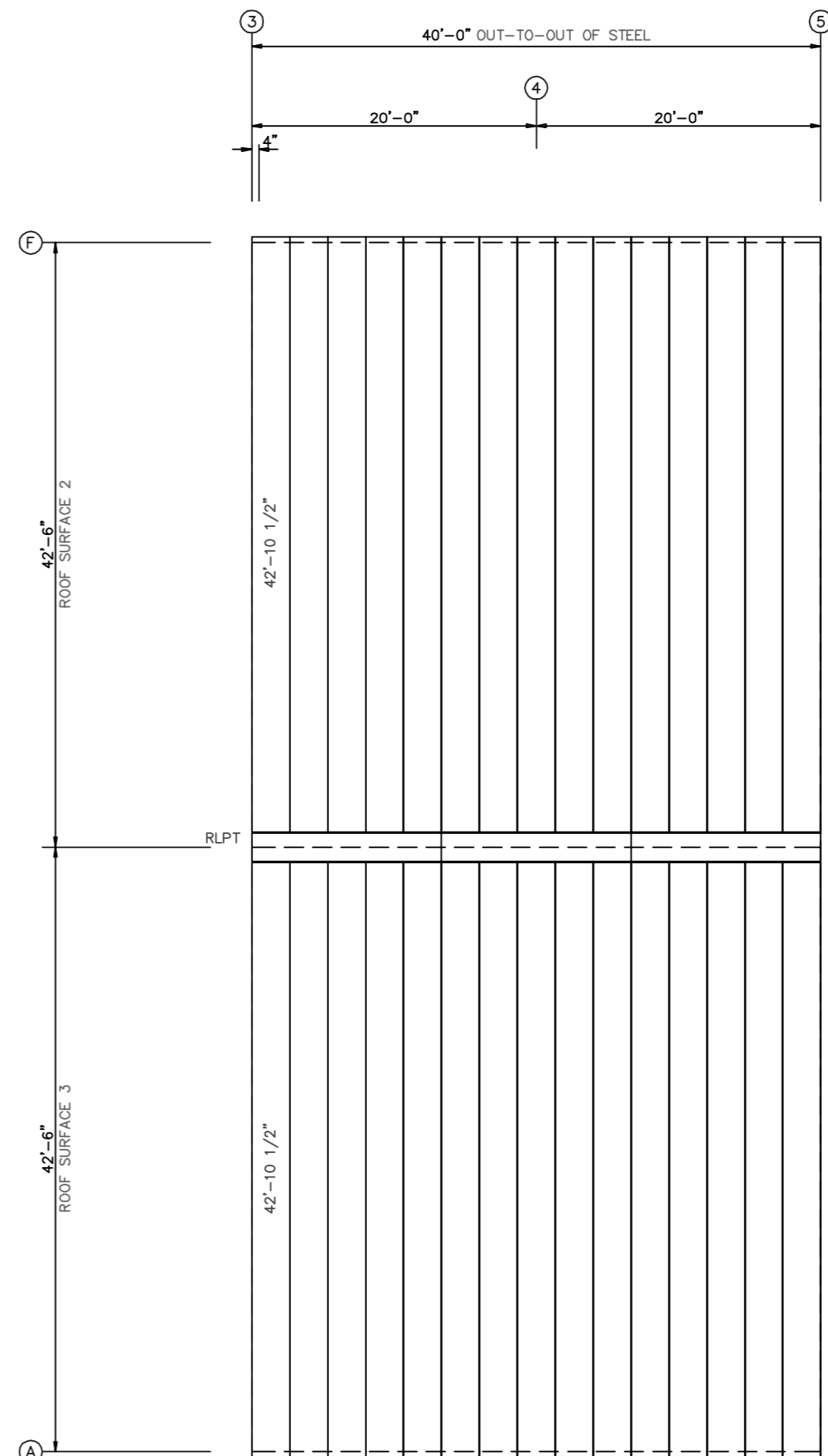
CROSS-SECTION FROM ENDWALL



CROSS-SECTION FROM SIDEWALL



ROOF SHEETING PLAN
PANELS: 24 Ga. L4 - Galvalume



DECKING PLAN
PANELS: 24 Ga. PBD - Polar White

GENERAL NOTES:
Panel "Start" and "End" dimensions must be followed for the proper installation of the gable trim(s) provided.

ALLIED
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| DRAWING STATUS | | REVISION HISTORY | |
|----------------|------------------|------------------|--|
| REV. | DESCRIPTION | DATE | |
| 1 | CUSTOMER REQUEST | 5/11/18 | |

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FOR CONSTRUCTION: FINAL DRAWINGS.

SPLICE BOLT TABLE

| Mark | Qty | | Int | Type | Dia | Length |
|------|-----|-----|-----|------|-------|--------|
| | Top | Bot | | | | |
| SP-1 | 4 | 4 | 0 | A325 | 1.000 | 2.75 |
| SP-2 | 4 | 4 | 0 | A325 | 0.500 | 1.50 |

FLANGE BRACES: Both Sides(U.N.)

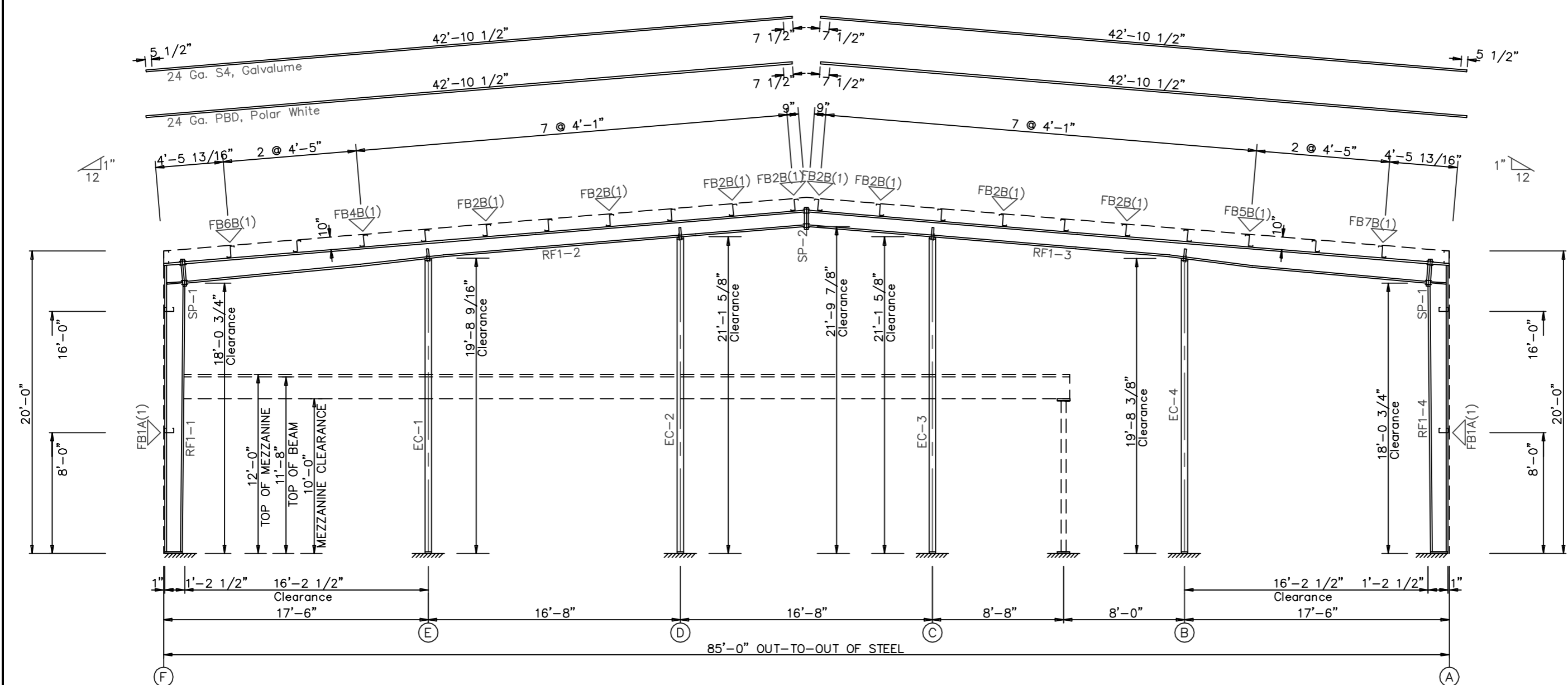
- FBxxA(1)
- A - L15X1/8
- B - L20X3/16

CAP PLATE BOLTS

| Mark | Qty | Type | Dia | Length |
|------|-----|------|-------|--------|
| | | | | |
| EC-2 | 4 | A325 | 0.750 | 1.50 |
| EC-3 | 4 | A325 | 0.750 | 1.50 |
| EC-4 | 4 | A325 | 0.500 | 1.25 |

MEMBER TABLE

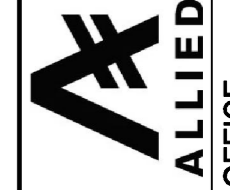
| Mark | Web Depth | | Web Thick | Plate Length | Outside Flange | | | Inside Flange | | |
|-------|-----------|------|-----------|--------------|----------------|---------|----------|---------------|----------|---------|
| | Start | End | | | W | Thk | Length | W | Thk | Length |
| RF1-1 | 11.0 | 14.0 | 0.188 | 230.4 | 6 x 1/4" | x 229.3 | 6 x 1/4" | x 13.8 | 6 x 1/4" | x 212.6 |
| RF1-2 | 14.0 | 12.0 | 0.188 | 140.3 | 6 x 1/4" | x 448.1 | 6 x 1/4" | x 140.3 | 6 x 1/4" | x 66.4 |
| | 12.0 | 10.0 | 0.188 | 66.4 | 6 x 1/4" | x 48.0 | 6 x 1/4" | x 288.6 | 6 x 1/4" | x 288.6 |
| | 10.0 | 10.0 | 0.188 | 229.5 | | | | | | |
| | 10.0 | 10.0 | 0.188 | 60.0 | | | | | | |
| RF1-3 | 10.0 | 10.0 | 0.188 | 60.0 | 6 x 1/4" | x 48.0 | 6 x 1/4" | x 288.6 | 6 x 1/4" | x 288.6 |
| | 10.0 | 10.0 | 0.188 | 229.5 | 6 x 1/4" | x 448.1 | 6 x 1/4" | x 66.4 | 6 x 1/4" | x 66.4 |
| | 10.0 | 13.0 | 0.188 | 66.4 | | | | | | |
| | 13.0 | 14.0 | 0.188 | 140.3 | | | | | | |
| RF1-4 | 14.0 | 11.0 | 0.188 | 230.4 | 8 x 1/4" | x 13.8 | 6 x 1/4" | x 229.3 | 6 x 1/4" | x 212.6 |
| EC-1 | W8X18 | | | | | | | | | |
| EC-2 | W8X28 | | | | | | | | | |
| EC-3 | W8X28 | | | | | | | | | |
| EC-4 | W8X18 | | | | | | | | | |



BUILDING CROSS SECTION: FRAME LINE 3

GENERAL NOTES:

- See Detail Sheets for Connection Information.
- See Shipping List for Flange Brace Lengths.



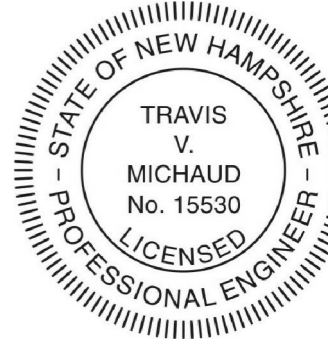
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 DATE: 5/8/18 REVISION: 0
 ENG: TVM DWN: BJC APPD: TVM

F.O. 21766

| REV. | DESCRIPTION | DATE |
|------|-------------|------|
| | | |

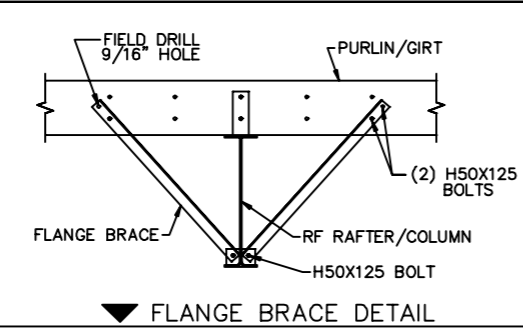
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 FOR CONSTRUCTION: FINAL DRAWINGS.



5/14/18

| SPLICE BOLT TABLE | | | | | | |
|-------------------|-----|-----|-----|------|-------|--------|
| Mark | Qty | | Int | Type | Dia | Length |
| | Top | Bot | | | | |
| SP-1 | 4 | 4 | 8 | A325 | 1.250 | 3.50 |
| SP-2 | 4 | 4 | 4 | A325 | 1.000 | 2.50 |



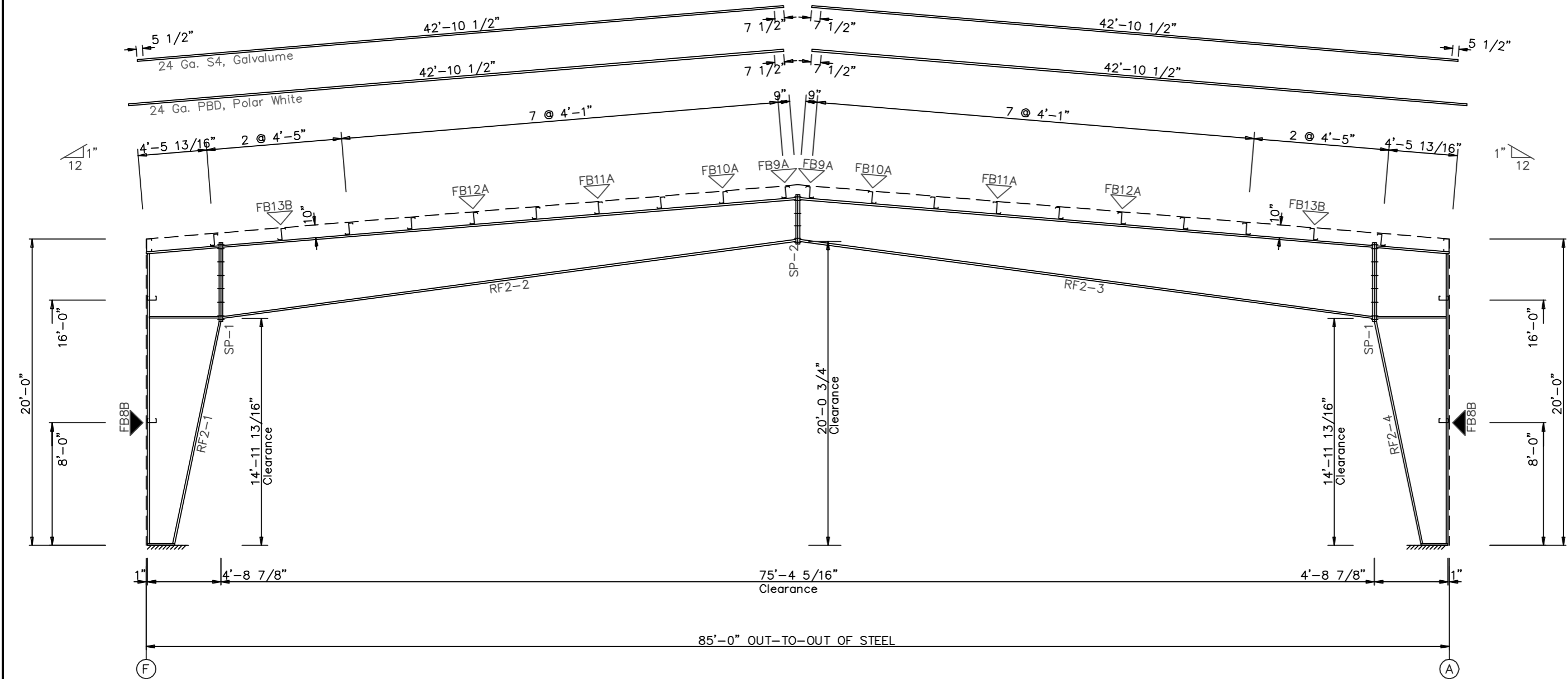
| Mark | Web Depth | | Web Thick | Plate Length | Outside Flange | | | Inside Flange | | |
|-------|-----------|-----|-----------|--------------|-------------------|-----|--------|------------------|-----|--------|
| | Start | End | | | W | Thk | Length | W | Thk | Length |
| RF2-1 | 18.0/56.0 | | 0.375 | 233.4 | 8 x 1/4" x 120.0 | | | 8 x 5/8" x 178.4 | | |
| | | | | | 8 x 5/16" x 108.7 | | | | | |
| | | | | | 8 x 5/16" x 57.4 | | | | | |
| | | | | | 8 x 5/16" x 211.7 | | | | | |
| RF2-2 | 54.0/43.0 | | 0.375 | 216.2 | 8 x 5/16" x 120.0 | | | 8 x 5/8" x 96.4 | | |
| | 43.0/37.0 | | | | 8 x 3/8" x 120.0 | | | 8 x 3/8" x 120.2 | | |
| | 37.0/31.0 | | | | 8 x 1/2" x 119.9 | | | 8 x 1/4" x 237.6 | | |
| RF2-3 | 31.0/36.9 | | 0.219 | 118.4 | 8 x 1/2" x 118.4 | | | 8 x 1/4" x 236.1 | | |
| | 36.9/43.0 | | | | 8 x 3/8" x 120.0 | | | 8 x 3/8" x 120.2 | | |
| | 43.0/54.0 | | | | 8 x 5/16" x 213.3 | | | 8 x 5/8" x 97.9 | | |
| | 56.0/18.0 | | | | 8 x 5/16" x 108.7 | | | 8 x 5/8" x 178.4 | | |
| RF2-4 | 18.0/56.0 | | 0.375 | 233.4 | 8 x 1/4" x 120.0 | | | | | |
| | | | | | 8 x 5/16" x 108.7 | | | | | |
| | | | | | 8 x 5/16" x 57.4 | | | | | |
| | | | | | 8 x 1/4" x 120.0 | | | | | |

FLANGE BRACES: Both Sides(U.N.)
 FBxxB(1)
 B - L20X3/16
 A - L15X1/8



400112 KUA OFFICE
 85'-0" x 40'-0" x 20'-0"
 DATE: 5/8/18 REVISION: 0
 ENG: TVM DWN: BJC APPD: TVM

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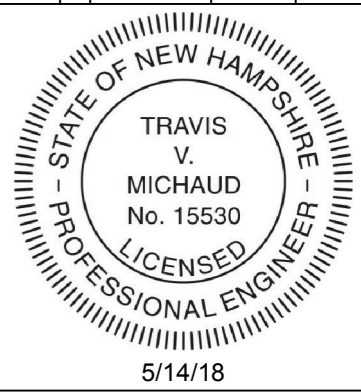
BUILDING CROSS SECTION: FRAME LINE 4

GENERAL NOTES:
 1. See Detail Sheets for Connection Information.
 2. See Shipping List for Flange Brace Lengths.

