

GENERAL NOTES

A. GENERAL

- 1. ALL STRUCTURAL WORK SHALL CONFORM TO THE PROJECT SPECIFICATIONS, DRAWINGS, AND THE 2015 INTERNATIONAL BUILDING CODE.
2. CONTRACTOR SHALL COORDINATE STRUCTURAL WORK WITH RELATED TRADES AND WITH OTHER DESIGN DISCIPLINE REQUIREMENTS PRIOR TO MAKING SUBMITTALS.
3. REFER TO OTHER DESIGN DISCIPLINE DRAWINGS AND SPECIFICATIONS FOR ADDITIONAL INFORMATION REQUIRED FOR THE SUBMITTALS AND INSTALLATION OF STRUCTURES, INCLUDING BUT NOT LIMITED TO DIMENSIONS, ELEVATIONS, SLOPES, LOCATIONS OF OTHER SYSTEMS AND EQUIPMENT, OPENINGS, WALLS, STAIRS, FINISHES, COATINGS, AND OTHER NON-STRUCTURAL ITEMS.
4. DETAILS LABELED AS TYPICAL DETAILS ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED.
5. CONTRACTOR IS RESPONSIBLE FOR COORDINATION DETAILS AND ACCURACY OF THE WORK; FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS; FOR SELECTING FABRICATION PROCESSES, FOR TECHNIQUES OF ASSEMBLY IN ACCORDANCE WITH GENERAL CONDITIONS AND DIVISION 1 SPECIFICATION REQUIREMENTS; AND FOR PERFORMING ALL WORK IN A SAFE AND SECURE MANNER IN ACCORDANCE WITH GOVERNING JOB SAFETY STANDARDS.
6. CONTRACTOR SHALL VERIFY ALL CONDITIONS AT THE SITE, INCLUDING LOCATIONS OF ALL EXISTING STRUCTURES AND EXISTING UTILITIES ABOVE AND BELOW GROUND (AS ANY INFORMATION SHOWN IS APPROXIMATE AND NOT NECESSARILY COMPLETE.)
7. LOADS APPLIED DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS NOTED ON THE DRAWINGS OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTIONS AS DETERMINED BY THE CONTRACTOR.
8. SHORING, BRACING, PROTECTING, AND MAINTAINING THE INTEGRITY OF ANY EXISTING, ADJACENT, AND/OR ONGOING PARTIALLY COMPLETED STRUCTURES IS THE RESPONSIBILITY OF THE CONTRACTOR.

B. FOUNDATION RELATED EARTHWORK

- 1. RECOMMENDATIONS AND DESIGNS CONTAINED IN GEOTECHNICAL REPORTING AS FOLLOWS:
2. THE GEOTECHNICAL REPORTING CONTAINS SPECIFIC REQUIREMENTS PERTAINING TO GRUBBING, SITE, SUBFLOOR AND BEARING SURFACE PREPARATION AND PROTECTION, STRUCTURAL FILL AND COMPACTION REQUIREMENTS; GROUND WATER MANAGEMENT; ETC. THAT ARE NOT NECESSARILY SHOWN BY THE DRAWINGS AND SPECIFICATIONS.
3. A LICENSED GEOTECHNICAL ENGINEER SHALL INSPECT AND REPORT ON ALL NATIVE SUB-GRADES FOR SLABS-ON-GRADE AND FOUNDATION PREPARED SOIL SURFACES PRIOR TO THE PLACEMENT OF ANY BACKFILL, FILL, AND FOUNDATION STRUCTURAL ELEMENTS.
4. FOOTINGS, PILE CAPS, AND SLABS CAST DIRECTLY AGAINST THE EARTH SHALL BE SIDE-FORMED AS REQUIRED TO KEEP EARTH OUT OF THE CONCRETE.
5. UNLESS NOTED OTHERWISE, PLACE AND COMPACT BACKFILL IN EQUAL CONTINUOUS LAYERS NOT EXCEEDING A MAXIMUM OF 8" OF COMPACTED DEPTH FOR HAND-HELD COMPACTION EQUIPMENT AND A MAXIMUM OF 12" INCHES COMPACTED DEPTH FOR VIBRATORY ROLLERS.
6. AT EARTH RETAINING AND FOUNDATION WALLS, BACKFILL LIFTS TO NOT EXCEED 12 INCH DIFFERENCE IN ELEVATION UNTIL FINAL ELEVATION ARE REACHED ON BOTH SIDES OF THE WALL.
7. THE CONSTRUCTION CONSIDERATIONS IN THE GEOTECHNICAL REPORTING AND PROJECT SPECIFICATIONS SHALL APPLY TO THIS PROJECT, INCLUDING BUT NOT LIMITED TO PROOFROLLING SUBGRADES AT THE EXCAVATION AND/OR BEARING ELEVATIONS; REMOVING AND REPLACING LOOSE OR SOFT POCKETS, FILL SLOPE CONSTRUCTIONS, ETC.
8. BACKFILL REQUIREMENTS:
A. FILL WITHIN BUILDING ENVELOPE AND EXTENDING OUTWARD AT 1:1 SLOPE TO ACCEPTABLE NATIVE SOIL CONDITIONS:
a. MATERIAL: "SAND-GRAVEL"; "GRANULAR"; "CRUSHED STONE" WITH GEOTEXTILE WRAP (SEE SECTIONS)
b. COMPACTION: 95% MODIFIED PROCTOR
B. BACKFILL DIRECTLY BELOW INTERIOR SLABS-ON-GRADE ASSEMBLIES (12 INCHES UNLESS NOTED OTHERWISE):
a. "CRUSHED STONE" WITHOUT GEOTEXTILE WRAP
b. COMPACTION: 95% MODIFIED PROCTOR
C. BACKFILL BELOW PAVEMENT, WALKS, ENTRY SLABS IN VICINITY OF BUILDING:
a. MATERIAL: "SAND-GRAVEL"; "GRANULAR"; "CRUSHED STONE" (SEE SECTIONS, LAND ARCH AND CIVIL)
b. COMPACTION: 95% MODIFIED PROCTOR
D. BACKFILL BEHIND RETAINING WALLS AND BASEMENT WALLS, OUTSIDE BUILDING ENVELOPE AND UNDER PAVEMENT, WALKS, ENTRY SLABS:
a. MATERIAL: "GRANULAR BACKFILL"
b. COMPACTION: 95% MODIFIED PROCTOR
E. BACKFILL ALONG EXTERIOR OF BUILDING AGAINST WALLS AND NOT UNDER PAVEMENT, WALKS, ENTRY SLABS:
a. MATERIAL: "SUITABLE NATIVE SOIL" COVERED BY 2 FEET DEEP BY 4 FEET WIDTH OF "LESS PERMEABLE FILL"
b. COMPACTION: 92% MODIFIED PROCTOR
9. BACKFILL MATERIALS: RECYCLED CONCRETE AGGREGATE TO BE USED IN WHOLE OR BLENDED WITH OTHER AGGREGATES TO ACHIEVE GRADATIONS BELOW.
A. "SAND-GRAVEL":
% BY WEIGHT PASSING SIEVES
- 4 INCH 100
- 1/2 INCH 50-85
- No. 4 45-75
- No. 10 10-35
- No. 20 0-8
B. "GRANULAR":
% BY WEIGHT PASSING SIEVES
- No. 4 100
- No. 10 30-95
- No. 40 10-60
- No. 200 0-8
C. "CRUSHED STONE" WITH GEOTEXTILE FABRIC:
% WEIGHT BY PASSING SIEVES
- 1 INCH 100
- 3/4 INCH 90-100
- 3/8 INCH 0-55
- No. 4 0-10
- No. 8 0-5
D. "SUITABLE NATIVE SOIL": ON SITE SAND OR GRAVEL REASONABLY FREE OF LOAM, SILT, CLAY, OR ORGANIC MATTER.
E. "LESS PERMEABLE FILL" GLACIAL TILL (SEE GEOTECHNICAL REPORT)
F. "RECYCLED CONCRETE AGGREGATE" STOCKPILED ON SITE FROM DECONSTRUCTION PROJECT.
10. GEOTEXTILE FABRIC: NON-WOVEN WITH 12 LAPPED SEAMS SEE GEOTECHNICAL REPORTING FOR USE AND METHOD.
A. GRAB STRENGTH OF 80 POUNDS MINIMUM MEETING ASTM D4632
B. PUNCTURE STRENGTH OF 25 POUNDS MINIMUM MEETING ASTM D4833
C. TRAPEZOID TEAR OF 25 POUNDS MINIMUM MEETING ASTM D4533
D. APPARENT OPENING SIZE OF NO. 70-100 (US SIEVE) MEETING ASTM D4751
11. INSULATION AT EXTERIOR SLABS AND WALKS (NOT PAVEMENTS): EXTRUDED POLYSTYRENE, STRENGTH OF 40 PSI (UNO)AND RATED FOR UNDERSLAB/UNDERGROUND USE. STAGGER AND DO NOT TAPE BOARD JOINTS.