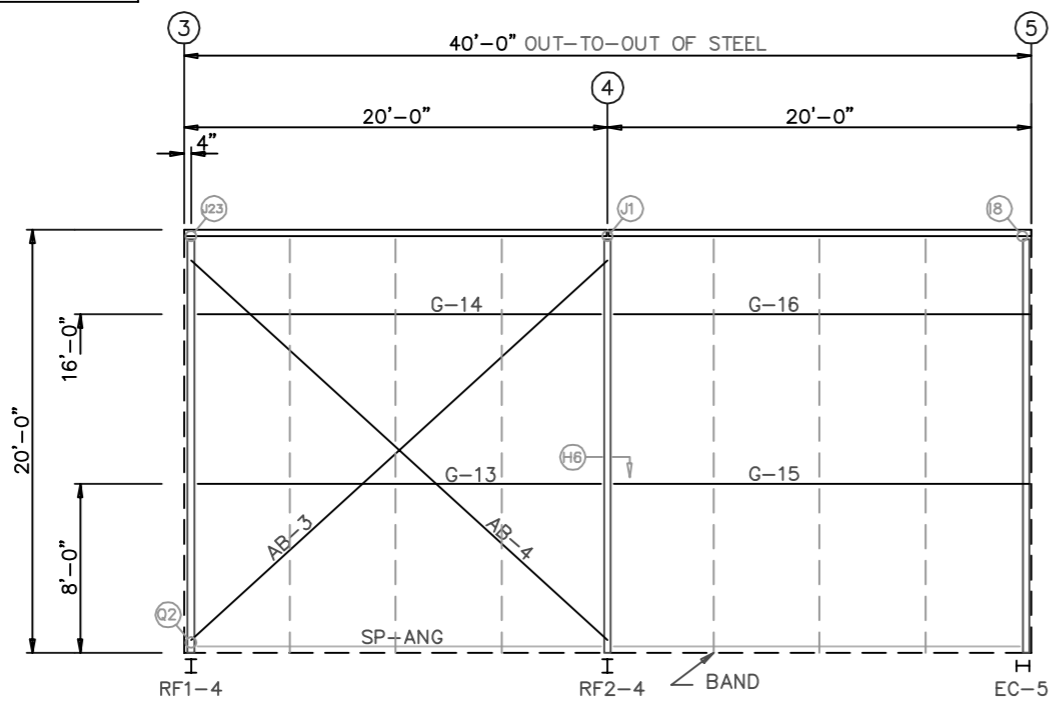
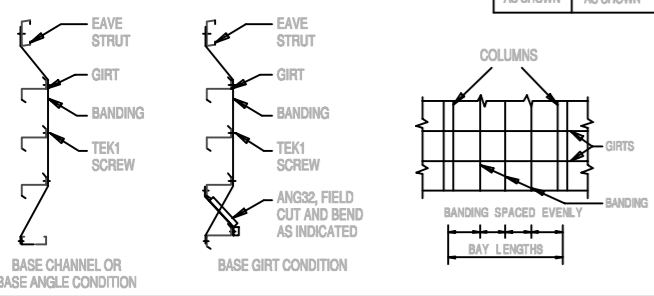
| REVISION HISTORY | |
|------------------|-------------|
| DATE | DESCRIPTION |
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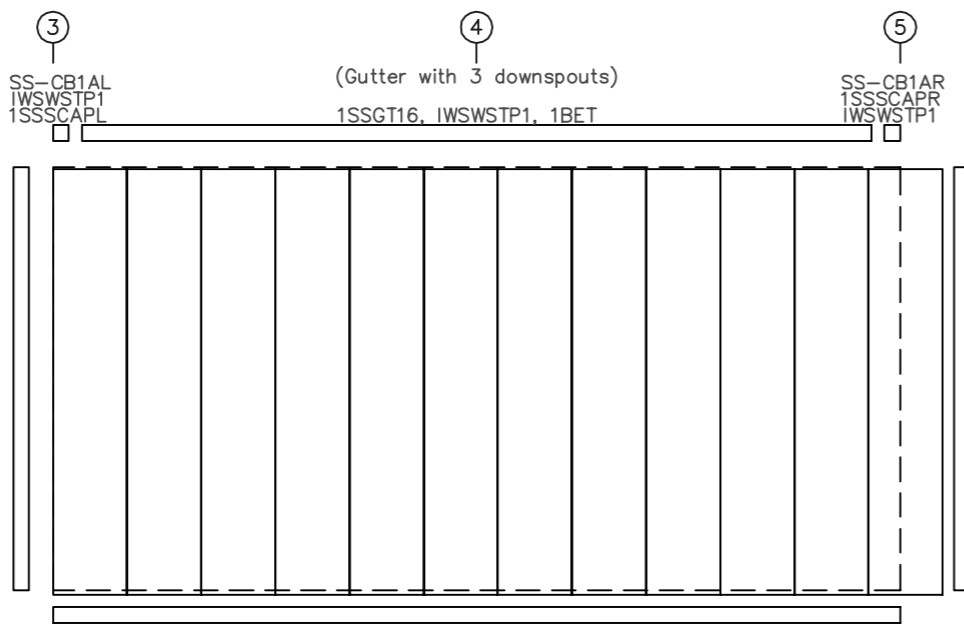
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STANDARD GIRT BRACING DETAIL FOR SIDEWALL GIRTS



SIDEWALL FRAMING: FRAME LINE A



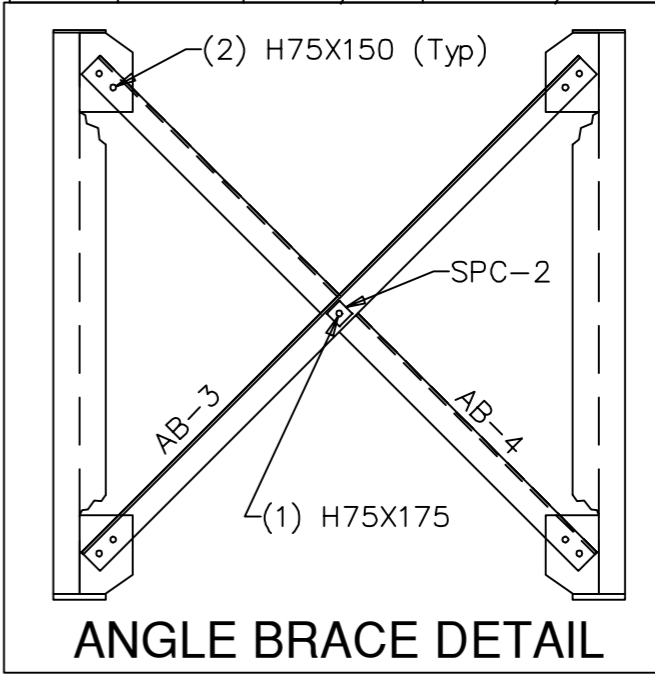
SIDEWALL SHEETING & TRIM: FRAME LINE A
PANELS: 3" CF MESA 26Ga EXTERIOR & 26Ga INTERIOR -
EXT: TBD & INT: TBD

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

MEMBER TABLE
FRAME LINE A

| QUAN | MARK | PART | LENGTH |
|------|------|---------|-------------|
| 1 | G-13 | 8X35C13 | 19'-0 1/2" |
| 1 | G-14 | 8X35C14 | 19'-0 1/2" |
| 1 | G-15 | 8X35C13 | 19'-7 1/2" |
| 1 | G-16 | 8X35C14 | 19'-7 1/2" |
| 1 | AB-3 | L30X1/4 | 24'-0 3/4" |
| 1 | AB-4 | L30X1/4 | 24'-0 7/16" |



ANGLE BRACE DETAIL

NOTE(S):
1. WALL PANEL MUST BE FASTENED TO SUPPORTS WITH MBCI FASTENING PATTERN FP2.

| TRIM COLORS | |
|---|-------------------|
| EAVE TRIM = TBD | CORNER TRIM = TBD |
| BASE TRIM = TBD | GUTTER = TBD |
| DOOR TRIM = TBD | DOWNPOUTS = TBD |
| RAKE TRIM = TBD | |
| * LINER TRIM = Liner panel color | |
| * SOFFIT TRIM = Soffit panel color | |
| * ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER. | |



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| REV. | DESCRIPTION |
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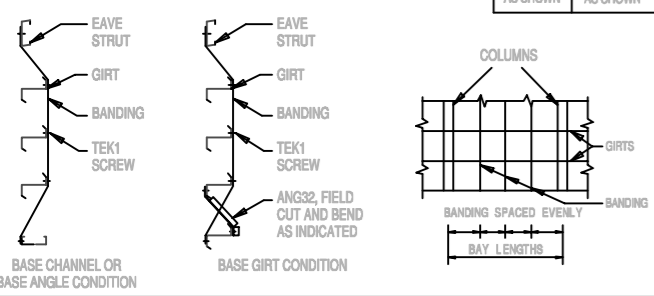
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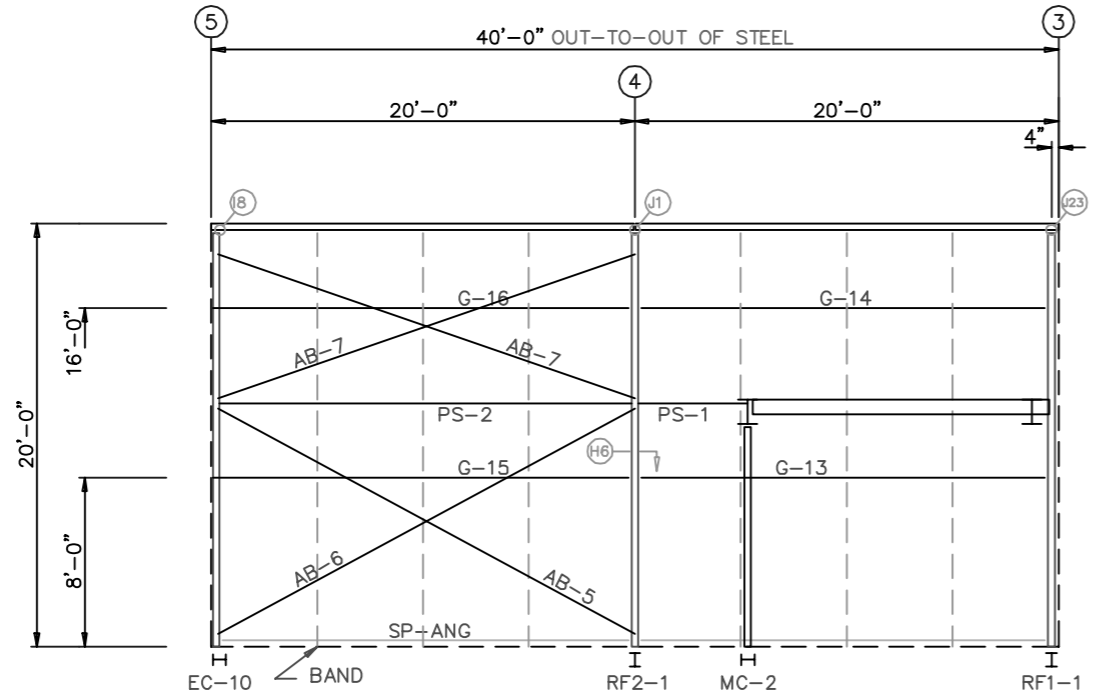
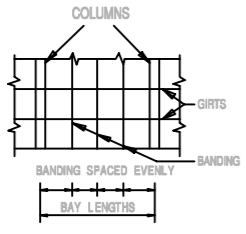
FOR CONSTRUCTION: FINAL DRAWINGS.



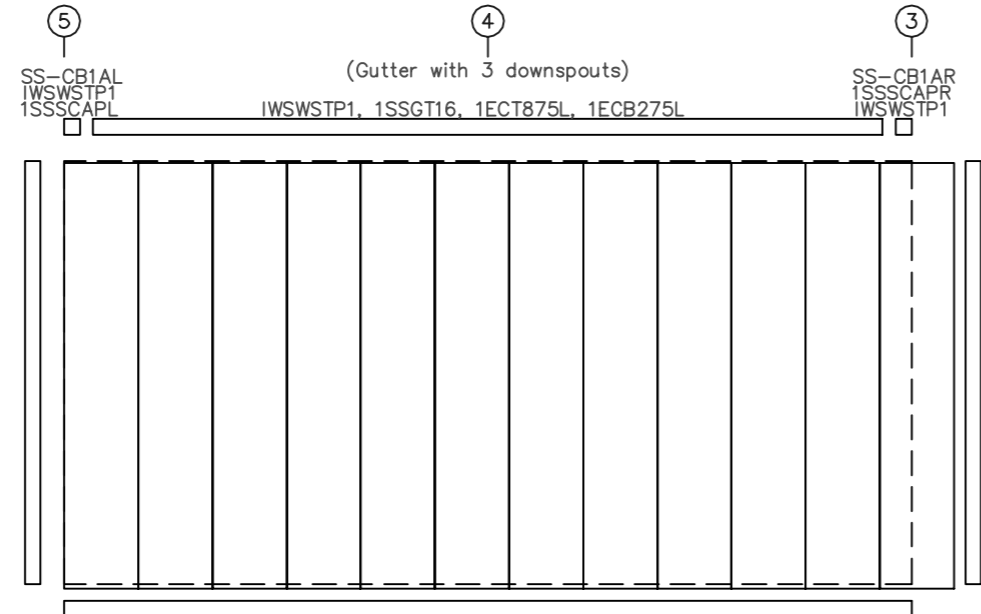
STANDARD GIRT BRACING DETAIL FOR SIDEWALL GIRTS



| BANDING REQUIREMENTS | |
|----------------------|------------|
| BAY WIDTH | RUNS REQ'D |
| AS SHOWN | AS SHOWN |



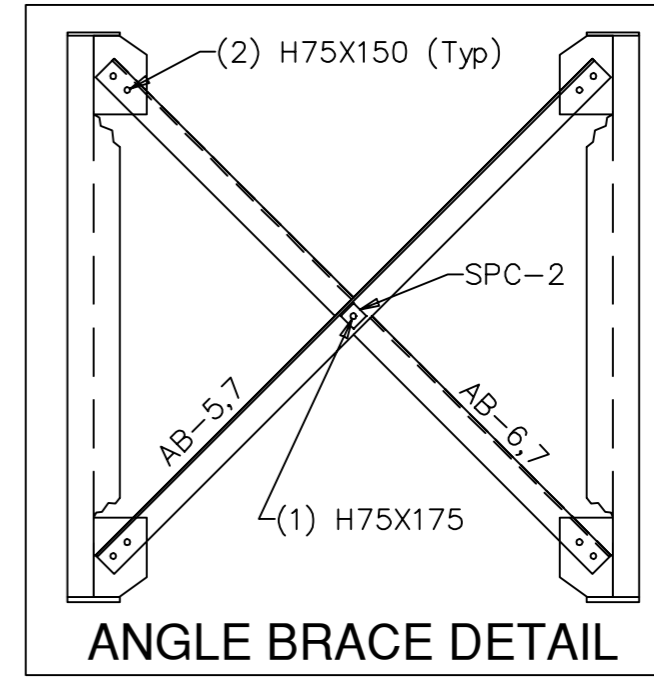
SIDEWALL FRAMING: FRAME LINE F



SIDEWALL SHEETING & TRIM: FRAME LINE F
PANELS: 3" CF MESA 26Ga EXTERIOR & 26Ga INTERIOR -
EXT: TBD & INT: TBD

| BOLT TABLE | | | |
|--------------|------|------|-----|
| FRAME LINE F | | | |
| LOCATION | QUAN | TYPE | DIA |
| PS/MB | 1 | A325 | 1" |
| PS/RF/EC | 1 | A325 | 1" |

| MEMBER TABLE | | | |
|--------------|------|---------|--------------|
| FRAME LINE F | | | |
| QUAN | MARK | PART | LENGTH |
| 1 | G-13 | 8X35C13 | 19'-0 1/2" |
| 1 | G-14 | 8X35C14 | 19'-0 1/2" |
| 1 | G-15 | 8X35C13 | 19'-7 1/2" |
| 1 | G-16 | 8X35C14 | 19'-7 1/2" |
| 1 | AB-5 | L30X1/4 | 19'-8 7/16" |
| 1 | AB-6 | L30X1/4 | 19'-8 11/16" |
| 2 | AB-7 | L30X1/4 | 18'-2 9/16" |
| 1 | PS-1 | P4X11 | 5'-4" |
| 1 | PS-2 | P4X11 | 19'-4" |