C. CONCRETE

- 1. CODES AND STANDARDS: COMPLY WITH THE PROVISIONS OF THE LATEST EDITIONS OF:
A. ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
B. ACI 301 "SPECIFICATIONS FOR STRUCTURAL CONCRETE"
C. ACI 304 "GUIDE FOR MIXING, TRANSPORTING AND PLACING CONCRETE"
D. ACI 305 "HOT WEATHER CONCRETING"
E. ACI 306 "STANDARD SPECIFICATION FOR COLD WEATHER CONCRETING"
F. ACI 308 "STANDARD PRACTICE FOR CURING CONCRETE"
2. CONCRETE TESTING: THE CONTRACTOR SHALL PREPARE A SET OF 4 CYLINDERS/TEST SET TO BE TESTED AT AN INDEPENDENT LABORATORY.
3. SUBMIT MIX DESIGN AND EITHER TRIAL MIX DESIGNS OR HISTORIC FIELD DATA FOR APPROVAL IN ACCORDANCE WITH ACI 318, CHAPTER 5, INCLUDE TECHNICAL DATA SHEETS, GRADATIONS, AND MATERIAL VERIFICATIONS ON ALL COMPONENTS.
4. COMPRESSIVE MIXTURES ARE NORMAL WEIGHT CONCRETE AS DELINEATED IN TABLE BELOW; SEE 03 3000 & NOTES BELOW FOR ADDITIONAL INFORMATION.
5. MAXIMUM AGGREGATE SIZE IN ACCORDANCE WITH ACI 301; CLEARLY NOTE LOCATION WHERE AGGREGATES GREATER THAN 1/2" MAXIMUM SIZE ARE PROPOSED FOR USE.
6. NO CHLORIDE OR OTHER UNAUTHORIZED ADMIXTURES SHALL BE USED.
7. WHEN AMBIENT TEMPERATURE IS BELOW 40 DEGREES FAHRENHEIT OR MORE THAN 90 DEGREES FAHRENHEIT PLACE AND PROTECT CONCRETE IN ACCORDANCE WITH ACI STANDARDS LISTED ABOVE.
8. CONCRETE PLACEMENT MAY REQUIRE ADJUSTMENT OF REINFORCEMENT, EMBEDDED ITEMS OR ANCHOR BOLTS.
9. COMPLY WITH ACI CODES AND PLACE CONCRETE IN A CONTINUOUS OPERATION WITHIN PLANNED JOINTS OR SECTIONS.
10. CURING: COVER OR WET CURE ALL ELEMENTS.
11. SEE 03 3000 FOR SURFACE FINISHES.
12. PROVIDE CONTROL AND CONSTRUCTION JOINTS BY DETAIL AND SPECIFICATION REQUIREMENTS.
A. SLABS SAW-CUT CONTROL JOINTS AS SOON AS CONCRETE HAS HARDENED ENOUGH TO WALK ON SURFACE WITHOUT DAMAGING CONCRETE AND NO MORE THAN 4 HOURS AFTER FINAL TROWEL.
B. WALLS CONTROL JOINTS: NOT EXCEEDING 20 FEET AND AT EACH INTEGRAL PILASTER.
13. HEADED STUD ANCHORS, DEFORMED BAR ANCHORS (DBA'S), AND OTHER EMBEDDED ITEMS AS SPECIFIED FOR STRUCTURAL STEEL.
14. CONCRETE MIXES AS SCHEDULED AND:
A. SLUMP: 3-5" BEFORE ADDITION OF WATER REDUCER, 6-8" AFTER ADDITION OF WATER REDUCER
B. ALL CONCRETE NORMALWEIGHT

D. CONCRETE REINFORCEMENT

- 1. SHOP DRAWINGS SHALL BE PROVIDED PRIOR TO START OF CONCRETE PLACING AND BE IN ACCORDANCE WITH:
A. ACI 301
B. ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT"
C. ACI SP-86 "ACI DETAILING MANUAL"
D. CRSI MSP "MANUAL FOR STANDARD PRACTICE"
E. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
2. REINFORCING STEEL SHALL BE DEFORMED BARS CONFORMING TO ASTM A615 GRADE 60, STEEL BARS PER ASTM A305, UNLESS NOTED OTHERWISE.
3. PROVIDE EPOXY-COATED BARS WHERE SHOWN MEETING ASTM A775 AND USING EPOXY COATED SUPPORTS, COATED WIRE, AND EPOXY COATING FOR REPAIR OF SURFACE PRIOR TO POURING.
4. WHERE SPECIFICALLY SHOWN ON THE DRAWINGS, WELD REINFORCING BARS IN ACCORDANCE WITH AWS D1.4 PRE-QUALIFIED JOINT, ELECTRODE E90 LOW HYDROGEN) AND PROCESS REQUIREMENTS INCLUDING COORDINATED WITH MILL CERTIFIED CARBON EQUIVALENT.
5. FIELD BENDING OR REINFORCEMENT SHALL CONFORM TO ACI 301, INCLUDING PRE-HEAT REQUIREMENTS.
6. WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A185 WITH A MINIMUM ULTIMATE TENSILE STRENGTH OF 70,000 PSI.
7. PROVIDE MINIMUM CONCRETE COVER TO REINFORCEMENT AS FOLLOWS, UNLESS OTHERWISE NOTED:
A. BOTTOM OF FOOTINGS, GRADE BEAMS, AND SLABS-ON-GRADE: 3"
B. SIDES OF FOOTINGS AND GRADE BEAMS: 2"
C. FOUNDATION WALLS, FROST WALLS, RETAINING WALLS, PIT WALLS: 2"
D. EXTERIOR WALLS (EXPOSED TO WEATHER): 2"
E. FACES OF WALLS OTHER THAN THOSE NOTED ABOVE: 3/4"
F. FOUNDATION PIERS: 2" TO TIES
G. ALL FACES OF BEAMS AND COLUMNS: 1-1/2" TO TIES
H. TOP AND BOTTOM OF ELEVATED SLABS: 3/4"
I. TOPPING SLAB: 3/4"
J. SLAB-ON-DECK: 3/4" FROM DECK, 3/4" FROM TOP SURFACE
8. ALL LAPS SHALL BE FULL TENSION LAPS (CLASS B SPLICE) UNLESS SPECIFICALLY NOTED OTHERWISE.
9. HEADED STUD ANCHORS, DEFORMED BAR ANCHORS (DBA'S), AND OTHER EMBEDDED ITEMS AS SPECIFIED FOR STRUCTURAL STEEL.
10. CHAIRS AND SPACERS SHALL BE PLACED TO ADEQUATELY SUPPORT REINFORCING DURING PLACEMENT.

E. CONCRETE FORMWORK

- 1. CONCRETE FORMS SHALL BE CLEAN AND FREE FROM DEBRIS.
2. COORDINATE WITH REINFORCING SUBMITTAL FOR OPENING AND ADDITIONAL REQUIREMENTS.
3. PROVIDE BRACING TO ENSURE STABILITY OF FORMWORK FOR PLACEMENT OPERATIONS.
4. ALL WALL SIDES AND SLAB EDGES EXPOSED TO VIEW AND PIT WALLS ON BOTH SIDES TO HAVE CLASS A - CLASS OF SURFACE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.

F. POST-INSTALLED ANCHORS INTO CONCRETE AND MASONRY

- 1. WHERE A MANUFACTURER'S ANCHORS IS SPECIFICALLY CALLED OUT ON THE DRAWINGS, IT SHALL BE CONSIDERED THE DESIGN BASIS FOR THE REQUIRED ANCHOR.
2. ADHESIVE ANCHORS, WHERE NOT SPECIFICALLY DETAILED, SHALL BE:
A. FOR CONCRETE AND CONCRETE MASONRY: HILTI HIT-HY150 MAX OR HIT-HY200
B. FOR EXISTING BRICK MASONRY: HILTI HIT-HY 70
C. INSTALL IN ACCORDANCE WITH MANUFACTURERS' SPECIFICATIONS.
3. EXPANSION ANCHORS, WHERE NOT SPECIFICALLY DETAILED, SHALL BE:
A. FOR CONCRETE: HILTI KWIK BOLT T2
B. FOR MASONRY: HILTI KWIK BOLT 3.
C. INSTALL IN ACCORDANCE WITH MANUFACTURERS' SPECIFICATIONS.
4. SCREW TYPE ANCHORS, WHERE NOT SPECIFICALLY DETAILED, SHALL FOR CONCRETE AND MASONRY: SIMPSON TITEN-HD INSTALL IN ACCORDANCE WITH MANUFACTURERS' SPECIFICATIONS.

G. CONCRETE MASONRY

- 1. MINIMUM COMPRESSIVE STRENGTH OF CONCRETE MASONRY SHALL BE Fm = 1500 PSI, DETERMINED BY THE UNIT STRENGTH METHOD.
2. CONCRETE MASONRY UNITS SHALL BE LOAD-BEARING NORMAL WEIGHT AGGREGATE CONCRETE MASONRY UNITS CONFORMING TO THE REQUIREMENTS OF ASTM C 90, GRADE N, TYPE 1 WITH MINIMUM AVERAGE NET-AREA COMPRESSIVE STRENGTH OF 1900 PSI.
3. MORTAR SHALL CONFORM TO ASTM C270 TYPE S WITH MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI.
4. GROUT SHALL CONFORM TO ASTM C476 WITH MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI.
5. DEFORMED REINFORCING BARS SHALL BE ASTM A615 GRADE 60.
6. JOINT REINFORCEMENT SHALL BE NO. 9 GALVANIZED TRUSS TYPE WIRE REINFORCING TO ASTM A-82 BY DUR-O-WAL UNLESS NOTED OTHERWISE.
7. ALL CONCRETE MASONRY UNITS SHALL BE PLACED IN RUNNING BOND.
8. CONCRETE BLOCK BENEATH BEAM BEARING POINTS SHALL BE FILLED SOLID WITH GROUT FOR A MINIMUM OF TWO FULL COURSES BENEATH THE BEAM AND FOR A WIDTH OF 24".
9. CONCRETE BLOCK PLACED BENEATH GRADE SHALL BE GROUTED SOLID.
10. TOP COURSES SHALL BE FULL COURSE AS REINFORCED AND GROUTED BOND BEAMS.
11. PROVIDE THE FOLLOWING REINFORCEMENT IN ADDITION TO THE SCHEDULED REINFORCEMENT:
A. ONE #5 VERTICAL CONTINUOUS FROM SUPPORT TO SUPPORT AT EACH CORNER, AT EACH SIDE OF OPENINGS, AND AT ENDS OF WALLS.
B. ONE #5 HORIZONTAL BAR AT TOP AND BOTTOM OF WALL OPENINGS; EXTEND 24" PAST END OF OPENING
12. LAP LENGTHS SHALL BE AS FOLLOWS FOR DEFORMED REINFORCEMENT:
#3 - 18"; #4 - 24"; #5 - 30"; #6 - 36"; #7 - 42"; #8 - 48"