ANGLE BRACE DETAIL

NOTE(S):
1. WALL PANEL MUST BE FASTENED TO SUPPORTS WITH MBCI FASTENING PATTERN FP2.

| TRIM COLORS | |
|---|-------------------|
| EAVE TRIM = TBD | CORNER TRIM = TBD |
| BASE TRIM = TBD | GUTTER = TBD |
| DOOR TRIM = TBD | DOWNSPOUTS = TBD |
| RAKE TRIM = TBD | |
| * LINER TRIM = Liner panel color | |
| * SOFFIT TRIM = Soffit panel color | |
| * ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER. | |



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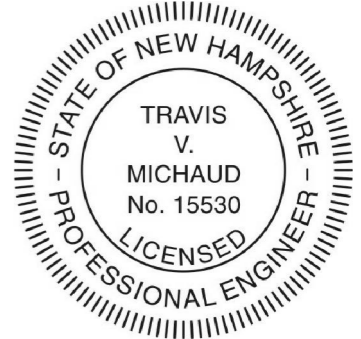
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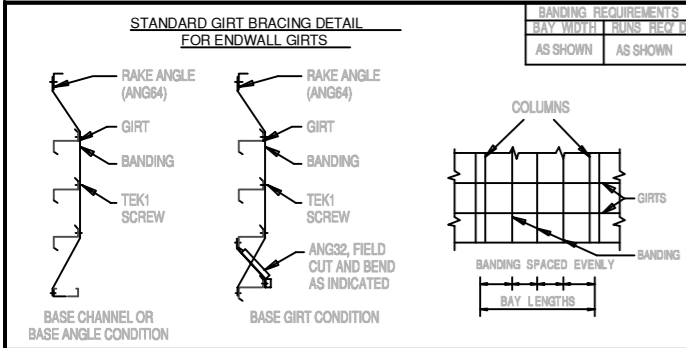
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5/14/18

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).



BOLT TABLE
FRAME LINE 3

| LOCATION | QUAN | TYPE | DIA | LENGTH |
|----------|------|------|------|--------|
| DJ-1/MB | 2 | A325 | 1/2" | 1 1/4" |

MEMBER TABLE
FRAME LINE 3

| QUAN | MARK | PART | LENGTH |
|------|------|---------|---------------|
| 1 | EC-1 | W8X18 | 19'-8 7/8" |
| 1 | EC-2 | W8X28 | 21'-1 7/8" |
| 1 | EC-3 | W8X28 | 21'-1 7/8" |
| 1 | EC-4 | W8X18 | 19'-8 11/16" |
| 2 | DJ-1 | 8X35C16 | 10'-6" |
| 2 | DJ-2 | 8X35C16 | 15'-11 3/4" |
| 1 | DH-1 | 8X35C16 | 8'-0" |
| 1 | DH-2 | 8X35C16 | 12'-0" |
| 1 | G-1 | 8X35C14 | 2'-3 3/16" |
| 1 | G-2 | 8X35C14 | 15'-11 11/16" |
| 1 | G-3 | 8X35C14 | 5'-2 3/8" |
| 1 | G-4 | 8X35C14 | 1'-11 3/8" |
| 1 | G-5 | 8X35C13 | 15'-11 11/16" |
| 1 | G-6 | 8X35C14 | 1'-6 3/16" |

ALLIED

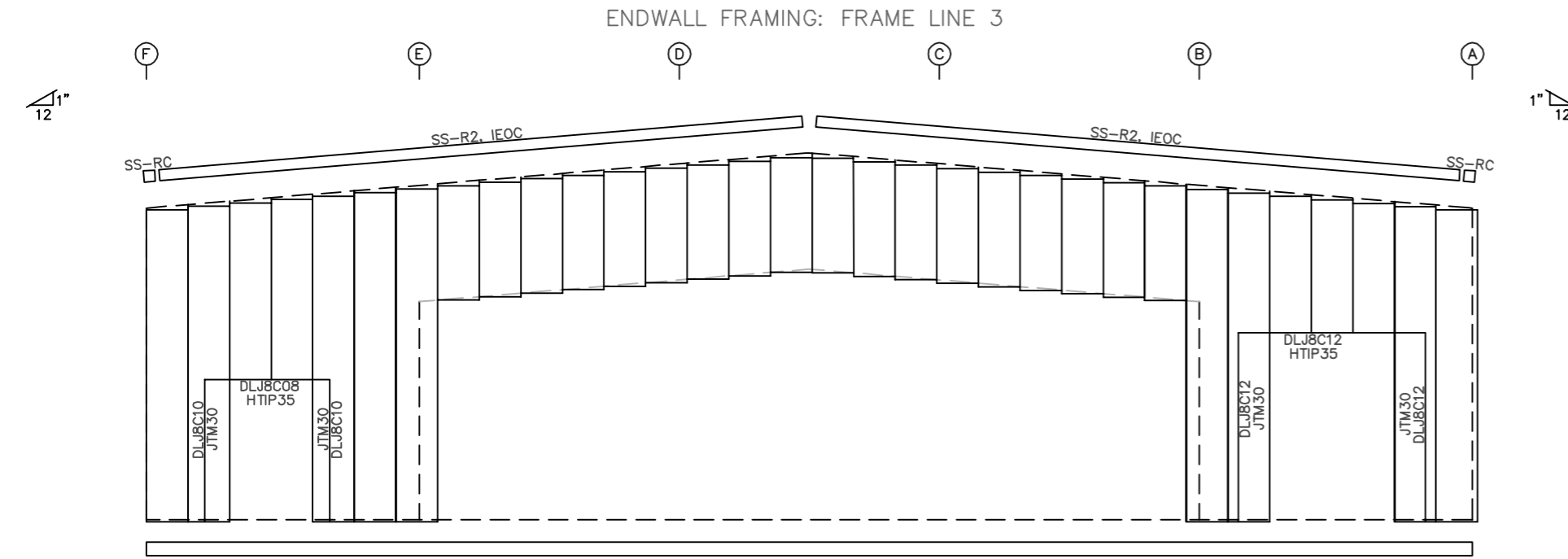
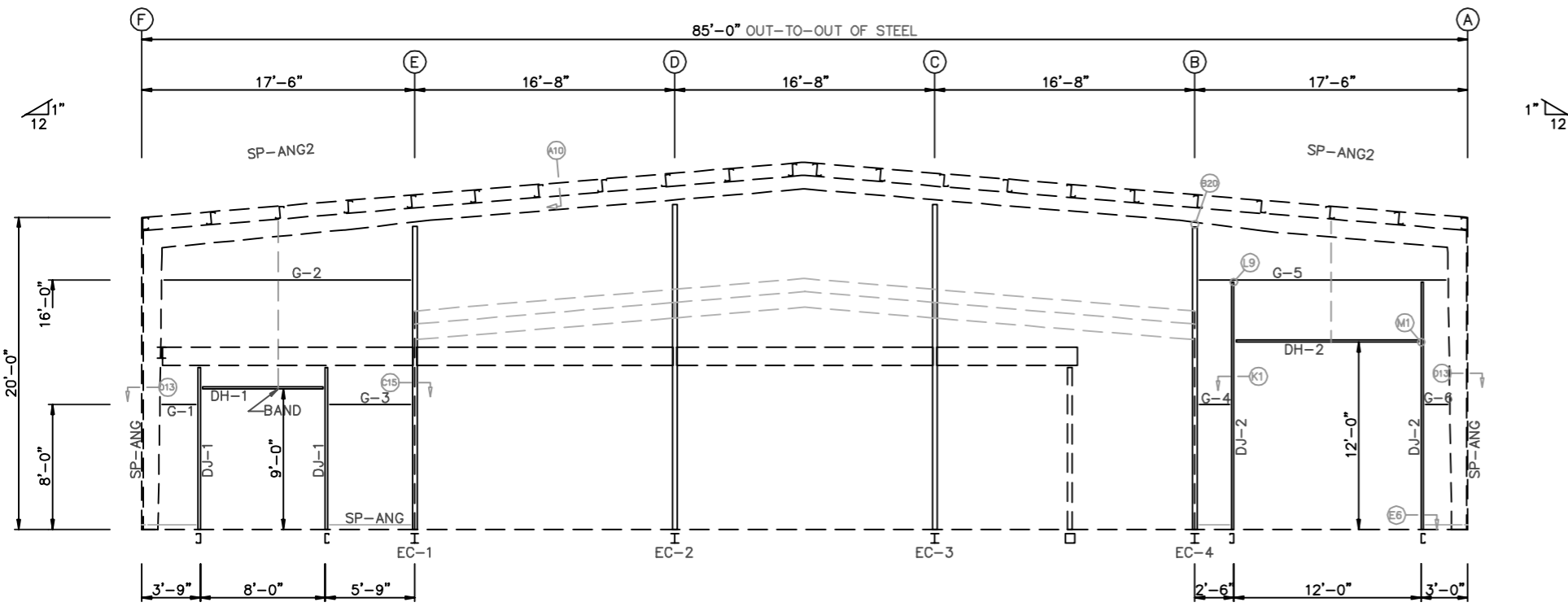
400112 KUA OFFICE

85'-0" x 40'-0" x 20'-0"

DATE: 5/8/18 REVISION: 0

ENG: TVM DWN: BJC APPD: TVM

F.O. 21766



ENDWALL SHEETING & TRIM: FRAME LINE 3
PANELS: 3" CF MESA 26Ga EXTERIOR & 26Ga INTERIOR -
EXT: TBD & INT: TBD

NOTE(S):
1. WALL PANEL MUST BE FASTENED TO SUPPORTS WITH MBCI FASTENING PATTERN FP2.

TRIM COLORS

| | |
|---|-------------------|
| EAVE TRIM = TBD | CORNER TRIM = TBD |
| BASE TRIM = TBD | GUTTER = TBD |
| DOOR TRIM = TBD | DOWNPOUTS = TBD |
| RAKE TRIM = TBD | |
| * LINER TRIM = Liner panel color | |
| * SOFFIT TRIM = Soffit panel color | |
| * ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER. | |

GENERAL NOTES:

1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
2. See detail C7A for field coping of coldform endwall column flange braces.
3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

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DRAWING STATUS

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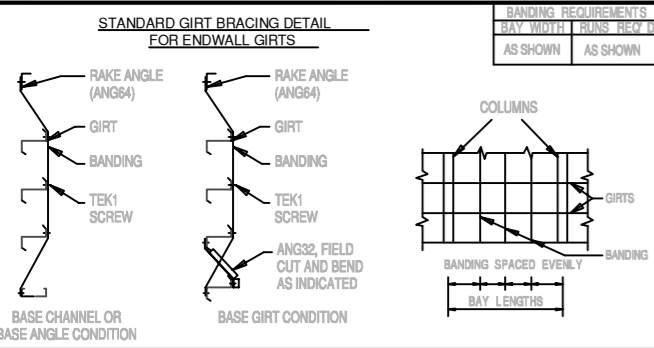
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TRAVIS V. MICHAUD
No. 15530
LICENSED PROFESSIONAL ENGINEER - STATE OF NEW HAMPSHIRE

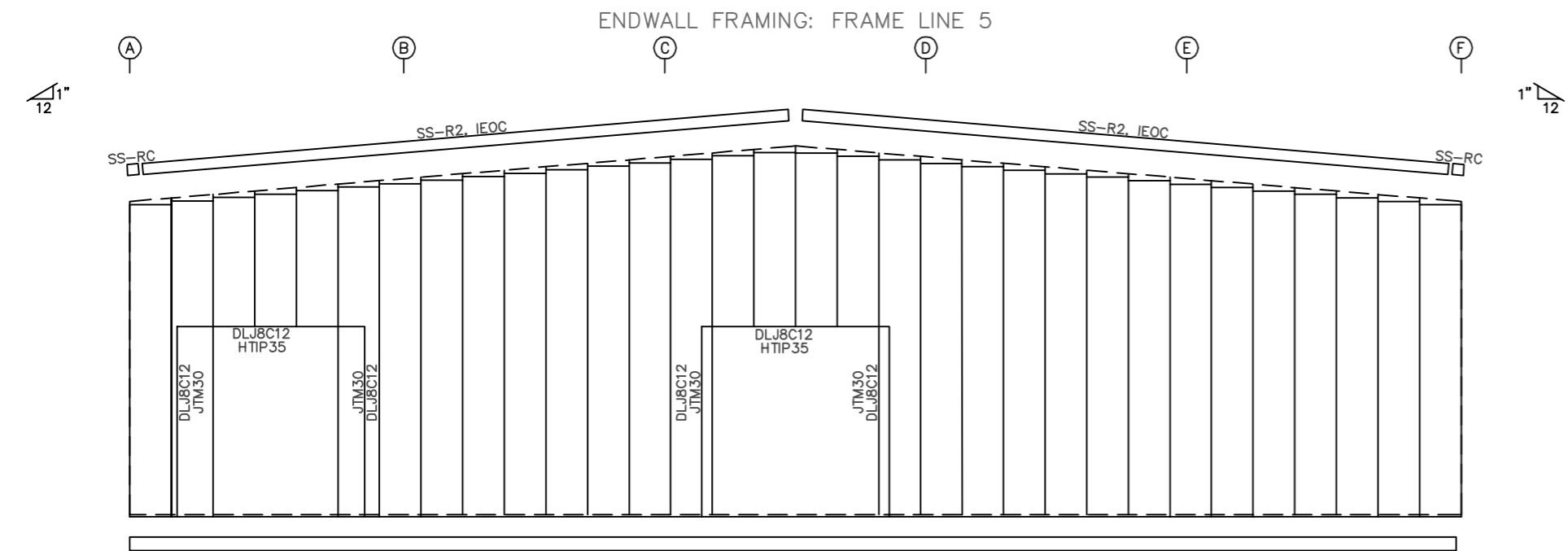
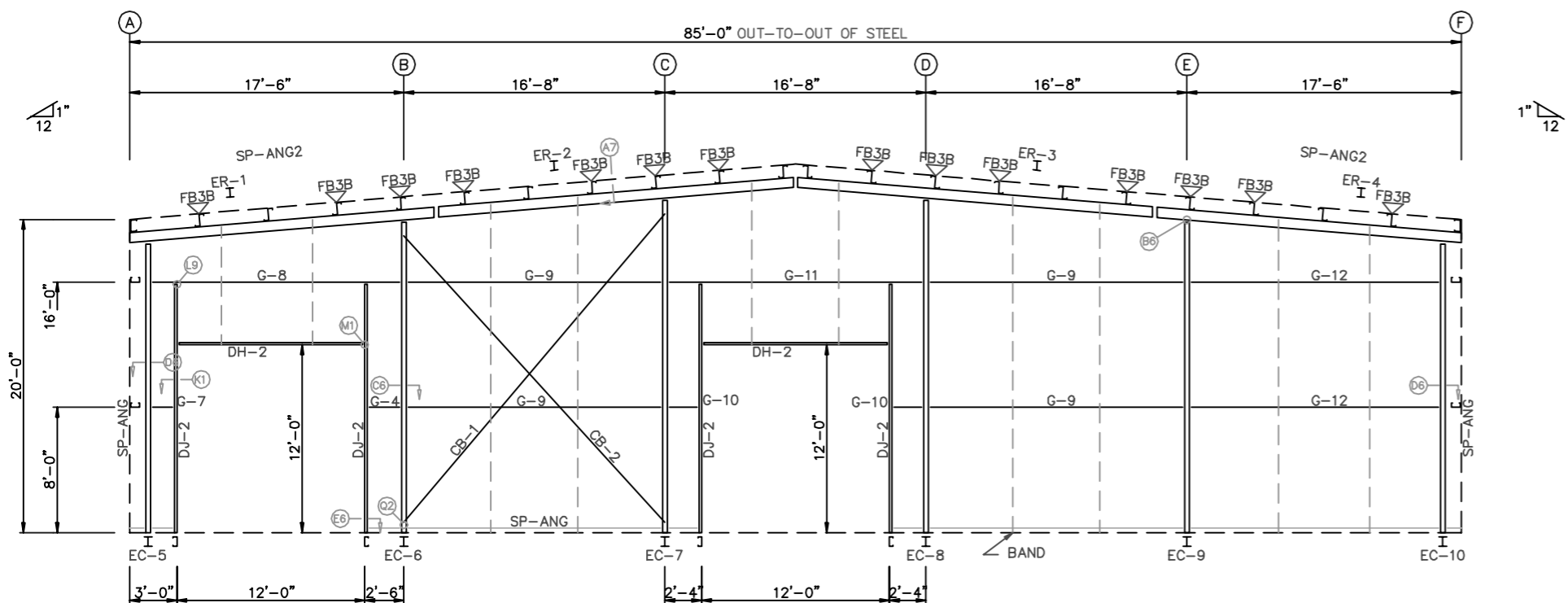
5/14/18



| BOLT TABLE FRAME LINE 5 | | | | |
|----------------------------|------|------|------|--------|
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| ER-1/ER-2 | 4 | A325 | 1/2" | 1 1/4" |
| ER-2/ER-3 | 8 | A325 | 1/2" | 1 1/2" |
| ER-3/ER-4 | 4 | A325 | 1/2" | 1 1/4" |
| Columns/Raf | 2 | A325 | 1" | 2" |

| MEMBER TABLE FRAME LINE 5 | | | |
|------------------------------|-------|----------|---------------|
| QUAN | MARK | PART | LENGTH |
| 1 | EC-5 | W8X18 | 18'-2 3/8" |
| 1 | EC-6 | W8X18 | 19'-6 11/16" |
| 1 | EC-7 | W8X18 | 20'-11 3/8" |
| 1 | EC-8 | W8X18 | 20'-11 3/8" |
| 1 | EC-9 | W8X18 | 19'-6 11/16" |
| 1 | EC-10 | W8X10 | 18'-2 3/8" |
| 1 | ER-1 | W12X16 | 19'-6 5/16" |
| 1 | ER-2 | W12X16 | 22'-11 15/16" |
| 1 | ER-3 | W12X16 | 22'-11 15/16" |
| 1 | ER-4 | W12X16 | 19'-6 5/16" |
| 4 | DJ-2 | 8X35C16 | 15'-11 3/4" |
| 2 | DH-2 | 8X35C16 | 12'-0" |
| 1 | G-4 | 8X35C14 | 1'-11 3/8" |
| 1 | G-7 | 8X35C14 | 1'-3 3/8" |
| 1 | G-8 | 8X35C14 | 15'-10 1/4" |
| 4 | G-9 | 8X35C14 | 16'-2 1/4" |
| 2 | G-10 | 8X35C14 | 1'-9 3/8" |
| 1 | G-11 | 8X35C13 | 16'-2 1/4" |
| 2 | G-12 | 8X35C14 | 15'-10 7/8" |
| 1 | CB-1 | CABLE500 | 23'-2 13/16" |
| 1 | CB-2 | CABLE500 | 22'-2 5/16" |

| FLANGE BRACE TABLE FRAME LINE 5 | | |
|------------------------------------|------|--------|
| QTY | MARK | LENGTH |
| 1 | FB3B | 1'-6" |



ENDWALL SHEETING & TRIM: FRAME LINE 5
 PANELS: 3" CF MESA 26Ga EXTERIOR & 26Ga INTERIOR -
 EXT: TBD & INT: TBD

- GENERAL NOTES:**
1. Use TEK5WW screws in place of SD150 panel screws at all 10 gage members.
 2. See detail C7A for field coping of coldform endwall column flange braces.
 3. All connections to door or window jambs where the clip is not designated in the clip table / drawing are made with JC# clips (#= Girt Depth).

NOTE(S):
 1. WALL PANEL MUST BE FASTENED TO SUPPORTS WITH MBCI FASTENING PATTERN FP2.

| TRIM COLORS | |
|---|-------------------|
| EAVE TRIM = TBD | CORNER TRIM = TBD |
| BASE TRIM = TBD | GUTTER = TBD |
| DOOR TRIM = TBD | DOWNSPOUTS = TBD |
| RAKE TRIM = TBD | |
| * LINER TRIM = Liner panel color | |
| * SOFFIT TRIM = Soffit panel color | |
| * ONLY APPLICABLE IF LINER TRIM OR SOFFIT PANEL IS INDICATED ON BUILDING ORDER. | |

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 ENG: TVM DWN: BJC APPD: TVM

| REVISION HISTORY | |
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| REV. | DESCRIPTION |
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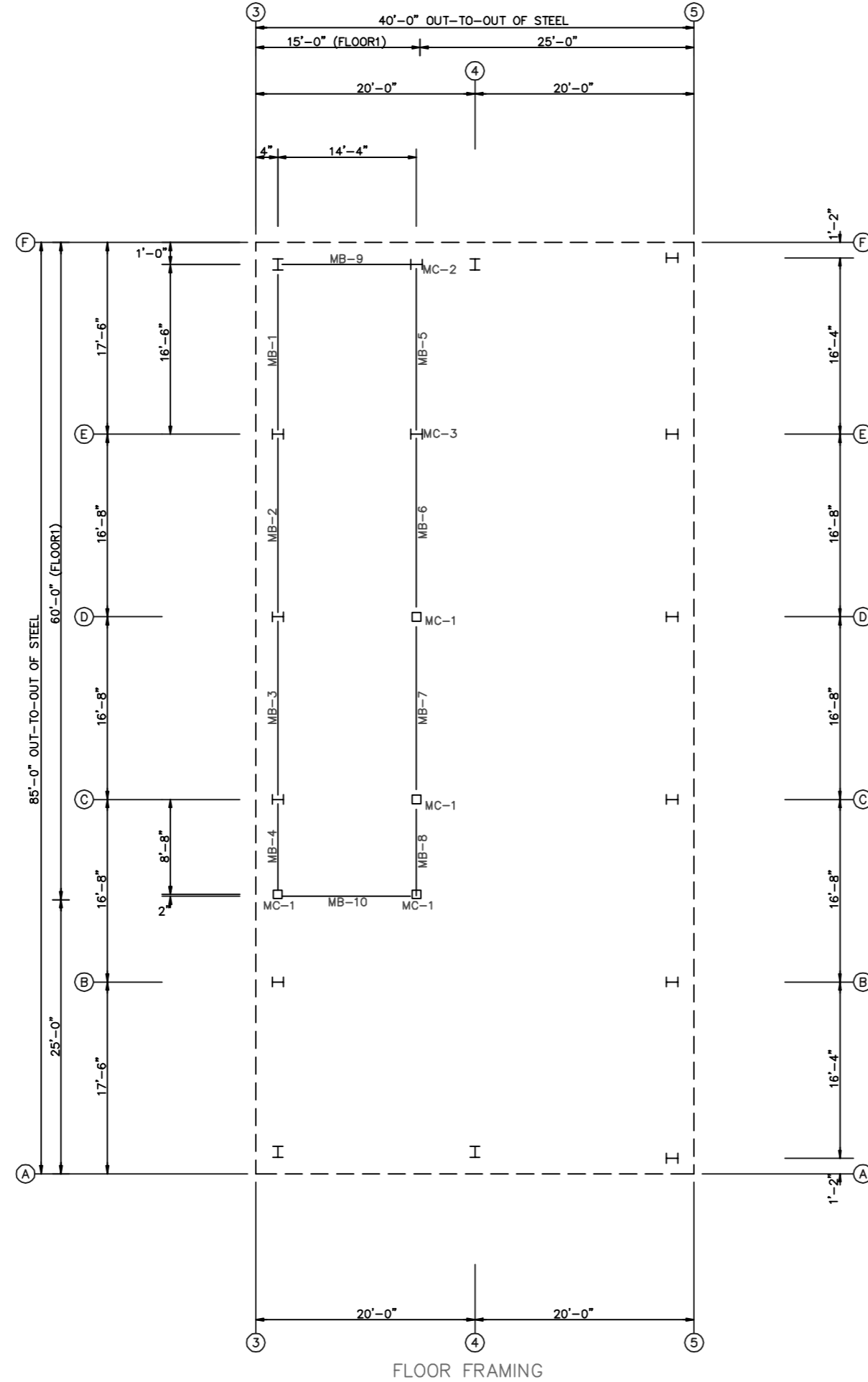
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FOR CONSTRUCTION: FINAL DRAWINGS.

STATE OF NEW HAMPSHIRE
 TRAVIS V. MICHAUD
 No. 15530
 LICENSED PROFESSIONAL ENGINEER
 5/14/18



FLOOR FRAMING

| BOLT TABLE | | | | |
|------------|------|------|------|--------|
| MEZZANINE | | | | |
| LOCATION | QUAN | TYPE | DIA | LENGTH |
| 12X35C10 | 4 | A325 | 1/2" | 1 1/2" |
| MB-9 | 2 | A325 | 1" | 2" |
| W14X22 | 3 | A325 | 3/4" | 1 3/4" |
| MC/MB | 4 | A325 | 1/2" | 1 1/2" |

| MEMBER TABLE | | | |
|--------------|-------|----------|--------------|
| Qty | Mark | Part | Length |
| 1 | MB-1 | W14X22 | 16'-0 15/16" |
| 1 | MB-2 | W14X22 | 16'-5 1/4" |
| 1 | MB-3 | W14X22 | 16'-5 1/4" |
| 1 | MB-4 | W14X22 | 9'-0 3/8" |
| 1 | MB-5 | W14X22 | 17'-4 1/4" |
| 1 | MB-6 | W14X22 | 16'-7 1/2" |
| 1 | MB-7 | W14X22 | 16'-7 1/2" |
| 1 | MB-8 | W14X22 | 9'-1 1/2" |
| 1 | MB-9 | W8X18 | 13'-11 7/8" |
| 1 | MB-10 | 12X35C10 | 12'-11 1/2" |
| 4 | MC-1 | HSS04044 | 10'-6 1/4" |
| 1 | MC-2 | W8X18 | 10'-6 1/4" |
| 1 | MC-3 | W8X18 | 10'-6 1/4" |



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85'-0" x 40'-0" x 20'-0"

DATE: 5/8/18 REVISION: 0

ENG: TVM DWN: BJC APPD: TVM

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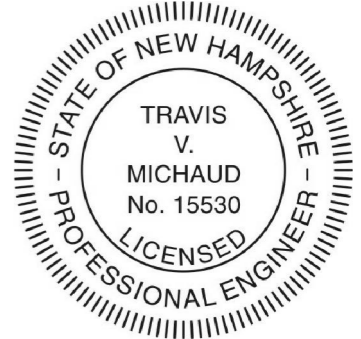
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REVISION HISTORY

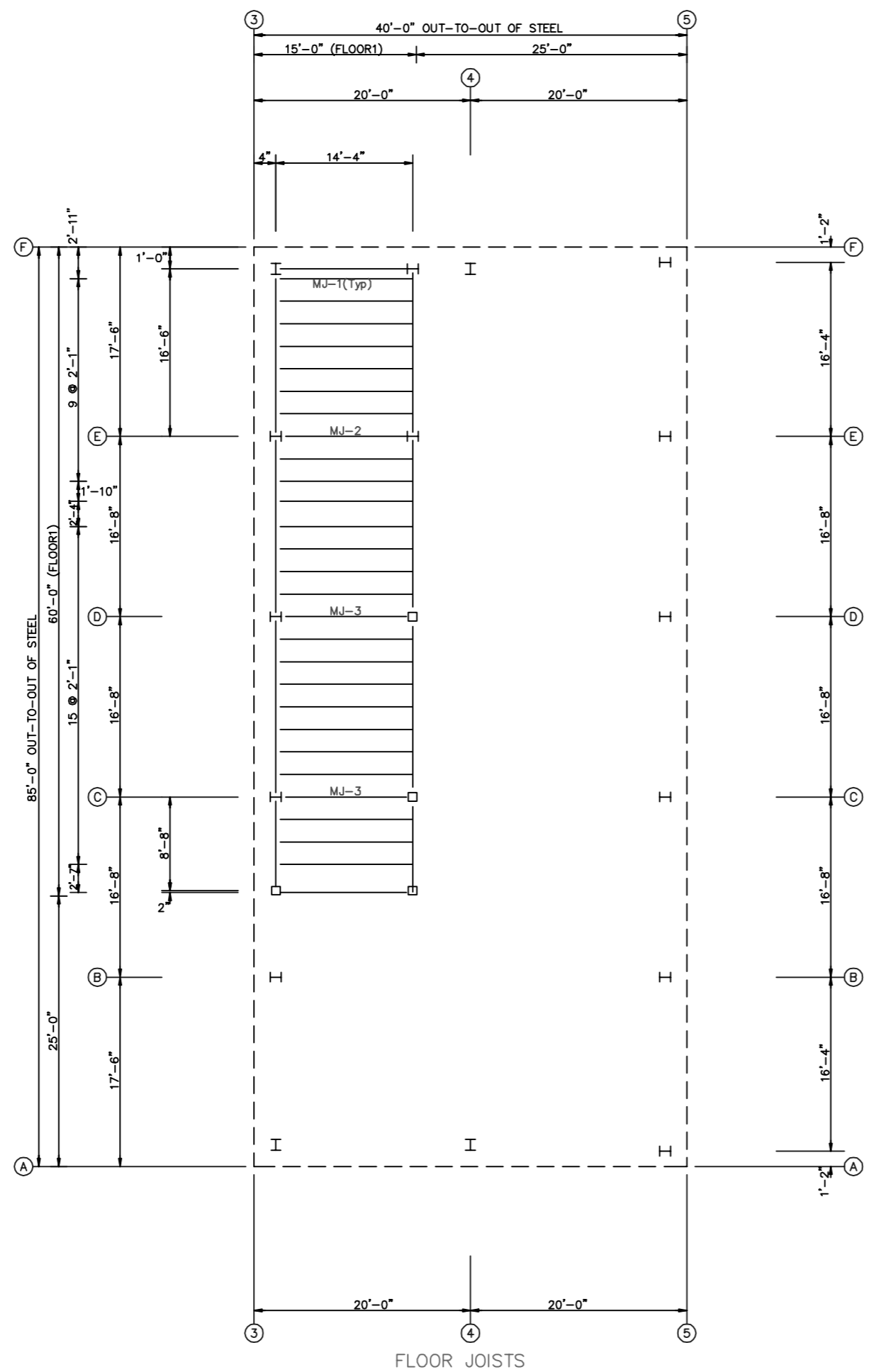
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 FOR CONSTRUCTION: FINAL DRAWINGS.



5/14/18



| | | | |
|------------|------|------|-------------|
| BOLT TABLE | | | |
| MEZZANINE | | | |
| LOCATION | QUAN | TYPE | DIA LENGTH |
| MJ/MB | 2 | A325 | 1/2" 1 1/4" |

| MEMBER TABLE | | | |
|--------------|------|----------|-------------|
| Qty | Mark | Part | Length |
| 24 | MJ-1 | 12X35C14 | 13'-2 1/4" |
| 1 | MJ-2 | 12X35C10 | 13'-8 1/4" |
| 2 | MJ-3 | 12X35C10 | 13'-8 5/16" |



400112 KUA OFFICE
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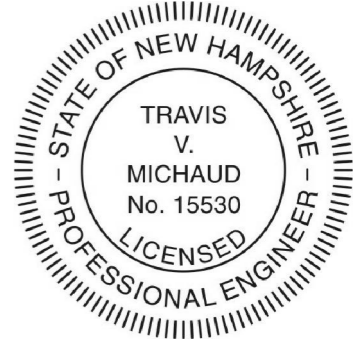
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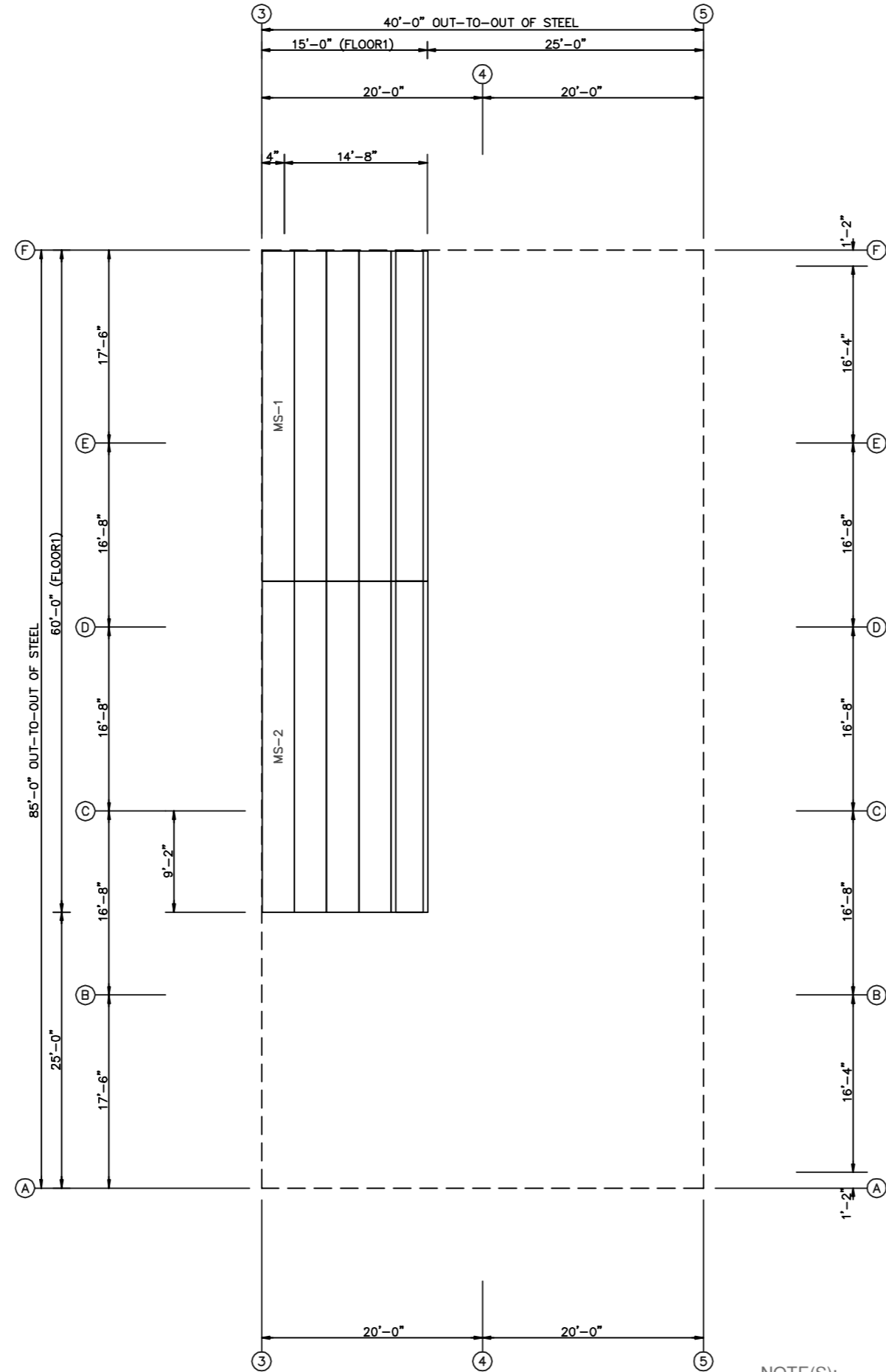
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| FLOOR PANEL TABLE | | | |
|-------------------|------|--------|------------|
| Qty | Mark | Part | Length |
| 6 | MS-1 | 06FD26 | 30'-2 1/2" |
| 6 | MS-2 | 06FD26 | 30'-3 3/4" |



FLOOR SHEETING & TRIM
 PANELS: 26 Ga. 6FD - Galvalume

NOTE(S):
 1. ALL DECK SHORING AND TEMPORARY SUPPORT DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE BUILDING PURCHASER/CUSTOMER.
 2. MINIMUM CONCRETE DECK REINFORCING IS 4x4-W2.9xW2.9 MESH INSTALLED PER NEW MILLENNIUM CRITERIA.