H. FASTENERS

- 1. SUBMIT IES REPORTS ON ALL FASTENERS USED, INCLUDING POST-APPLIED CONCRETE ANCHORS
2. BLOCKING

I. WOOD FRAMING NOTES:

- 1. UNLESS OTHERWISE SPECIFIED, EACH PIECE OF LUMBER SHALL BEAR THE GRADE MARK, STAMP, OR OTHER IDENTIFYING MARKS INDICATING GRADES OF MATERIAL AND RULES OR STANDARDS UNDER WHICH PRODUCED.
2. PROTECT LUMBER AND OTHER PRODUCTS FROM DAMPNESS BOTH DURING AND AFTER DELIVERY AT THE SITE.
3. STORE SEASONED MATERIALS IN DRY PORTIONS OF BUILDING.
4. PROTECT SHEET MATERIALS FROM CORNERS BREAKING AND DAMAGING SURFACES WHILE UNLOADING.
5. NOMINAL SIZES ARE INDICATED EXCEPT AS SHOWN BY DETAIL DIMENSIONS.
6. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19%.
7. LIGHT GAGE METAL CONNECTIONS SHALL BE SIMPSON, SUBMIT MANUFACTURERS SPECIFICATION SHEETS.
8. ALL WOOD IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESERVATIVE PRESSURE TREATED, P.P.T.
1. LUMBER GRADES
1. 2x6 AND 2x4 BEARING WALLS, INTERIOR AND EXTERIOR LOCATIONS: SPRUCE-PINE-FIR No. 1/No.2 AS GRADED BY NLGA
2. STRUCTURAL ROOF AND FLOOR FRAMING: SPRUCE-PINE-FIR No. 1/NO.2 AS GRADED BY NLGA
3. PRESERVATIVE PRESSURE TREATED LUMBER: SOUTHERN PINE NO. 2, AS GRADED BY SPLB
4. LAMINATED VENEER LUMBER (LVL):
A. PROVIDE LVL HEADERS AND BEAMS AS INDICATED.
B. LVL FRAMING SHALL BE LAMINATED DOUGLAS FIR OR SOUTHERN PINE (GP LAM BY GEORGIA PACIFIC OR MICROLAM BY TRUS-JOIST OR EQUAL) MEETING THE FOLLOWING MINIMUM ALLOWABLE STRESS CRITERIA:
- FB (BENDING STRESS) = 2600 PSI
- FV (HORIZ. SHEAR STRESS) = 285 PSI
- E (MODULUS OF ELASTICITY) = 1,900,000 PSI
- FC (COMPRESSIONS PERPENDICULAR TO GRAIN) = 750 PSI
MISCELLANEOUS LUMBER: PROVIDE WOOD FOR SUPPORT OR ATTACHMENT OF THE WORK INCLUDING NON-BEARING PARTITIONS, CHAMF STRIPS, BUCKS, NAILERS, BLOCKING, FURRING, GROUNDS, STRIPPING AND SIMILAR MEMBERS. PROVIDE LUMBER OF SIZES AND SHAPES INDICATED. GRADE: SPRUCE-PINE-FIR STUD GRADE AS GRADED BY NLGA.

2. MATERIALS

- 1. PLYWOOD ROOF SHEATHING: APA RATED SHEATHING, SPAN RATING AS REQUIRED TO SUIT SUPPORT SPACING INDICATED, EXPOSURE DURABILITY 1, SANDED.
2. FLOOR SHEATHING: 3/4" ADVANTEK
3. FASTENERS AND ANCHORS: FURNISH ITEMS OF ROUGH HARDWARE, METAL CONNECTORS, BOLTS, ETC., REQUIRED TO COMPLETE THE WORK.
4. SILL GASKET ON TOP OF FOUNDATION WALL: 1/4 INCH THICK, PLATE WIDTH WIDE, CLOSED CELL POLYETHYLENE URETHANE FOAM FROM CONTINUOUS ROLLS.
5. SUBFLOOR GLUE: APA AFG-01, WATERPROOF OF WATER SOLVENT BASE, AIR CURE TYPE, CARTRIDGE DISPENSED.
6. BUILDING PAPER: NO. 15 ASPHALT FELT, PLAIN UNTREATED CELLULOSE BUILDING PAPER.
7. WOOD PRESERVATIVE (PRESSURE TREATMENT): AWPA TREATMENT ACQ USING WATER BORNE PRESERVATIVE WITH 0.4 PERCENT RETAINAGE.
8. SET STRUCTURAL MEMBERS LEVEL AND PLUMB, IN CORRECT POSITION.
9. MAKE PROVISIONS FOR ERECTION LOADS, AND FOR SUFFICIENT TEMPORARY BRACING TO MAINTAIN STRUCTURE SAFE, PLUMB, AND IN TRUE ALIGNMENT UNTIL COMPLETION OF ERECTION AND INSTALLATION OF PERMANENT BRACING.
10. PLACE HORIZONTAL MEMBERS, CROWN SIDE UP.
11. CONSTRUCT LOAD BEARING FRAMING FULL LENGTH WITHOUT SPLICES.
12. DOUBLE MEMBERS AT OPENINGS OVER 24 INCHES WIDE.
13. CONSTRUCT DOUBLE JOIST HEADERS AT FLOOR AND CEILING OPENINGS AND UNDER WALL STUD PARTITIONS THAT ARE PARALLEL TO FLOOR JOISTS.
14. BRIDGE JOISTS FRAMING IN EXCESS OF 8 FEET SPAN AT MID-SPAN AND WHERE SHOWN ON DRAWINGS.
15. SECURE ROOF SHEATHING WITH LONGER EDGE PERPENDICULAR TO FRAMING MEMBERS AND WITH ENDS STAGGERED AND SHEET ENDS OVER BEARING.
16. USE SHEATHING CLIPS BETWEEN SHEETS BETWEEN ROOF FRAMING MEMBERS.
17. WHERE TONGUE AND GROOVE PLYWOOD IS USED, FULLY ENGAGE TONGUE AND GROOVE EDGES.
18. SECURE WALL SHEATHING WITH LONG DIMENSION PERPENDICULAR TO WALL STUDS.
19. PLACE BUILDING PAPER HORIZONTALLY OVER WALL SHEATHING; WEATHER LAP EDGES AND ENDS.
20. SECURE SUB-FLOOR SHEATHING WITH LONGER EDGE PERPENDICULAR TO FLOOR FRAMING AND WITH END JOINTS STAGGERED AND SHEET ENDS OVER BEARINGS.
21. TOLERANCES:
A. FRAMING MEMBERS: 1/4 INCH FROM TRUE POSITION, MAXIMUM.
B. SURFACE FLATNESS OF FLOOR: 1/4 INCH IN 10 FEET MAXIMUM, AND 1/2 INCH IN 30 FEET MAXIMUM.
22. ALL POSTS AND COLUMNS FROM HEADERS AND BEAMS SHALL BEAR CONTINUOUSLY TO CONCRETE FOUNDATIONS INCLUDING BLOCKING IN FLOOR AND ROOF SPACES.
23. ALL BOTTOM BEARING PLATES, FOR STUD WALLS OR BEAM BEARING, SHALL BE ANCHORED TO THE FOUNDATION WITH 1/2" DIAMETER ANCHOR BOLTS AT 4'-0" ON CENTER, UNLESS NOTED OTHERWISE.
24. ALL BEARING WALLS SHALL BE BLOCKED AT 4'-0" ON CENTER, VERTICALLY, UNLESS NOTED OTHERWISE.
25. ALL WOOD IN CONTACT WITH CONCRETE SHALL BE PRESERVATIVE PRESSURE TREATED, P.P.T.
26. ALL FASTENERS FOR PRESSURE TREATED WOOD TO BE G90 HOT-DIPPED GALVANIZED.
27. ALL HANGERS FOR PRESSURE TREATED WOOD TO BE G90 HOT-DIPPED GALVANIZED.
28. PROVIDE 1/4" NOMINAL GAP BETWEEN WOOD FRAMING AND HORIZONTAL FACES OF CONCRETE WALLS.

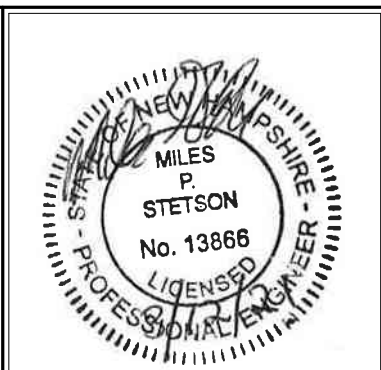


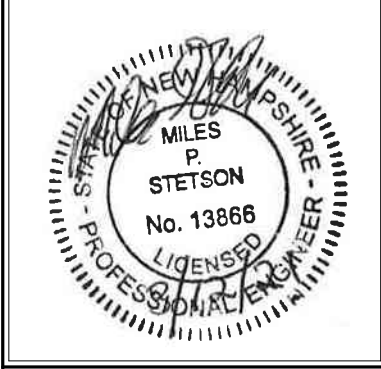
Table with 3 columns: Date, Description, No.

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235 RT 120 PLAINFIELD, NH

GENERAL NOTES NH PLAINFIELD LUNDY ADDITION

Table with 2 columns: Field, Value. Includes Design By (MAN), Checked By (MS), Drawn By (MAN), Scale (1/8" = 1'-0"), Date, Issue Date.



Date	
Description	
No.	

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235 RT 120
 PLAINFIELD, NH

GENERAL NOTES CONT.
 NH PLAINFIELD LUNDY ADDITION

Designed By: MAN
 Checked By: MS
 Drawn By: MAN
 Scale: As indicated
 Date: Issue Date

FASTENING SCHEDULE
 (TABLE 2304.9.1, IBC 2012)