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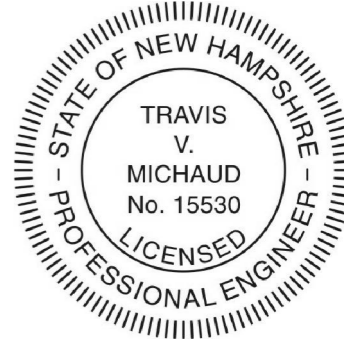
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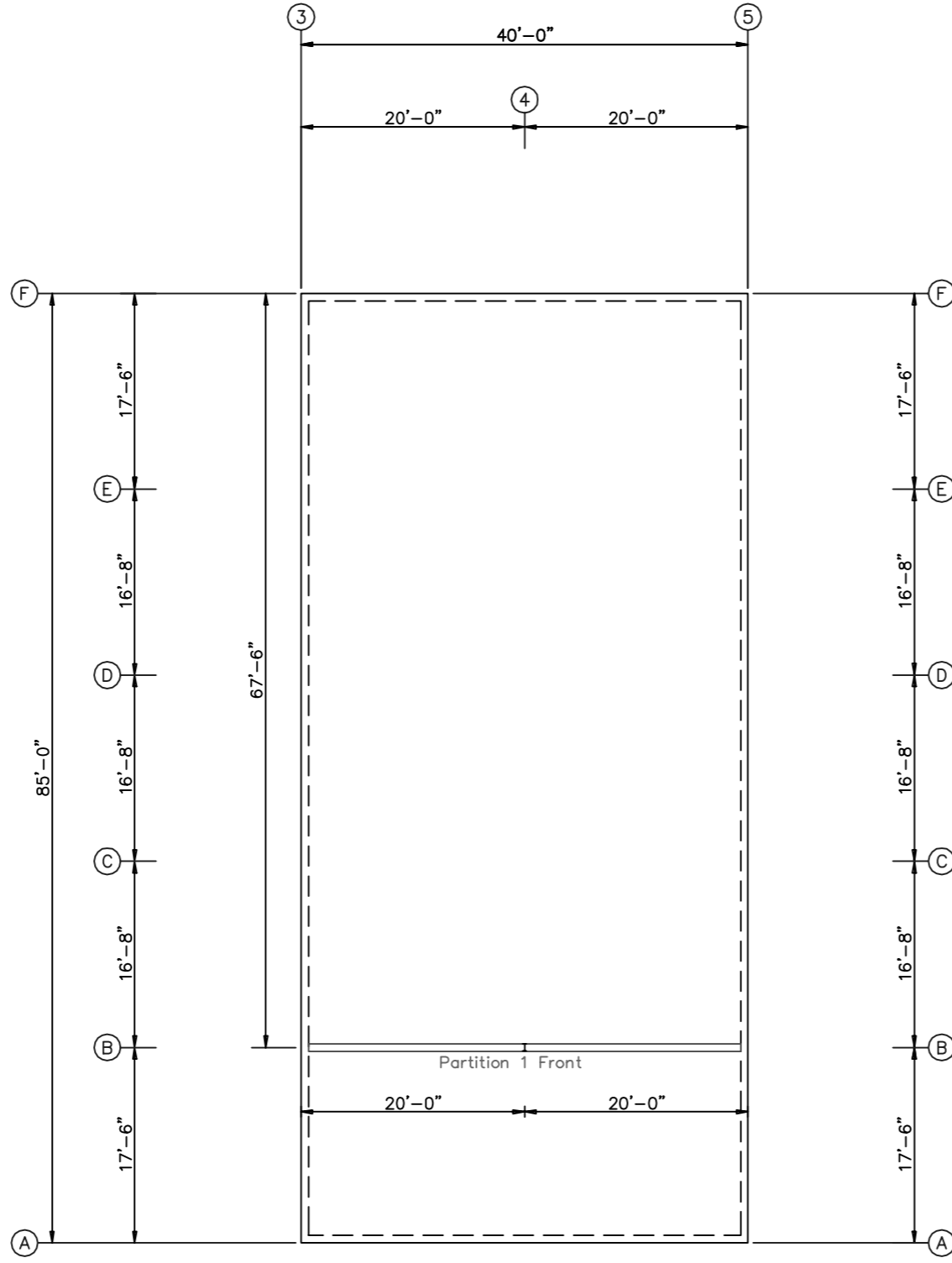
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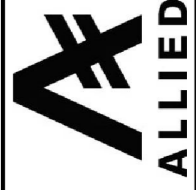


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PARTITION WALL PLAN

NOTES:
 1) 3/4" QUICK BOLTS NOT BY CORLE BUILDING SYSTEMS.



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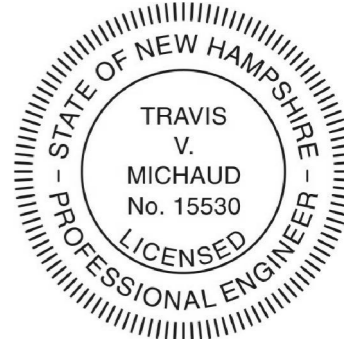
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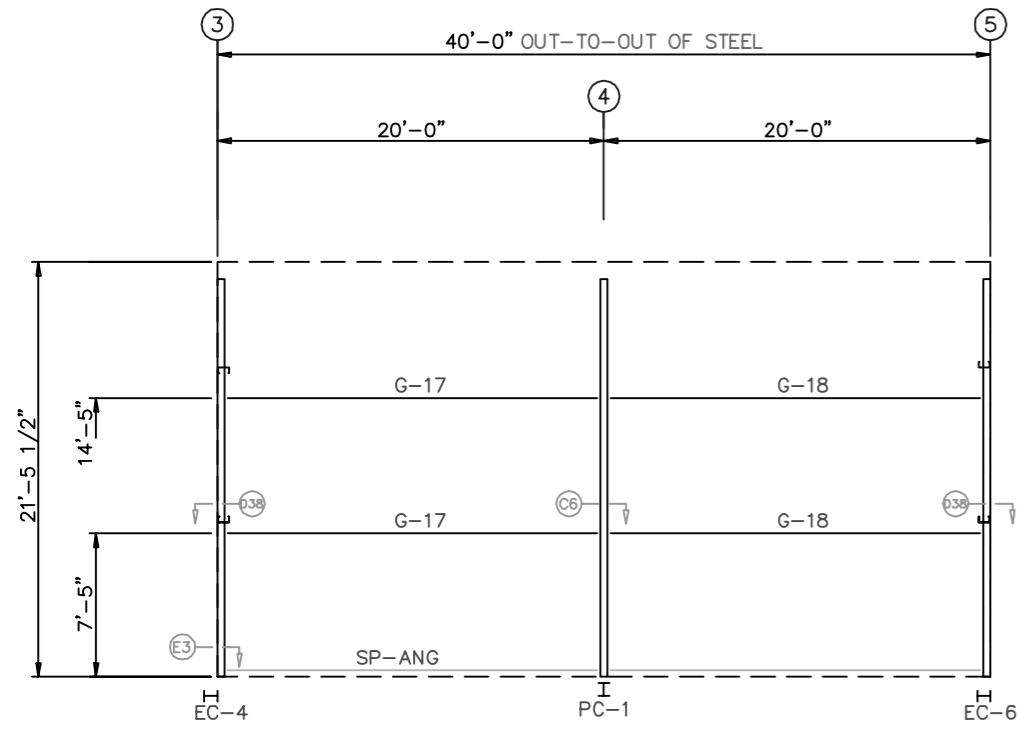
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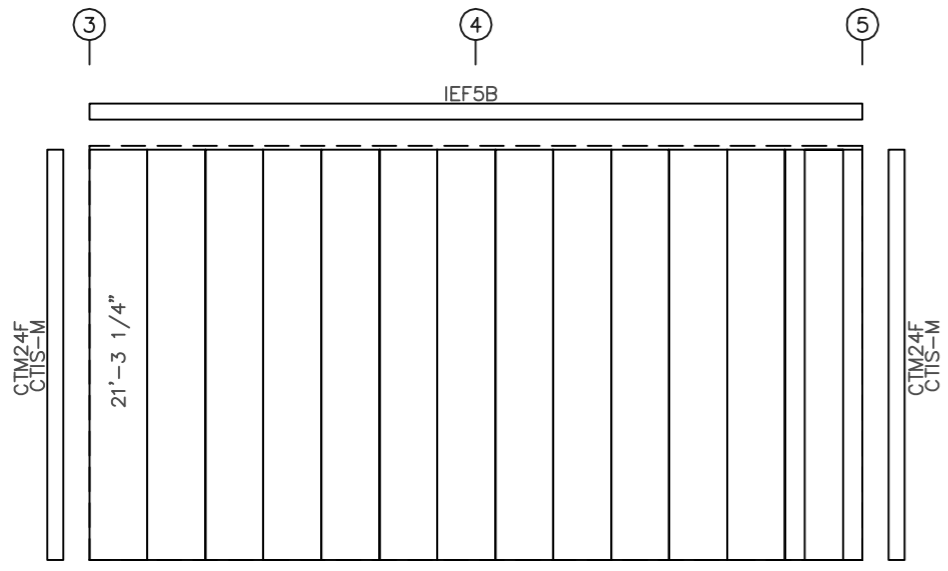
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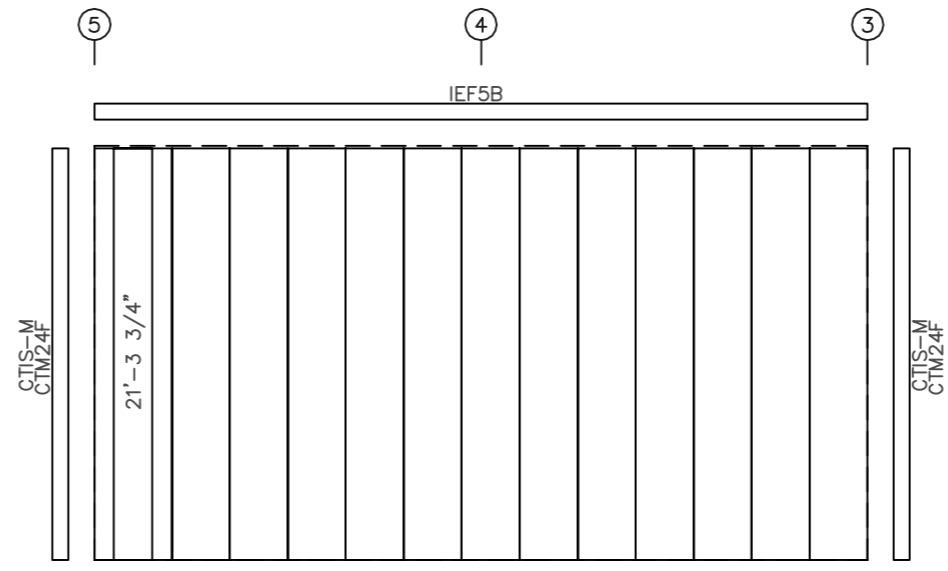
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PARTITION 1 FRAMING



PARTITION 1 FRONT SHEETING & TRIM
PANELS: 26 Ga. R - TBD



PARTITION 1 BACK SHEETING & TRIM
PANELS: 26 Ga. R - TBD

MEMBER TABLE
PARTITION 1

| QUAN | MARK | PART | LENGTH |
|------|------|---------|--------------|
| 1 | PC-1 | W8X10 | 16'-3 13/16" |
| 2 | G-17 | 8X25Z16 | 18'-9 3/8" |
| 2 | G-18 | 8X25Z16 | 19'-1 3/8" |



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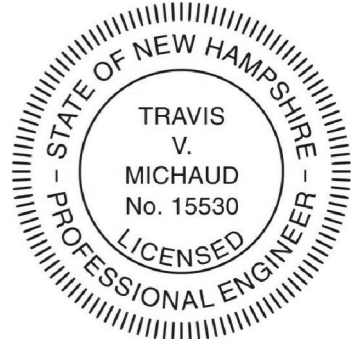
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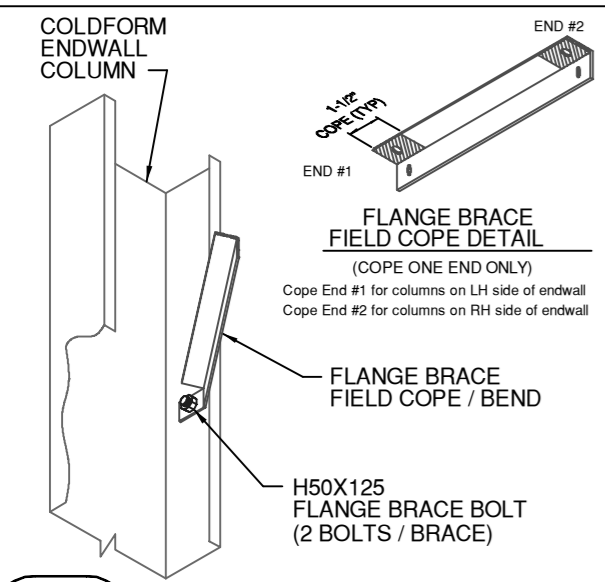
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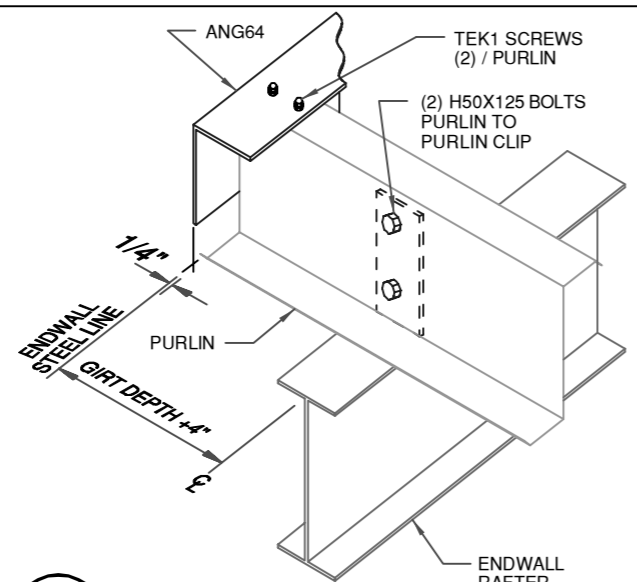
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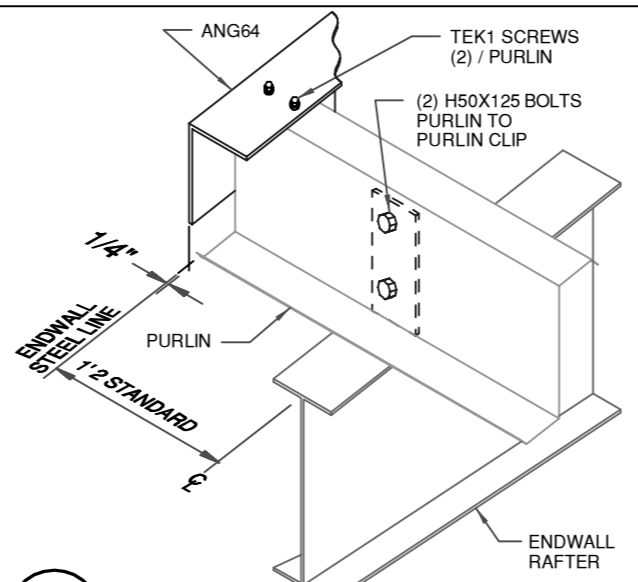
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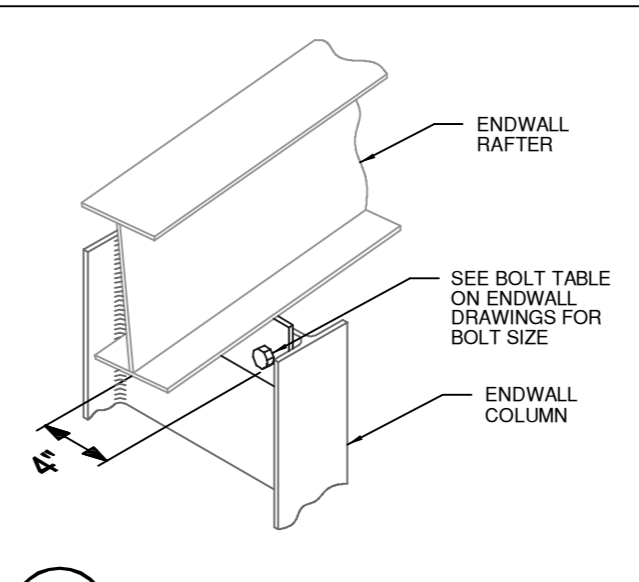
C7A FLANGE BRACE TO BYPASS COLDFORM ENDWALL COLUMN



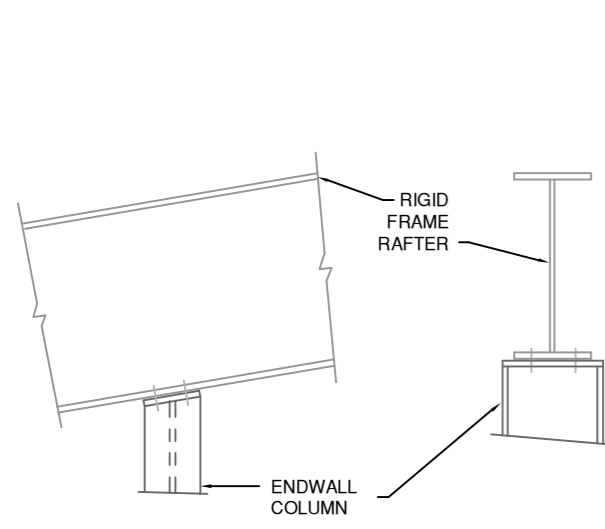
A7 PURLIN TO ENDWALL RAFTER CONNECTION



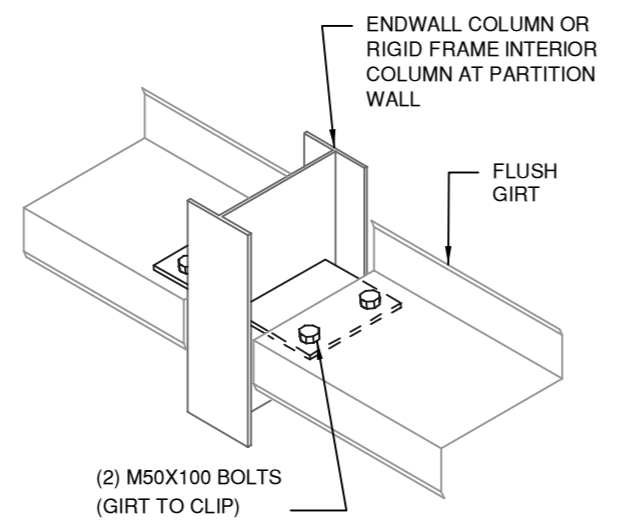
A10 PURLIN TO RIGID FRAME ENDWALL RAFTER CONNECTION



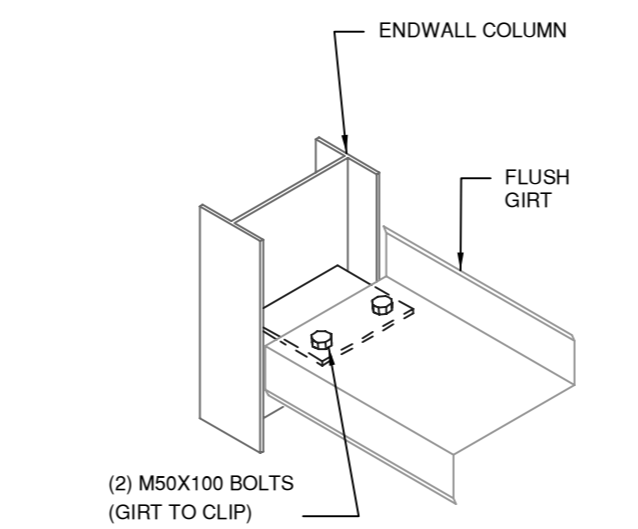
B6 ENDWALL COLUMN TO RAFTER CONNECTION



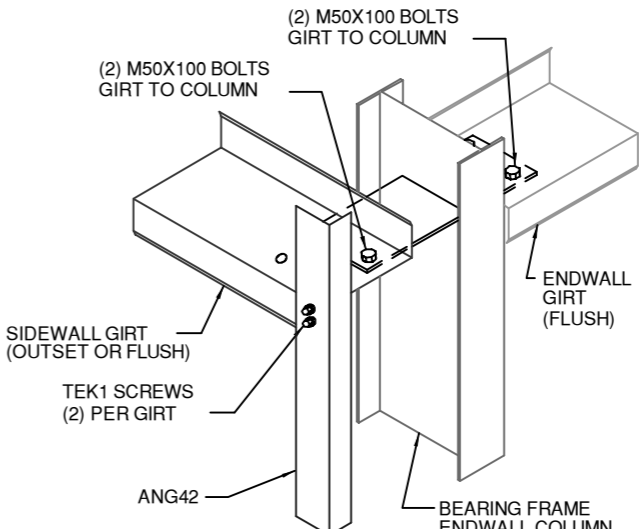
B20 COLUMN TO RIGID FRAME RAFTER



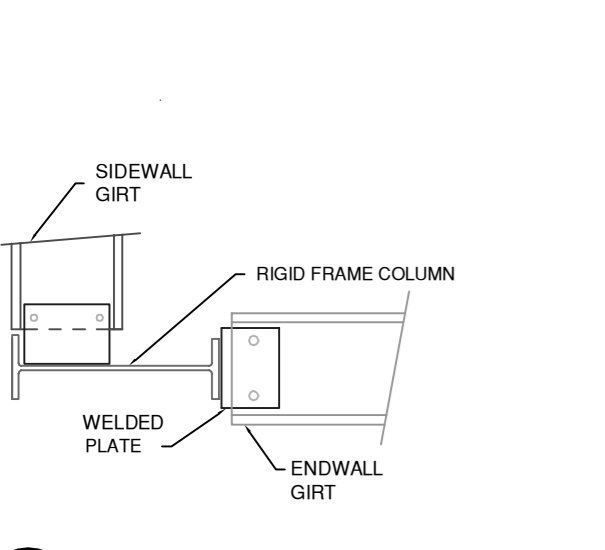
C6 FLUSH GIRT TO COLUMN



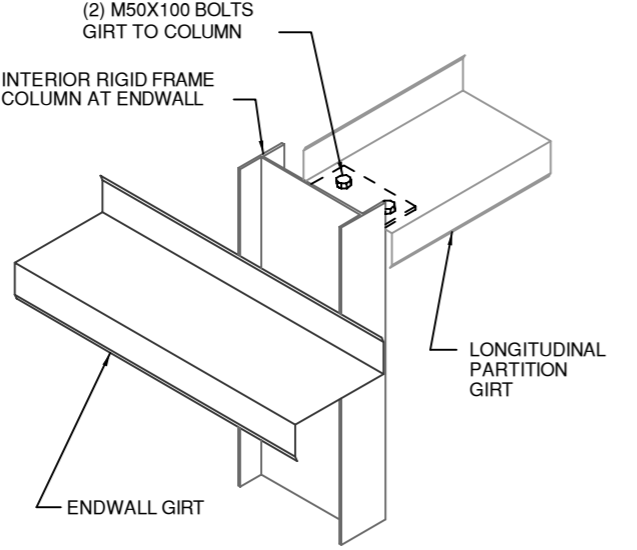
C15 FLUSH GIRT ENDING AT COLUMN



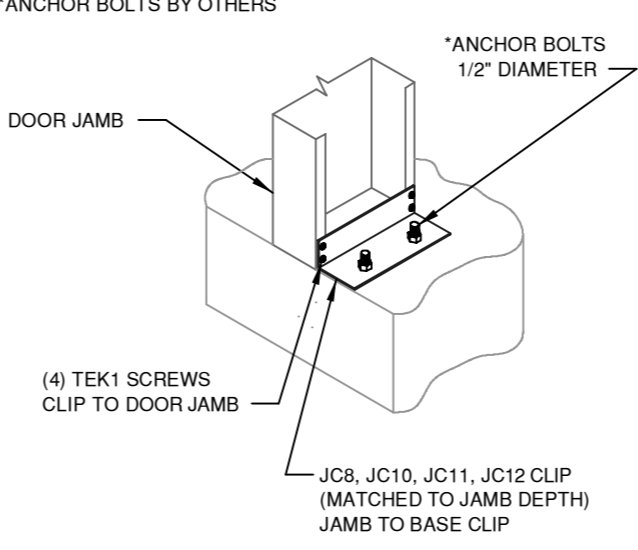
D6 WALL GIRTS TO FLUSH CORNER COLUMN



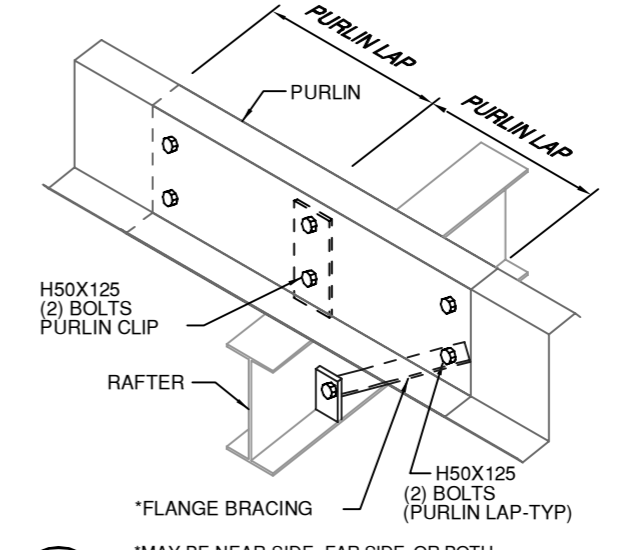
D13 CORNER COLUMN TO WALL GIRT



D38 LONGITUDINAL PARTITION GIRT TO RIGID FRAME COLUMN AT ENDWALL



E6 DOOR JAMB TO FOUNDATION



G2 ROOF PURLIN TO INTERIOR FRAME RAFTER



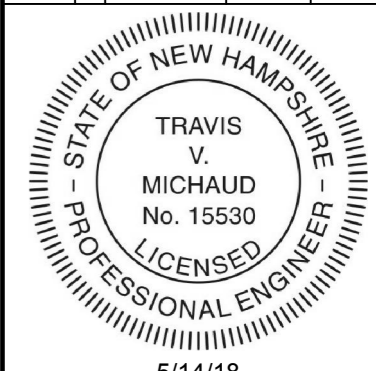
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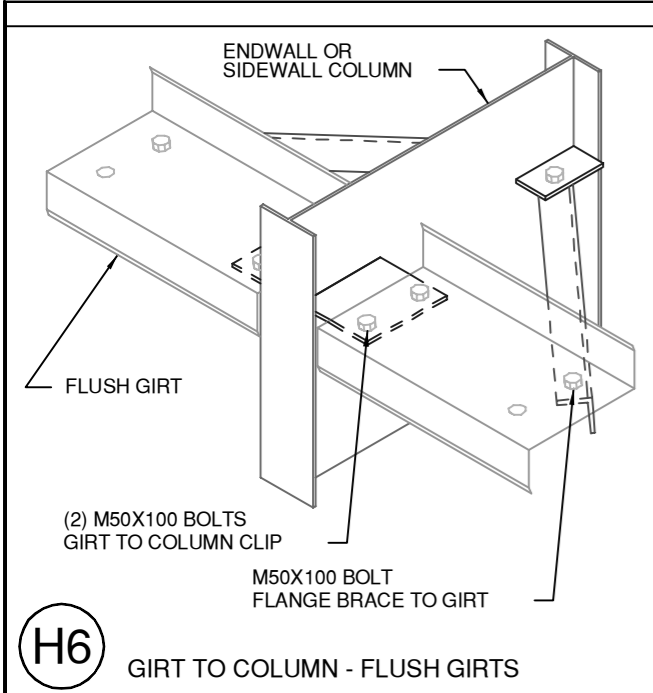
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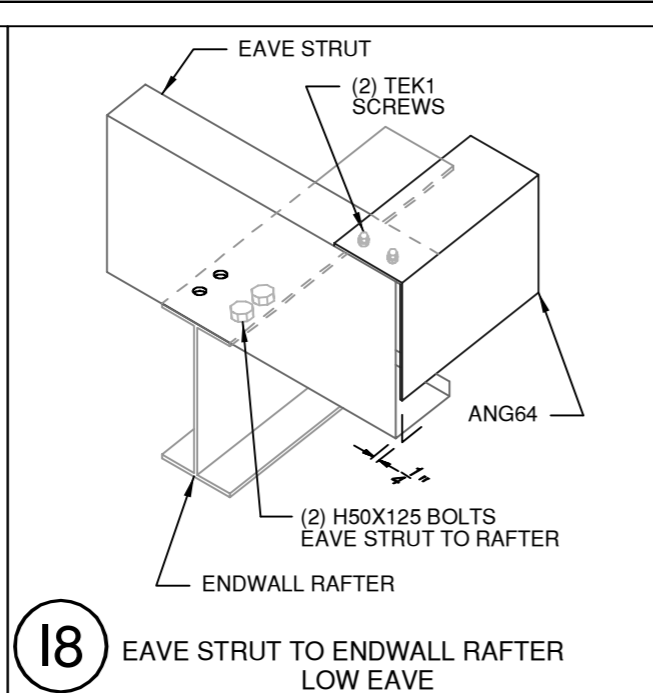
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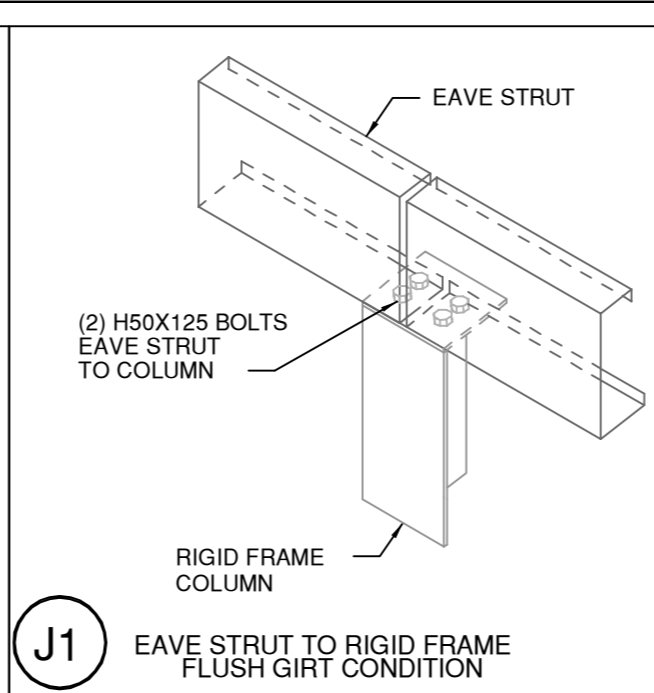