CONNECTION	FASTENING ^{a-m}	LOCATION
1. JOIST TO SILL OR GIRDER	(3) 8d COMMON (2-1/2" x 0.131") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
2. BRIDGING TO JOIST	(2) 8d COMMON (2-1/2" x 0.131") (2) 3" x 0.131" NAILS (2) 3" 14 GAGE STAPLES	TOENAIL EACH END
3. 1" x 6" SUBFLOOR OR LESS TO EACH JOIST	(2) 8d COMMON (2-1/2" x 0.131")	FACE NAIL
4. WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST	(3) 8d COMMON (2-1/2" x 0.131")	FACE NAIL
5. 2" SUBFLOOR TO JOIST OR GIRDER	(2) 16d COMMON (3-1/2" x 0.162")	BLIND AND FACE NAIL
6. SOLE PLATE TO JOIST OR BLOCKING	16d (3-1/2" x 0.135") AT 16"oc 3" x 0.131" NAILS AT 8"oc 3" 14 GAGE STAPLES AT 12"oc	TYPICAL FACE NAIL
SOLE PLATE TO JOIST OR BLOCKING AT BRACED WALL PANEL	(3) 16d (3-1/2" x 0.135") AT 16"oc (4) 3" x 0.131" NAILS AT 16"oc (4) 3" 14 GAGE STAPLES AT 16"oc	BRACE WALL PANELS
7. TOP PLATE TO STUD	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	END NAIL
8. STUD TO SOLE PLATE	(4) 8d COMMON (2-1/2" x 0.131") (4) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	END NAIL
9. DOUBLE STUDS	16d (3-1/2" x 0.135") AT 24"oc 3" x 0.131" NAIL AT 8"oc 3" 14 GAGE STAPLE AT 8"oc	FACE NAIL
10. DOUBLE TOP PLATES	16d (3-1/2" x 0.135") AT 16"oc 3" x 0.131" NAIL AT 12"oc 3" 14 GAGE STAPLE AT 12"oc	TYPICAL FACE NAIL
DOUBLE TOP PLATES	(8) 16d COMMON (3-1/2" x 0.162") (12) 3" x 0.131" NAILS (12) 3" 14 GAGE STAPLES	LAP SPLICE
11. BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3) 8d COMMON (2-1/2" x 0.131") AT 24"oc (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
12. RIM JOIST TO TOP PLATE	8d (2-1/2" x 0.131") AT 6"oc 3" x 0.131" NAIL 6"oc 3" 14 GAGE STAPLE AT 6"oc	TOENAIL
13. TOP PLATES, LAPS AND INTERSECTIONS	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL
14. CONTINUOUS HEADER, TWO PIECES	16d COMMON (3-1/2" x 0.162")	16"oc ALONG EDGE
15. CEILING JOISTS TO PLATE	(3) 8d COMMON (2-1/2" x 0.131") (5) 3" x 0.131" NAILS (5) 3" 14 GAGE STAPLES	TOENAIL
16. CONTINUOUS HEADER TO STUD	(4) 8d COMMON (2-1/2" x 0.131")	TOENAIL
17. CEILING JOISTS, LAPS OVER PARTITIONS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3) 16d COMMON (3-1/2" x 0.162") MINIMUM, TABLE 2308.10.4.1 (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
18. CEILING JOISTS TO PARALLEL RAFTERS (SEE SECTION 2308.10.4.1, TABLE 2308.10.4.1)	(3) 16d COMMON (3-1/2" x 0.162") MINIMUM, TABLE 2308.10.4.1 (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
19. RAFTER TO PLATE (SEE SECTION 2308.10.1, TABLE 2308.10.1)	(3) 8d COMMON (2-1/2" x 0.131") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAIL
20. 1" DIAGONAL BRACE TO EACH STUD AND PLATE	(2) 8d COMMON (2-1/2" x 0.131") (2) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL
21. 1" x 8" SHEATHING TO EACH BEARING	(3) 8d COMMON (2-1/2" x 0.131")	FACE NAIL
22. WIDER THAN 1" x 8" SHEATHING TO EACH BEARING	(3) 8d COMMON (2-1/2" x 0.131")	FACE NAIL
23. BUILT-UP CORNER STUDS	16d COMMON (3-1/2" x 0.162") 3" x 0.131" NAILS 3" 14 GAGE STAPLES	24"oc 16"oc 16"oc

FASTENING SCHEDULE
 (TABLE 2304.9.1, IBC 2012)

CONNECTION	FASTENING	LOCATION
24. BUILT-UP GIRDER AND BEAMS	20d COMMON (4" x 0.192") AT 32"oc 3" x 0.131" NAILS 3" 14 GAGE STAPLES	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES
25. 2" PLANKS	(2) 20d COMMON (4" x 0.192") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL NAIL AT ENDS AND AT EACH SPLICE
26. COLLAR TIE TO RAFTER	(3) 10d COMMON (3" x 0.148") (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
27. JACK RAFTER TO HIP	(3) 10d COMMON (3" x 0.148") (4) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAILS
	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL
28. ROOF RAFTER TO 2-by RIDGE BEAM	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	TOENAILS
	(2) 16d COMMON (3-1/2" x 0.162") (3) 3" x 0.131" NAILS (3) 3" 14 GAGE STAPLES	FACE NAIL
29. JOIST TO BAND JOIST	(3) 16d COMMON (3-1/2" x 0.162") (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL
30. LEDGER STRIP	(3) 16d COMMON (3-1/2" x 0.162") (4) 3" x 0.131" NAILS (4) 3" 14 GAGE STAPLES	FACE NAIL AT EACH JOIST
31. WOOD STRUCTURAL PANELS AND PARTICLEBOARD SUBFLOOR, ROOF AND WALL SHEATHING (TO FRAMING)	1/2" AND LESS 6d ¹ 2-3/8" x 0.113 NAIL ^o 1-3/4" 16 GAGE ^o 19/32" TO 3/4" 8d OR 6d ^e 2-3/8" x 0.113 NAIL ^p 2" 16 GAGE ^p 8d ^q 7/8" TO 1" ^r 1-1/8" TO 1-1/4" ^r 3/4" AND LESS 6d ^s 7/8" TO 1" ^s 1-1/8" TO 1-1/4" 10d OR 8d ^s	
SINGLE FLOOR (COMBINATION SUBFLOOR-UNDERLAYMENT TO FRAMING)	3/4" AND LESS 6d ^s 7/8" TO 1" ^s 1-1/8" TO 1-1/4" 10d OR 8d ^s	
32. PANEL SIDING (TO FRAMING)	1/2" OR LESS 6d ^t 5/8" 8d ^t	
33. FIBERBOARD SHEATHING	1/2" No. 11 GAGE ROOFING NAIL ^h 6d COMMON NAIL (2" x 0.113") 25/32" No. 16 GAGE STAPLE ⁱ No. 11 GAGE ROOFING NAIL ^h 8d COMMON NAIL (1/2" x 0.113") No. 16 GAGE STAPLE ⁱ	
34. INTERIOR PANELING	1/4" 4d ^j 3/8" 6d ^k	

FOR S1: 1" = 25.4 mm

a. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.

b. NAILS SPACED AT 6" ON CENTER AT PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS EXCEPT 6" AT SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.

c. COMMON OR DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2-1/2" x 0.131"; 10d - 3" x 0.148").

d. COMMON (6d - 2" x 0.113"; 8d - 2-1/2" x 0.131"; 10d - 3" x 0.148").

e. DEFORMED SHANK (6d - 2" x 0.113"; 8d - 2-1/2" x 0.131"; 10d - 3" x 0.148").

f. CORROSION-RESISTANT SIDING (6d - 1-7/8" x 0.106"; 8d - 2-3/8" x 0.128") OR CASING (6d - 2" x 0.099"; 8d - 2-1/2" x 0.113").

g. FASTENERS SPACED 3 INCHES ON CENTER AT EXTERIOR EDGES AND 6 INCHES ON CENTER AT INTERMEDIATE SUPPORTS, WHEN USED AS STRUCTURAL SHEATHING. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS.

h. CORROSION-RESISTANT ROOFING NAILS WITH 7/16-INCH-DIAMETER HEAD AND 1-1/2-INCH LENGTH FOR 1/2-INCH SHEATHING AND 1-3/4-INCH LENGTH FOR 25/32-INCH SHEATHING.

i. CORROSION-RESISTANT STAPLES WITH NOMINAL 7/16-INCH CROWN OR 1-INCH CROWN AND 1-1/4-INCH LENGTH FOR 1/2-INCH SHEATHING AND 1-1/2-INCH LENGTH FOR 25/32-INCH SHEATHING. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS NOTED OTHERWISE).

j. CASING (1-1/2" x 0.080") OR FINISH (1-1/2" x 0.072") NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.

k. PANEL SUPPORTS AT 24 INCHES. CASING OR FINISH NAILS SPACED 6 INCHES ON PANEL EDGES, 12 INCHES AT INTERMEDIATE SUPPORTS.

l. FOR ROOF SHEATHING APPLICATIONS, 8d NAILS (2-1/2" x 0.113") ARE THE MINIMUM REQUIRED FOR WOOD STRUCTURAL PANELS. STAPLES SHALL HAVE A MINIMUM CROWN WIDTH OF 7/16 INCH.

m. FOR ROOF SHEATHING APPLICATIONS, FASTENERS SPACED 4 INCHES ON CENTER AT EDGES, 8" AT INTERMEDIATE SUPPORTS.

n. FASTENERS SPACED AT 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS FOR SUBFLOOR AND WALL SHEATHING AND 3 INCHES ON CENTER AT EDGES, 6 INCHES AT INTERMEDIATE SUPPORTS FOR ROOF SHEATHING.

o. FASTENERS SPACED AT 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

p. FASTENERS SPACED AT 4 INCHES ON CENTER AT EDGES, 8 INCHES AT INTERMEDIATE SUPPORTS.

SHEAR WALL SCHEDULE

WALL MARK	SHEATHING	NAILING		HOLDOWN	BOTTOM PLATE ATTACHMENT
		AT PANEL EDGES	AT INTERMEDIATE SUPPORTS		
SW-1	1/2" THICK ON ONE SIDE	8d NAILS AT 6"oc	8d NAILS AT 12"oc	HD-A	SEE DETAIL 9/S-4

NOTES:
 1. PROVIDE BLOCKING AT ALL PANEL EDGES.
 2. ALL STUDS SHALL BE SPF No. 1 / No. 2 OR BETTER.
 3. ALL WALL PLATES SHALL BE No. 1 / No. 2 OR BETTER.
 4. ALL SHEATHING SHALL BE APA RATED.
 5. ALL SHEAR WALLS TO EXTEND FROM FLOOR DIAPHRAGMS TO FLOOR OR ROOF DIAPHRAGMS.

HOLD DOWN SCHEDULE

MARK	HOLD DOWN	MINIMUM VERTICAL WOOD MEMBER	CONCRETE ANCHOR	DETAIL
HD-A	DTTZ-SDS2.5	(2) 2x6	1/2" dia. EPOXY ANCHOR 4 1/2" EMBEDMENT	1/S-4

NOTES:
 1. HOLD DOWNS BY SIMPSON STRONG-TIE.
 2. EPOXY ANCHORS TO BE HILTI HAS RODS DRILLED AND SET WITH HILTI HY 200 ADHESIVE SYSTEM. MAINTAIN 3" CONCRETE EDGE DISTANCE.