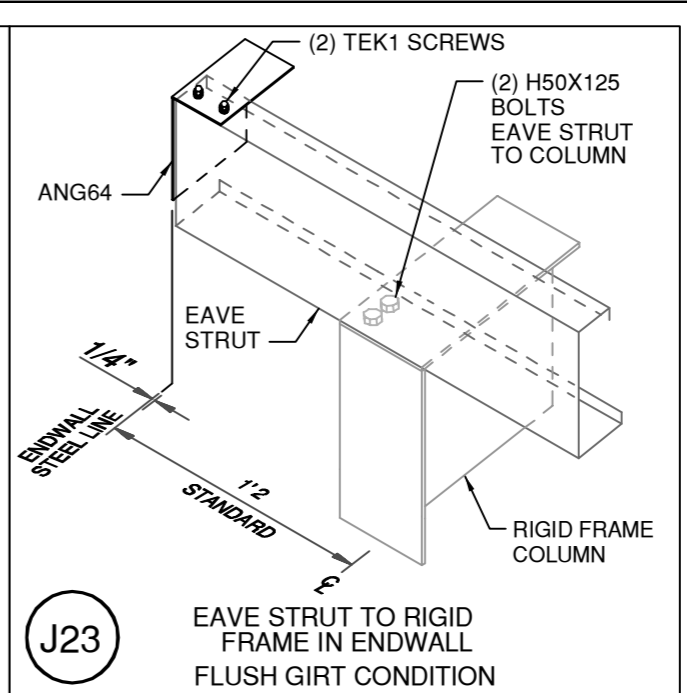
H6 GIRT TO COLUMN - FLUSH GIRTS



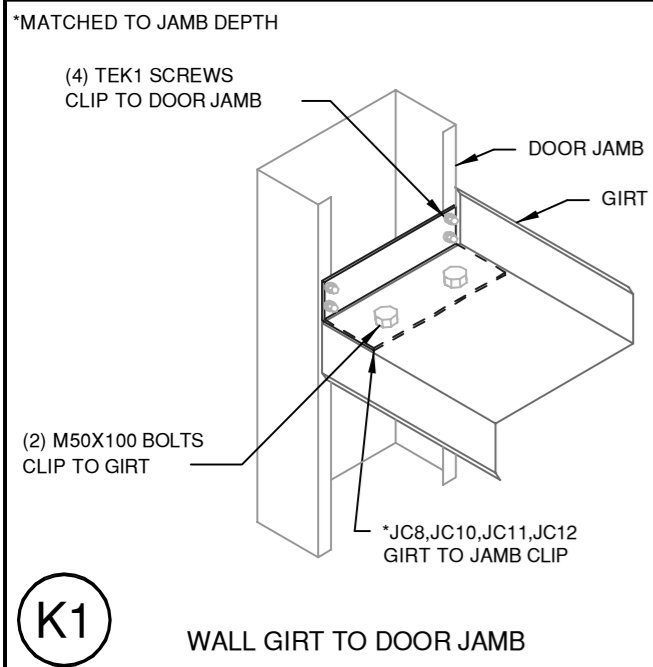
I8 EAVE STRUT TO ENDWALL RAFTER
LOW EAVE



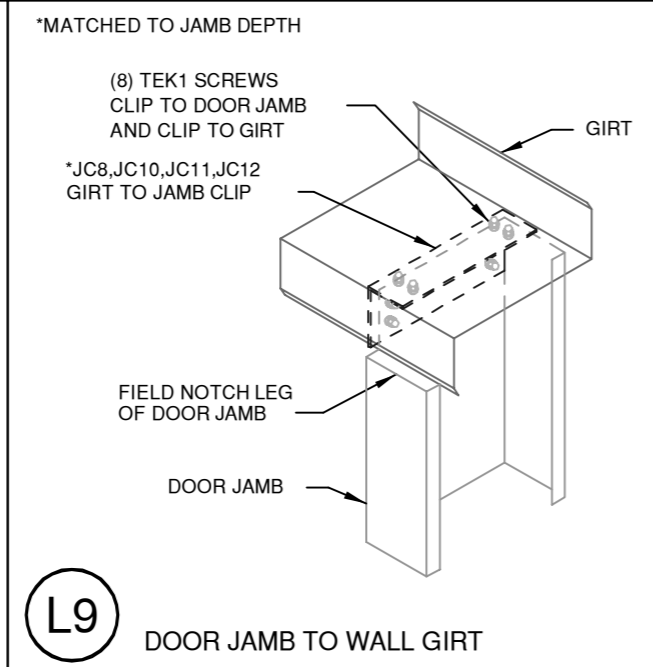
J1 EAVE STRUT TO RIGID FRAME
FLUSH GIRT CONDITION



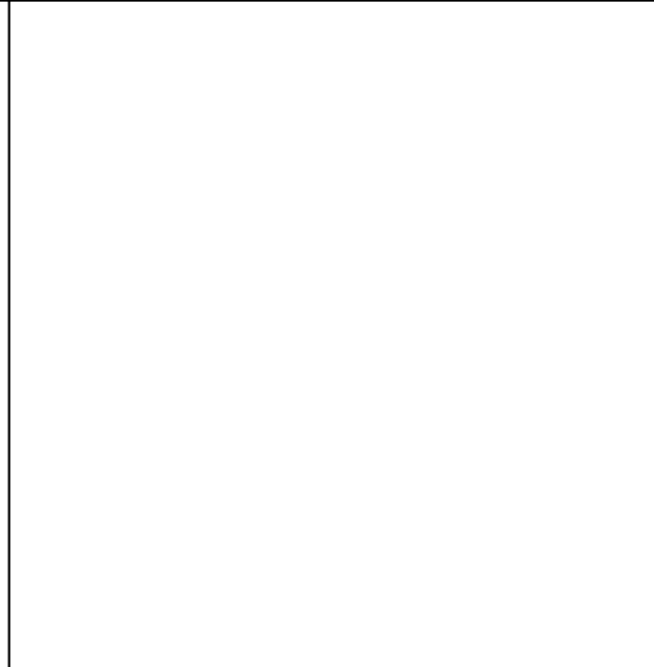
J23 EAVE STRUT TO RIGID FRAME IN ENDWALL
FLUSH GIRT CONDITION



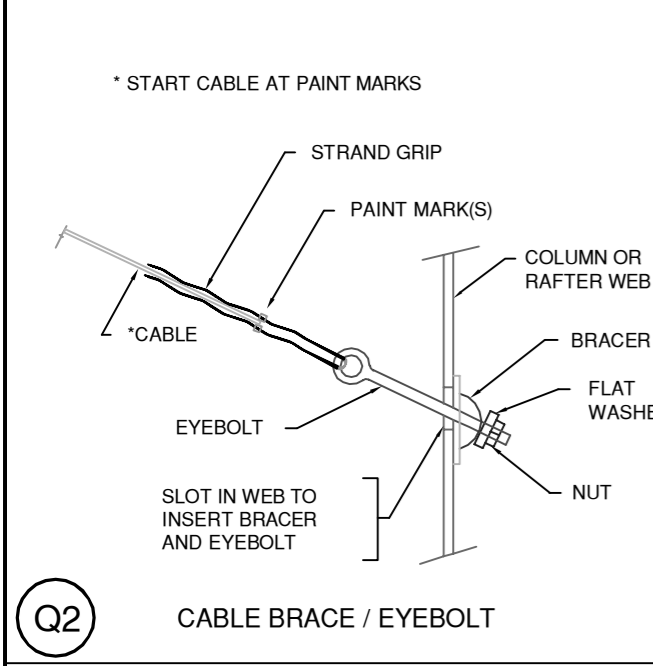
K1 WALL GIRT TO DOOR JAMB



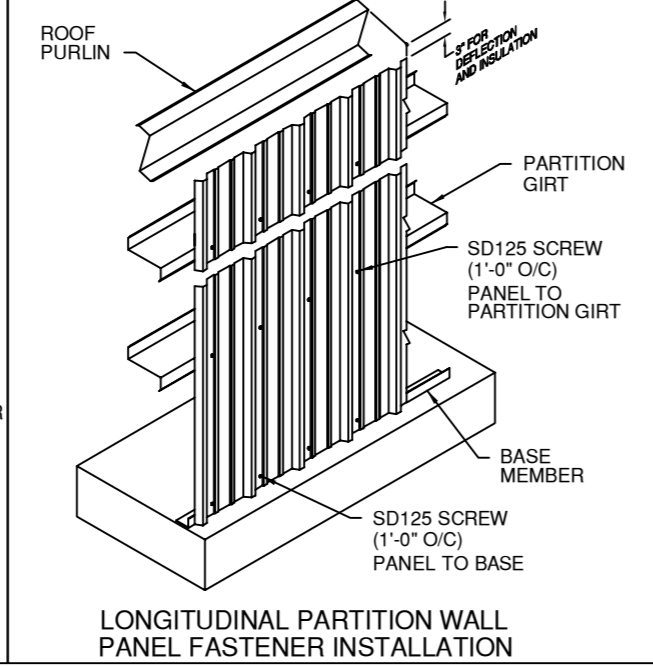
L9 DOOR JAMB TO WALL GIRT



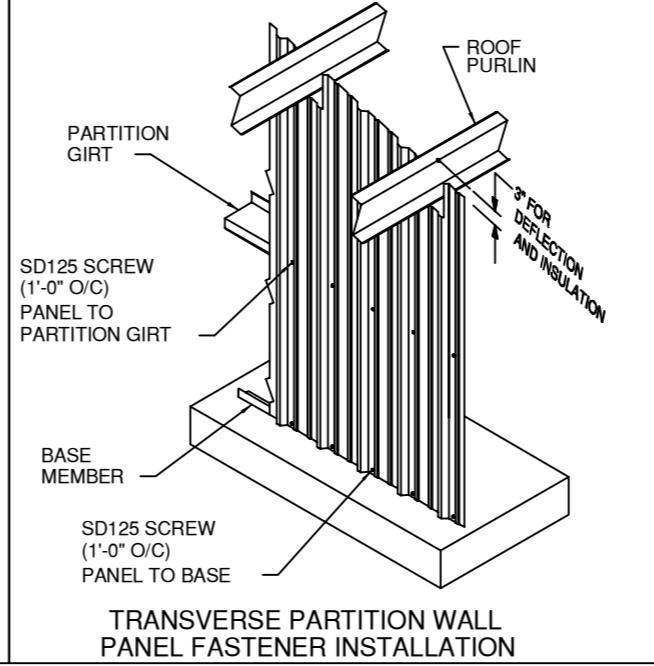
M1 DOOR HEADER TO DOOR JAMB



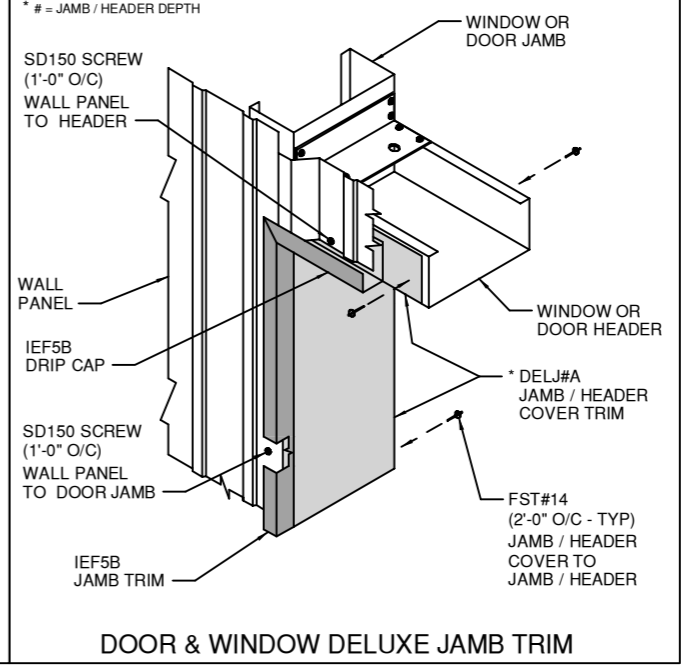
Q2 CABLE BRACE / EYEBOLT



LONGITUDINAL PARTITION WALL
PANEL FASTENER INSTALLATION



TRANSVERSE PARTITION WALL
PANEL FASTENER INSTALLATION



DOOR & WINDOW DELUXE JAMB TRIM



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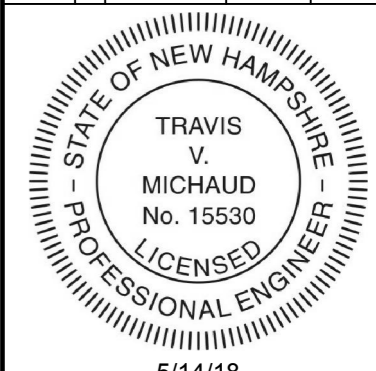
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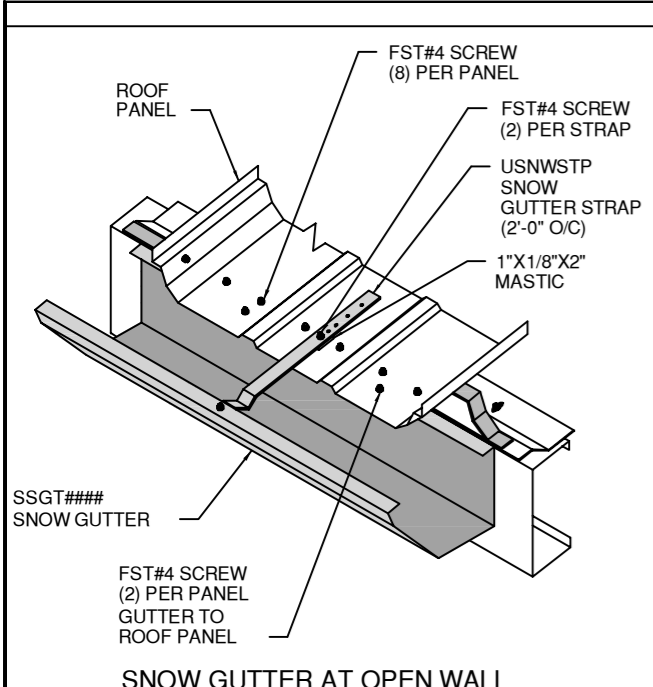
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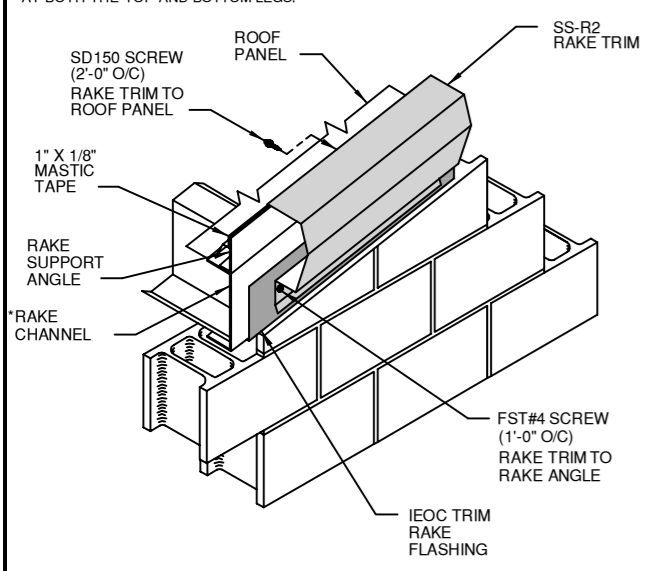
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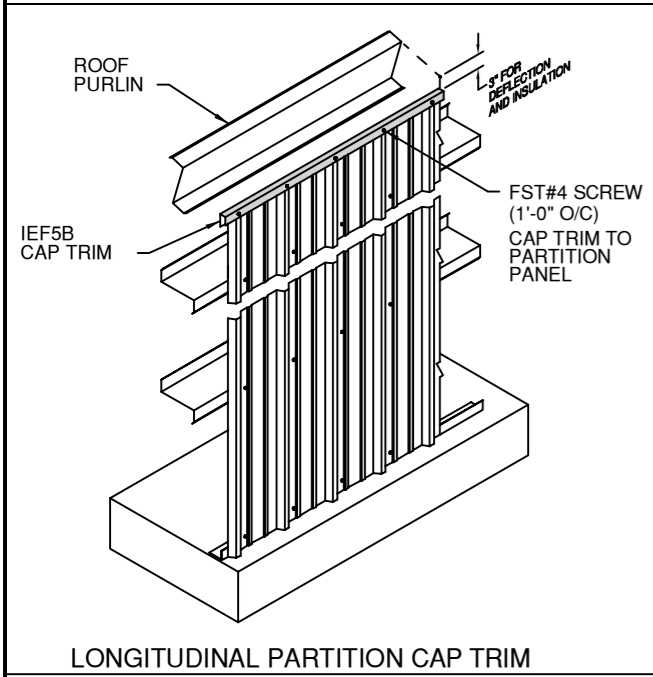


SNOW GUTTER AT OPEN WALL

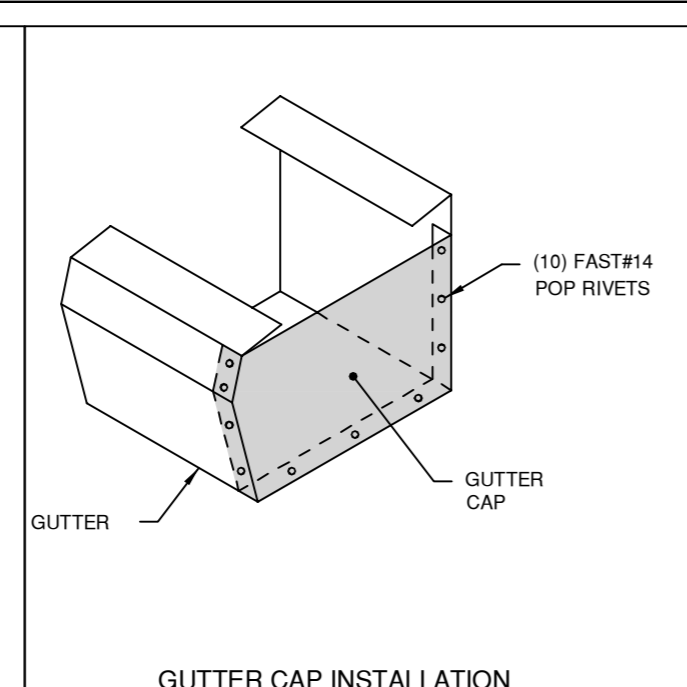
*ATTACH RAKE CHANNEL TO PURLINS WITH (2) TEK1 SCREWS AT BOTH THE TOP AND BOTTOM LEGS.