BASIS OF DESIGN

1. Building Code:	IBC 2015 & ASCE 7-10
2. Dead Loads:	
a. Roof Dead Load:	20 psf
3. Live Loads:	
a. Roof Live Load:	Snow Load Governs
b. Floor Live Load (live load reduction not used):	50 psf
4. Roof Snow Load:	
a. Ground Snow Load, P _g :	80 psf
b. Flat Roof Snow Load, P _r :	69 psf
c. Snow Exposure Factor, C _e :	1.0
d. Snow Load Importance Factor, I _s :	1.0
e. Thermal Factor, C _t :	1.1
1. Wind Design Data:	
a. Basic Wind Speed (3-second gust), V:	115 mph
b. Wind Exposure:	C
c. Internal Pressure Coefficients:	+/- 0.18
d. Components and Cladding Wind Pressure:	per ASCE 7
2. Earthquake Design Data:	
a. Seismic Importance Factor, I:	1.0
b. Risk Category:	II
c. Mapped Spectral Response Acceleration, S _s :	0.244
d. Mapped Spectral Response Acceleration S ₁ :	0.083
e. Site Class:	D
f. Spectral Response Coefficient, S _{ps} :	0.280
g. Spectral Response Coefficient, S _{or} :	0.133
h. Seismic Design Category:	C
i. Basic Seismic-Force-Resisting System:	Light-Framed walls sheathed with wood structural panels rated for shear resistance
j. Analysis Procedure Used:	Equivalent Lateral Force Procedure
3. Allowable Soil Bearing Pressure:	2000 psf (Assumed to be verified by other at time of construction)

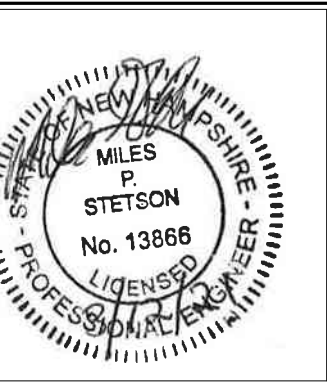
ABBREVIATIONS

AB	ANCHOR BOLT	MC	MOMENT CONNECTION
AFF	ABOVE FINISH FLOOR	N.S.	NEAR SIDE
AL	ALUMINUM	oc	ON CENTER
B.O.F.	BOTTOM OF FOOTING	P#	PIER DESIGNATION
DWG	DRAWING	PL	PLATE
E.F.	EACH FACE	SS	STAINLESS STEEL
ELEV.	ELEVATION	STD	STANDARD
EP	EMBED PLATE	T.O.C.	TOP OF CONCRETE
EQ	EQUAL	T.O.S.	TOP OF STEEL
E.S.	EACH SIDE	T.O.SHELF	TOP OF SHELF
E.W.	EACH WAY	T.O.W.	TOP OF WALL
EX.	EXISTING	TYP.	TYPICAL
F#	FOOTING DESIGNATION	U.N.O.	UNLESS NOTED OTHERWISE
FND	FOUNDATION	V.I.F.	VERIFY IN FIELD
F.S.	FAR SIDE		
H.T.	HEAVY TIMBER		

DRAWING LEGEND

NOTE: NOT ALL SYMBOLS AND NOTATIONS USED

	NORTH ARROW		CONCRETE
	ELEVATION		GROUT or FINE CRUSHED GRAVEL
	TOP OF FOOTING ELEV.		GRATING
	SECTION NUMBER DRAWING WHERE SHOWN		LEDGE/ROCK
	DECK SPAN DIRECTION OR GRATING DIRECTION		3/4" CRUSHED STONE
	SLOPE DIRECTION, and MAGNITUDE		COMPACTED GRANULAR FILL
	BEAM/COLUMN SPLICE		RIGID INSULATION
	ROOF PITCH		WOOD
	FOOTING STEP		UNDISTURBED SUBGRADE
	OPENING		CMU BLOCK
	MOMENT CONNECTION		BRICK
	GUARDRAIL/RAILING		
	BEAM PENETRATION		



Date	
Description	
No.	

ENGINEERING VENTURES PC
 208 Flynn Avenue, Suite 2A, Burlington, VT 05401
 tel. 802-563-6225 fax. 802-865-6306
 85 Mechanic Street, Suite B-2, Lebanon, NH 03766
 tel. 603-442-9333 fax. 603-442-9331
 www.engineeringventures.com

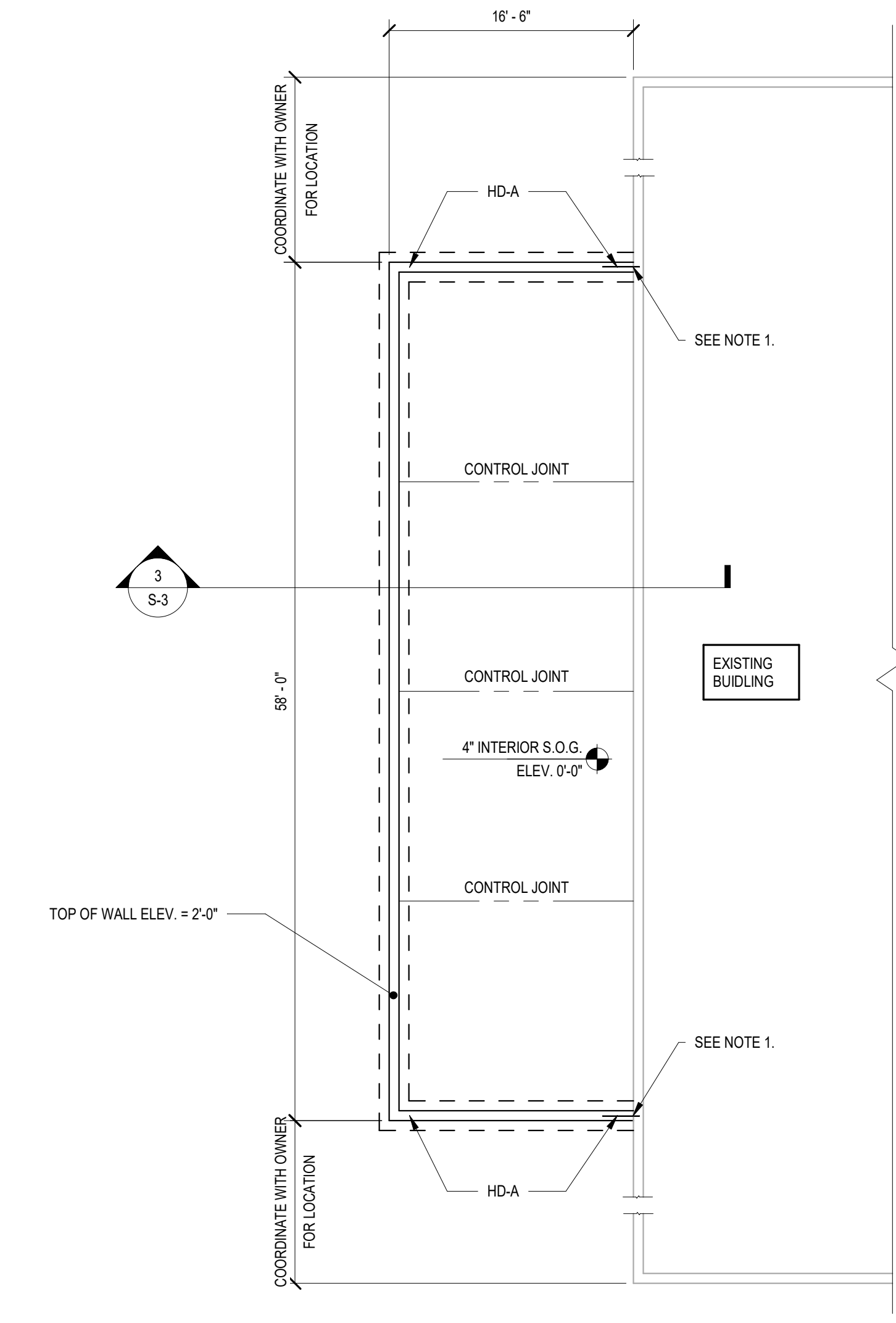
235 RT 120
 PLAINFIELD, NH

PLANS & SECTIONS
 NH PLAINFIELD LUNDY ADDITION

Designed By:	MAN
Checked By:	MS
Drawn By:	MAN
Scale:	As indicated
Date:	Issue Date