RAKE TRIM AT OPEN OR MASONRY WALL (ROOF SURFACES $\leq 90^\circ$ EAVE TO PEAK)

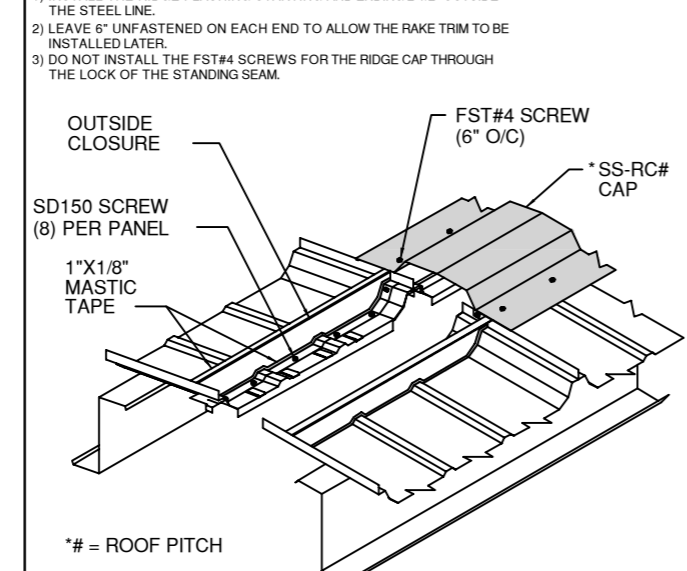


LONGITUDINAL PARTITION CAP TRIM

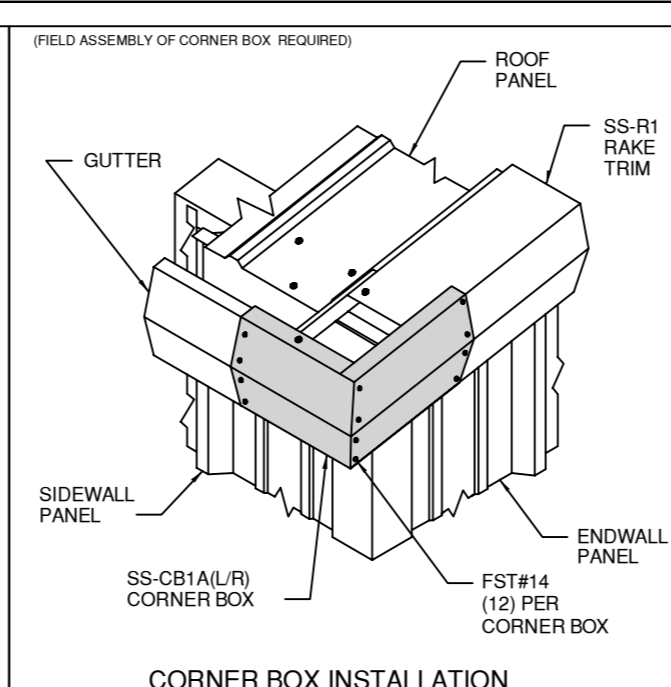


GUTTER CAP INSTALLATION

NOTE(S):
 1) INSTALL THE RIDGE FLASHING STARTING AND ENDING 2-1/2" OUTSIDE THE STEEL LINE.
 2) LEAVE 6" UNFASTENED ON EACH END TO ALLOW THE RAKE TRIM TO BE INSTALLED LATER.
 3) DO NOT INSTALL THE FST#4 SCREWS FOR THE RIDGE CAP THROUGH THE LOCK OF THE STANDING SEAM.

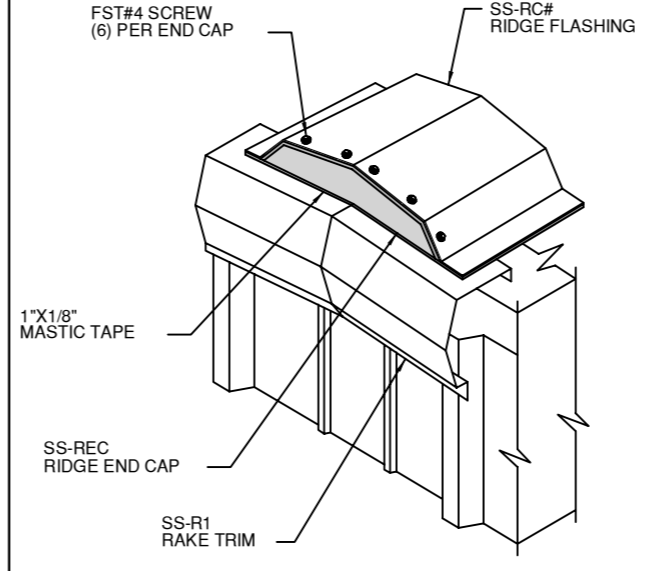


RIDGE CAP TRIM

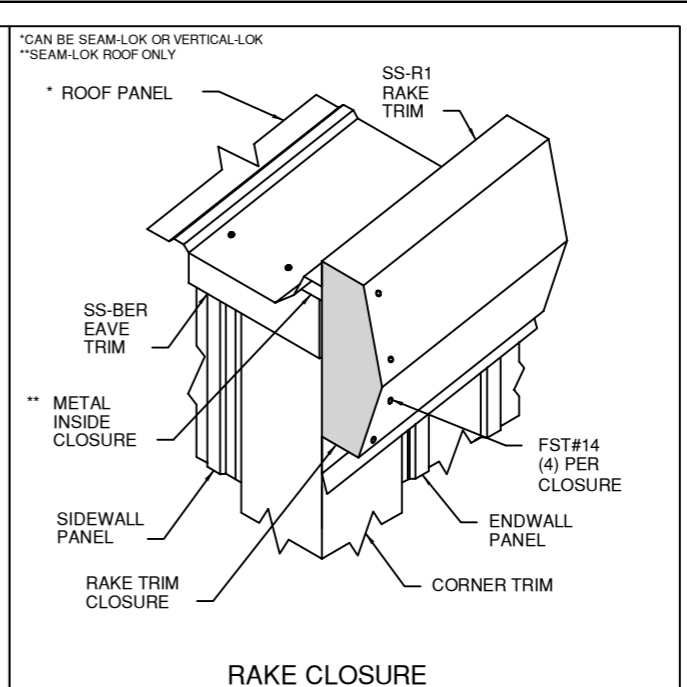


CORNER BOX INSTALLATION

(FIELD ASSEMBLY OF CORNER BOX REQUIRED)

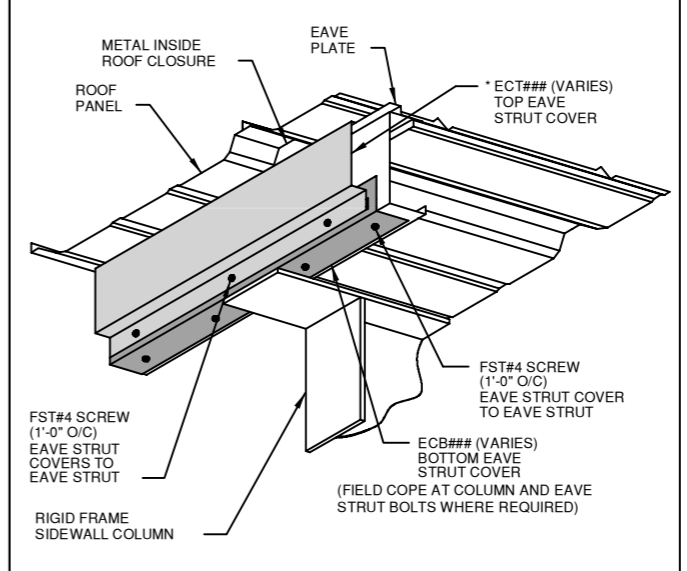


RIDGE CAP END CLOSURE



RAKE CLOSURE

*CAN BE SEAM-LOK OR VERTICAL-LOK
 **SEAM-LOK ROOF ONLY
 * ATTACH EAVE TRIM TO EAVE PLATE WITH (3) FST#14/PIECE UNTIL ROOF PANEL TO EAVE PLATE SCREWS ARE INSTALLED.



GABLE OR SINGLE SLOPE-LOW EAVE OPEN WALL OR MASONRY CONDITION



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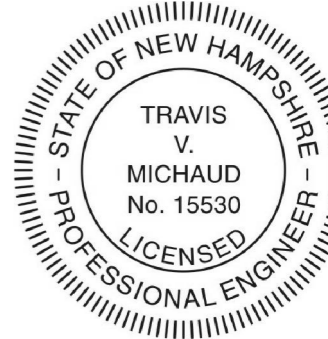
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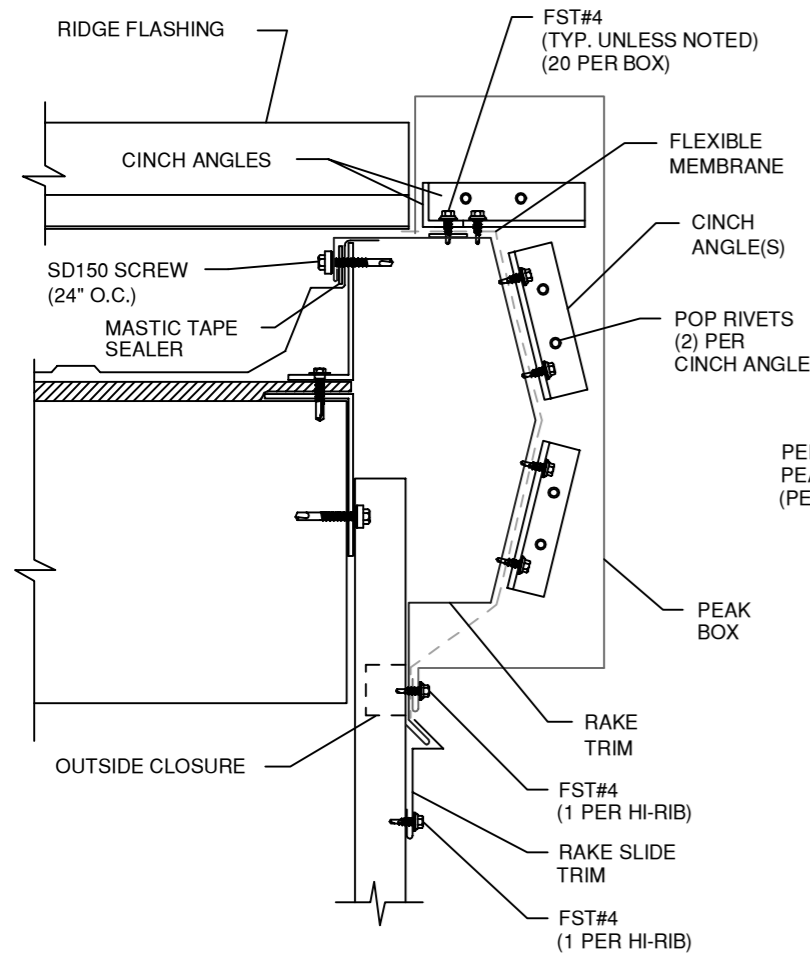
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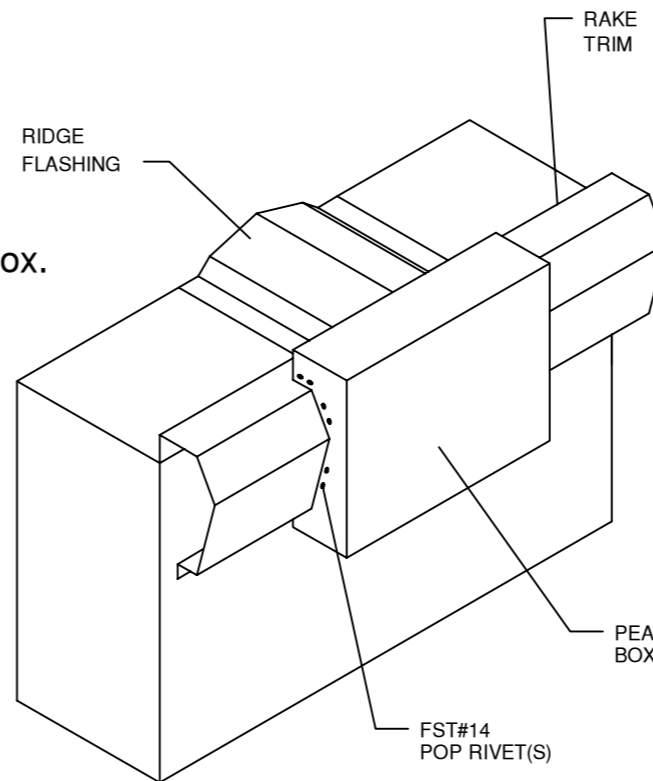
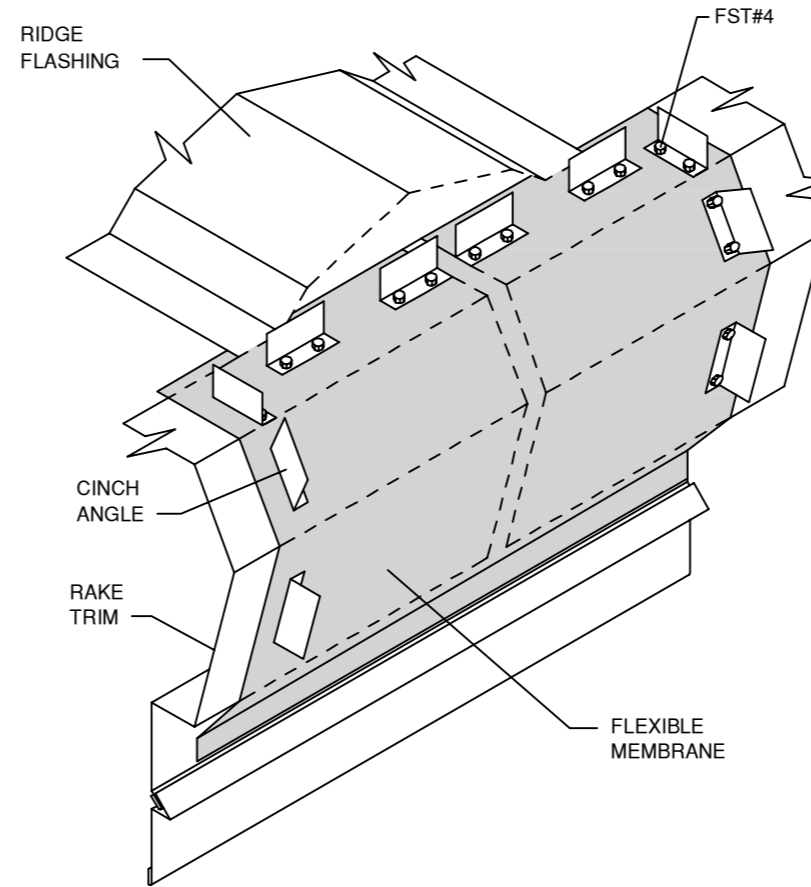
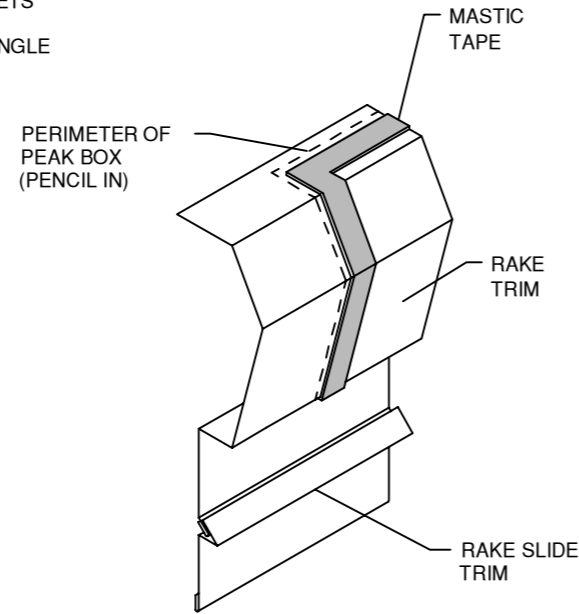
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TRIM DETAILS PEAK BOX INSTALLATION SEAM-LOK / VERTICAL-LOK



PEAK BOX INSTALLATION

1. Install rake trim on each side of ridge to within 1" of centerline of building.
2. Install ridge flashing so that it is on the top leg of rake trim, 2" back from outside edge.
3. Temporarily set peak box in place and mark perimeter of box on rake trim. Remove peak box.
4. Just inside mark, install tape sealer continuously across the top of rake trim, then down the face of the rake trim on both sides of ridge.
5. Place flexible membrane over tape sealer and hold in place with cinch angles. Cinch angles should be attached with fastener #4. To prevent leaks, flexible membrane should be tight against rake trim with no wrinkles at the sealed edges.
6. Hook top of peak box over cinch angles installed on top of rake trim and attach bottom of peak box to endwall with fastener #4.
7. Install FST#14 (pop rivets). (2) per cinch angle.
8. Seal the rake and ridge cap intersections with urethane sealant.



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