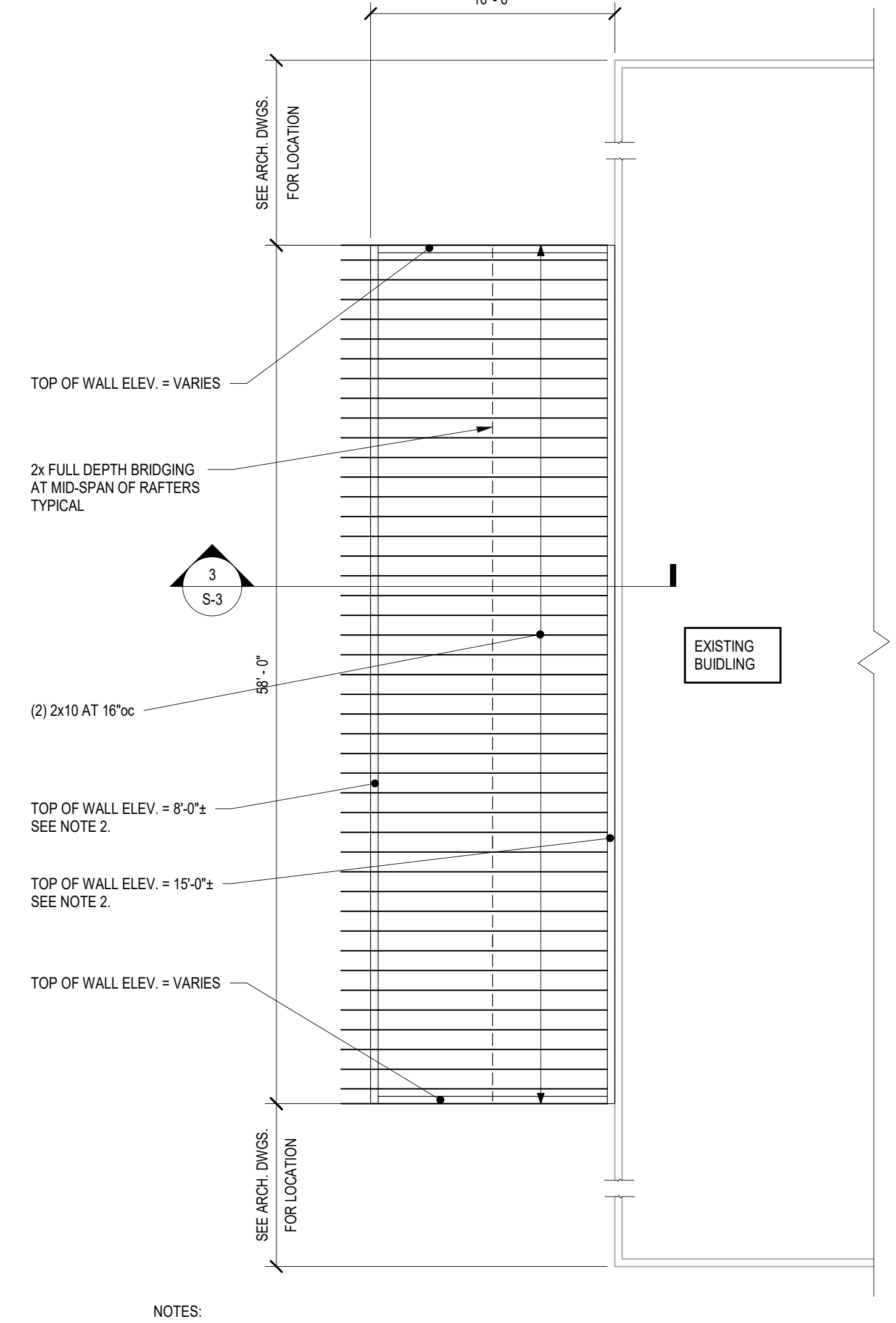
S-3

EV Project #21119



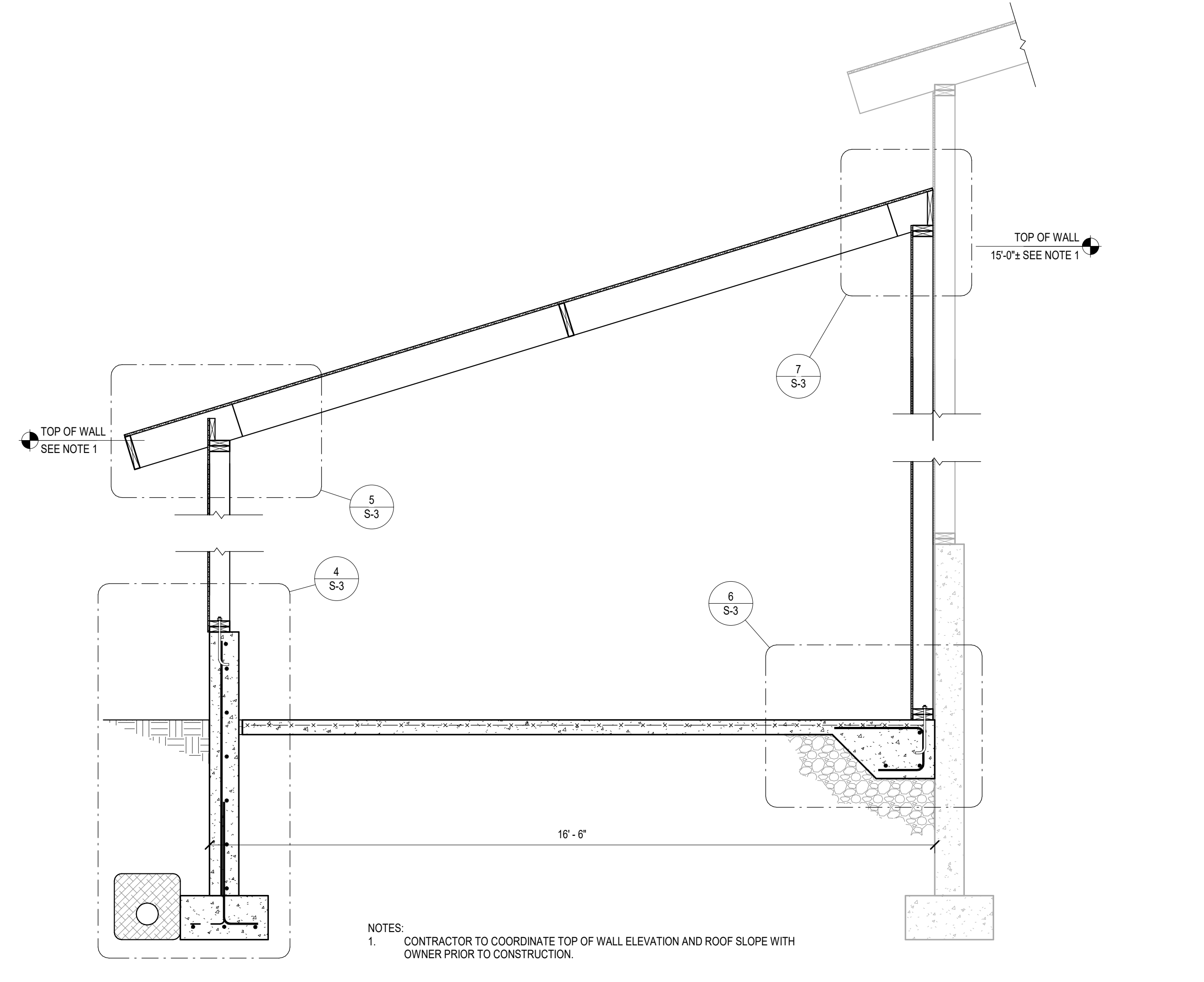
NOTES:
 1. #5x30" LG. EMBEDDED 5" INTO EXISTING CONCRETE WITH HILTI HIT-HY 200 ADHESIVE AT 12" OC VERTICAL SPACING

1 FOUNDATION PLAN
 1/8" = 1'-0"



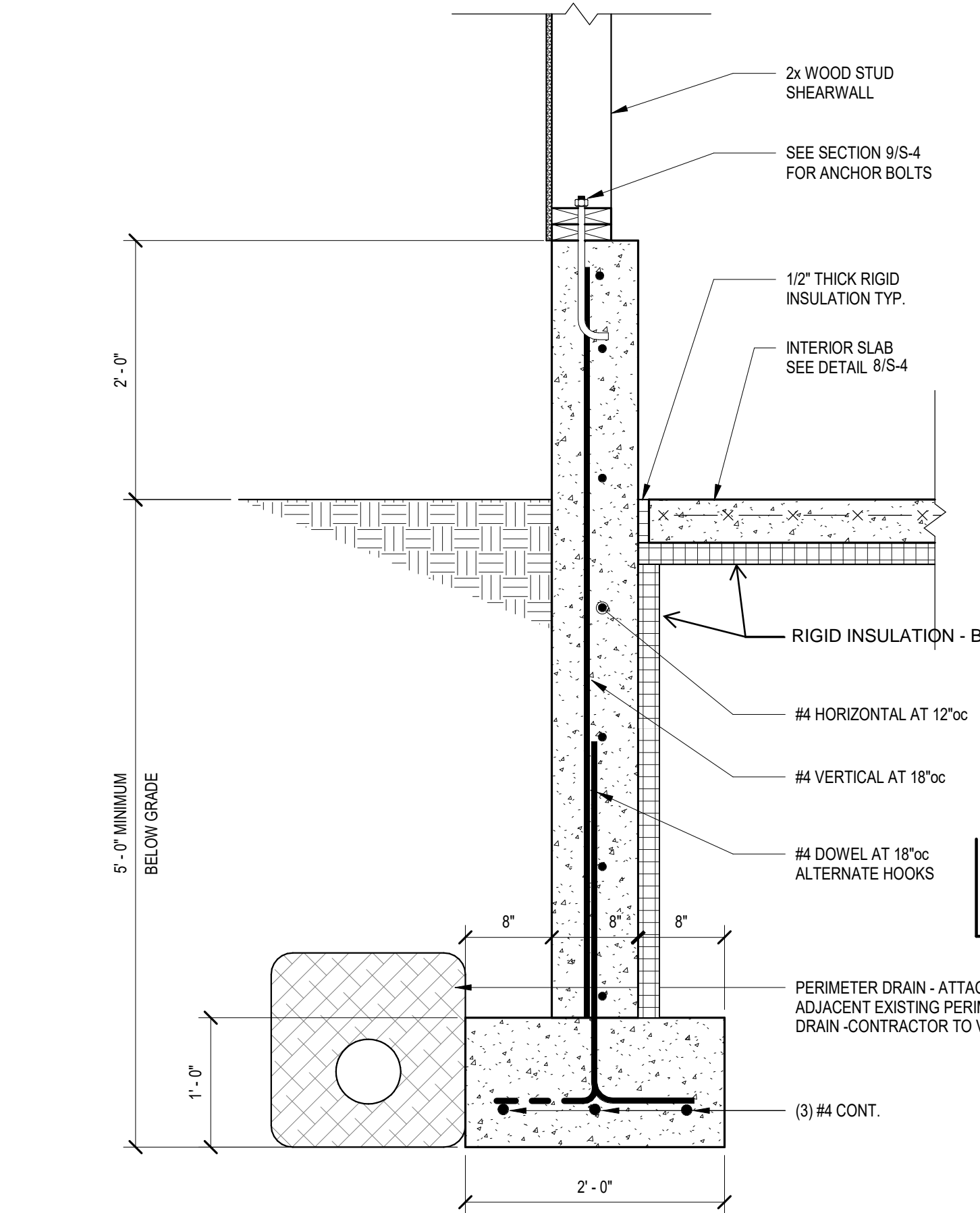
NOTES:
 1. UNLESS NOTED OTHERWISE, ALL WALLS SHALL BE "SW"
 2. CONTRACTOR TO COORDINATE TOP OF WALL ELEVATION AND ROOF SLOPE WITH OWNER PRIOR TO CONSTRUCTION

2 ROOF FRAMING PLAN
 1/8" = 1'-0"

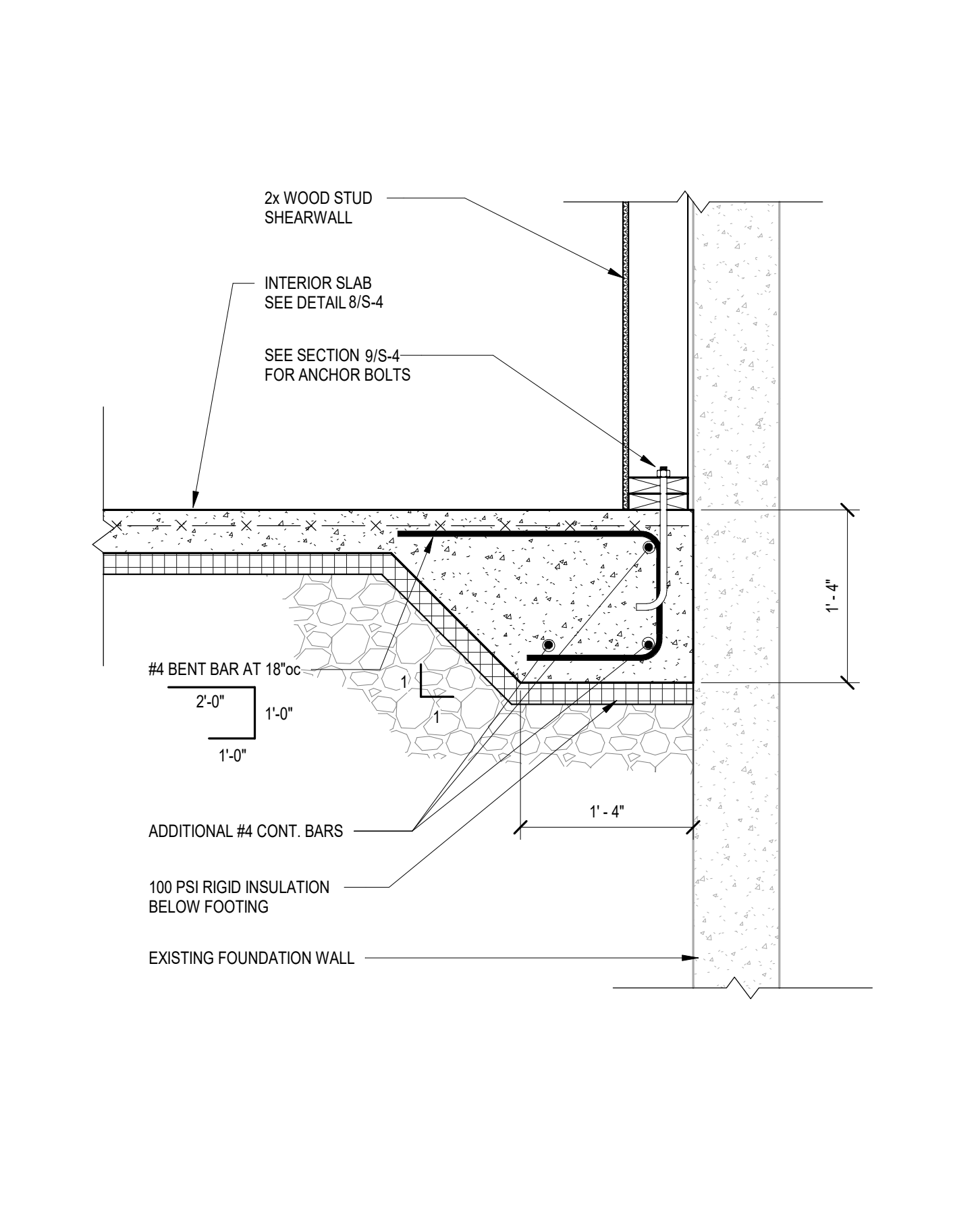


NOTES:
 1. CONTRACTOR TO COORDINATE TOP OF WALL ELEVATION AND ROOF SLOPE WITH OWNER PRIOR TO CONSTRUCTION.

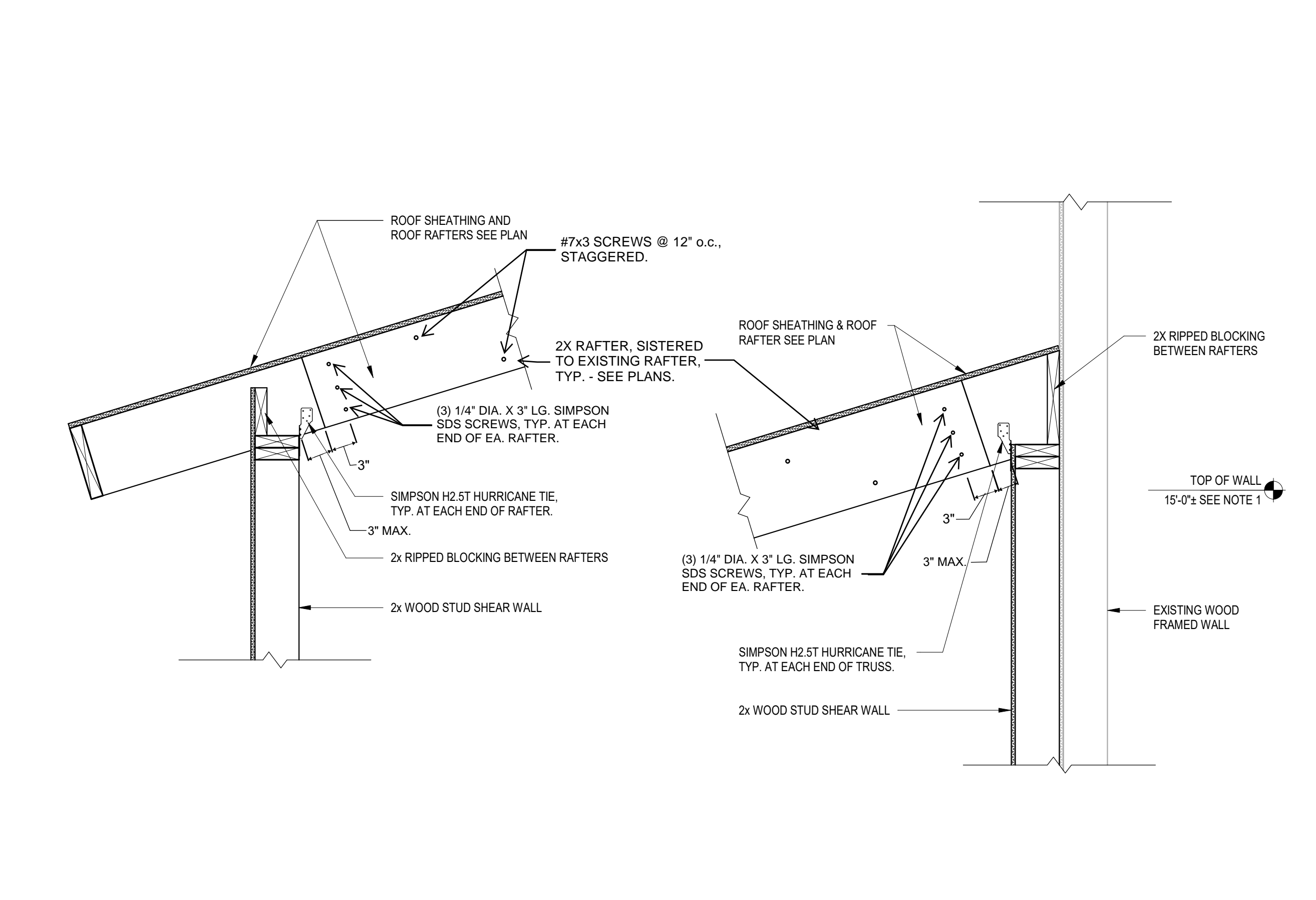
3 SECTION
 1/2" = 1'-0"



4 SECTION
 1" = 1'-0"

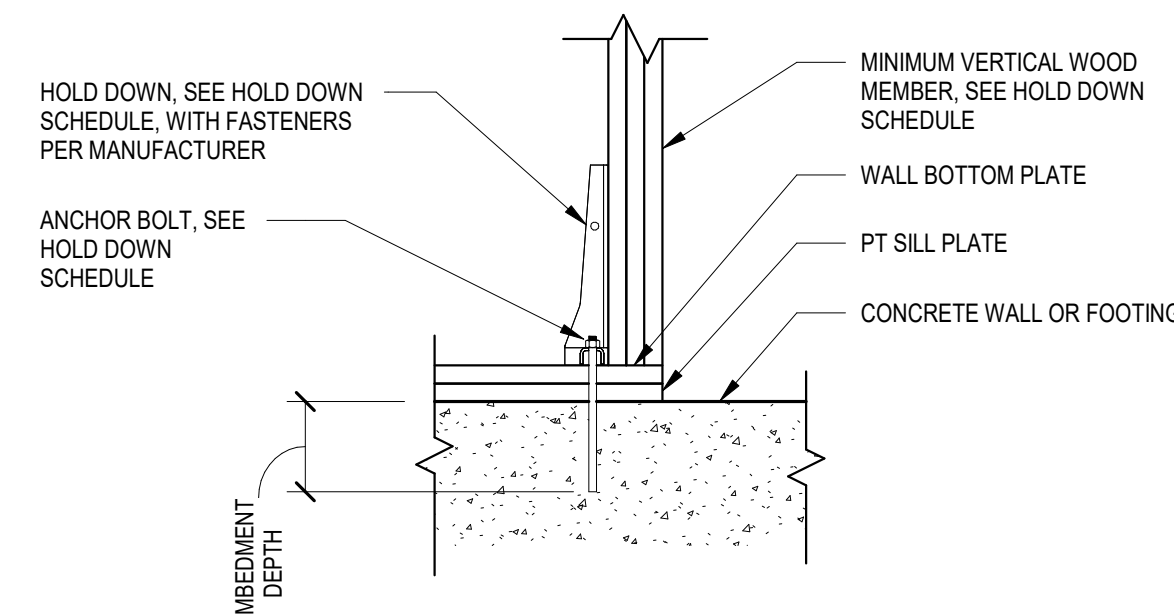


6 SECTION
 1" = 1'-0"

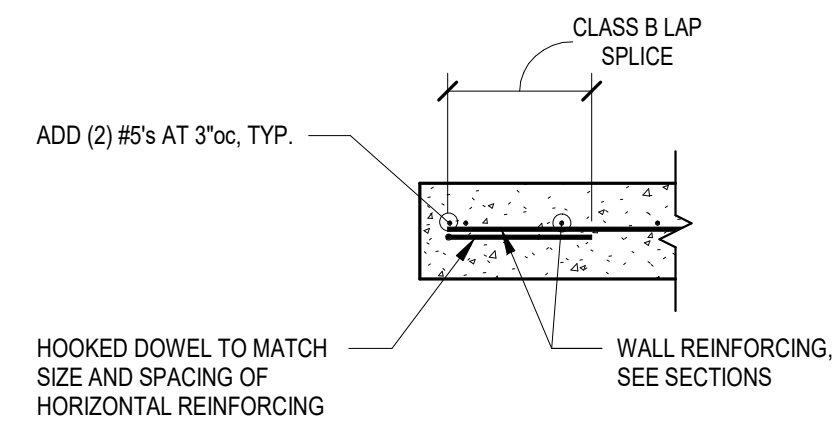


5 SECTION
 1" = 1'-0"

7 SECTION
 1" = 1'-0"

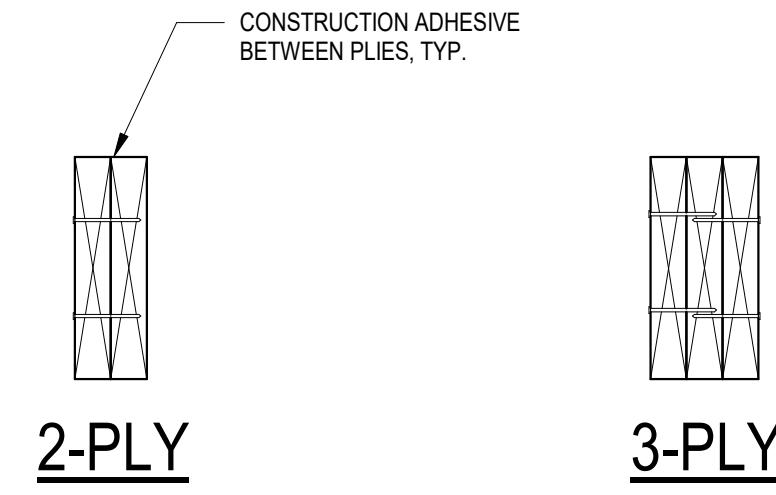


1 TYPICAL SHEAR WALL HOLD DOWN
3/4" = 1'-0"



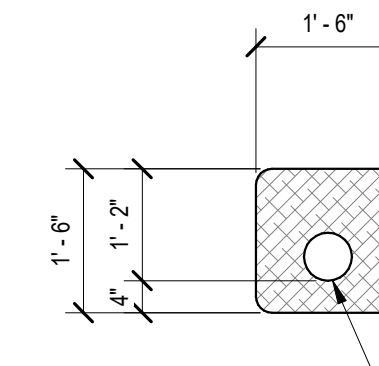
PART PLAN-SINGLE LAYER OF REINFORCING

2 TYPICAL CONCRETE WALL END
1/2" = 1'-0"



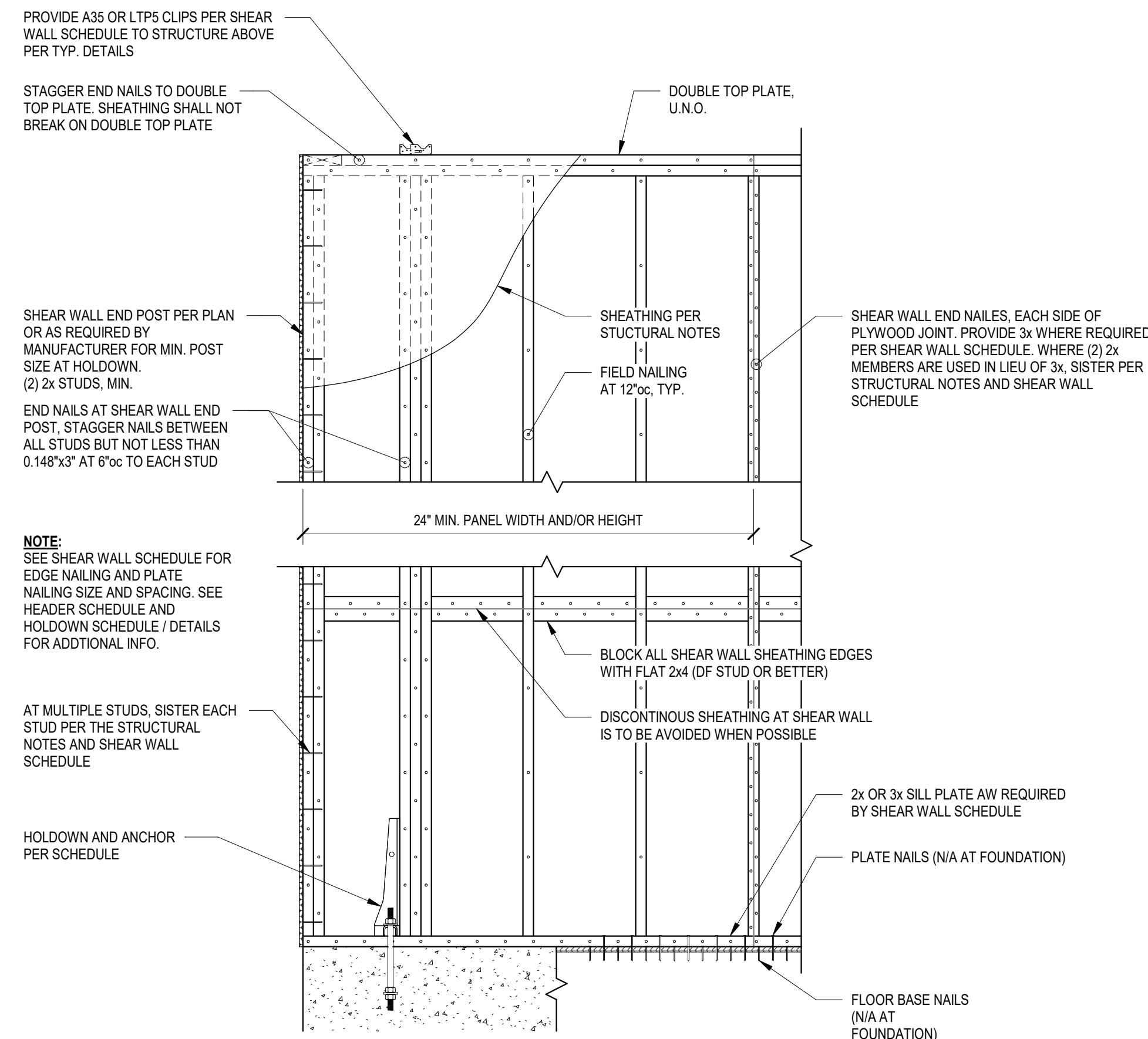
2x FRAMING
MEMBERS ≤ 12" DEEP: 2 ROWS OF 10d NAILS AT 12"oc
MEMBERS > 12" DEEP: 3 ROWS OF 10d NAILS AT 12"oc

3 TYPICAL MULTI-MEMBER BEAM CONNECTION
1 1/2" = 1'-0"

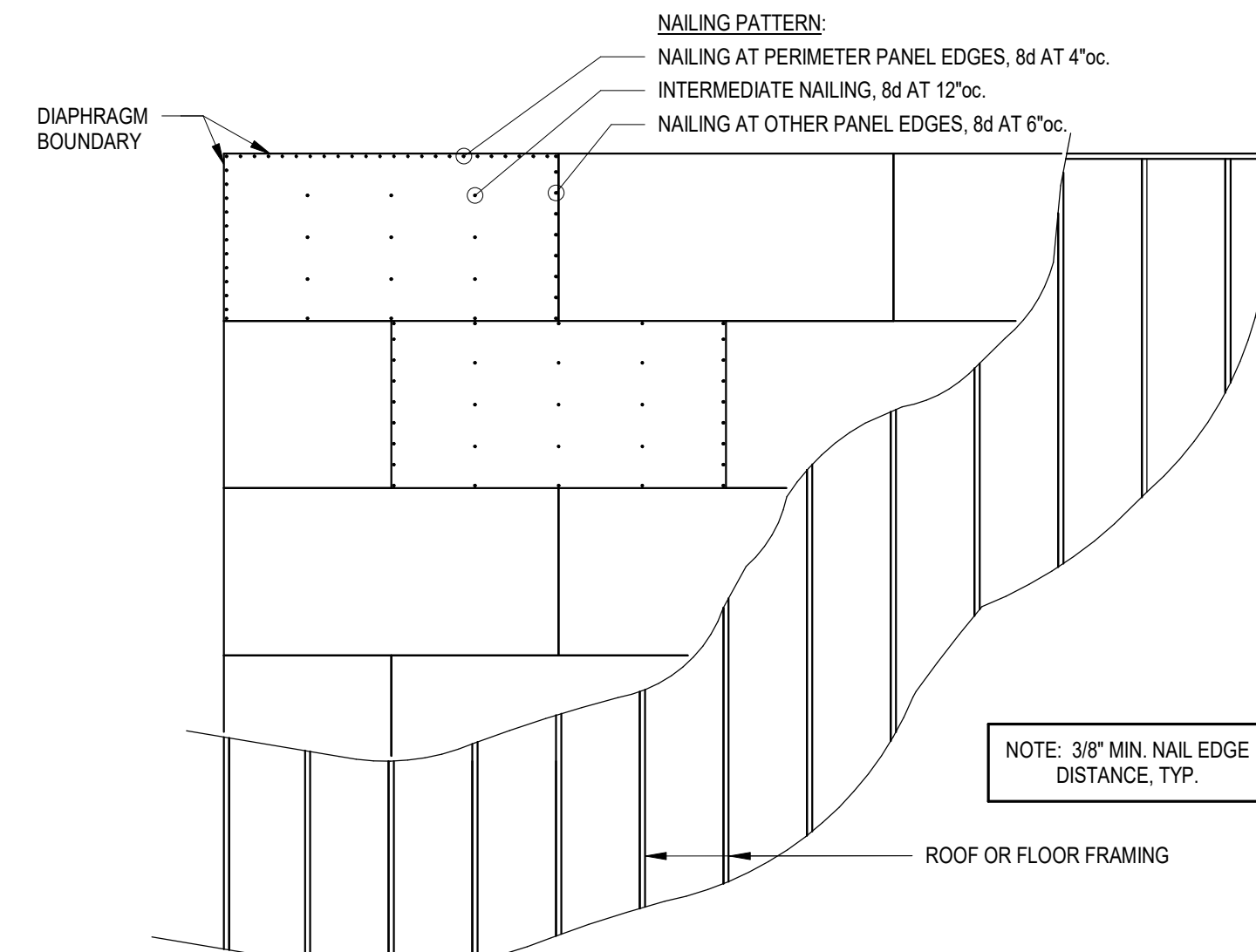


6" PERFORATED PVC DRAIN. PLACE IN 18" OF CRUSHED STONE. WRAP IN MIRAFIL 140N FILTER FABRIC OR EQUIVALENT. REFER TO SITE ENGINEERING DRAWINGS FOR OUTLET LOCATION AND DETAILS. PROVIDE CLEAN OUTS AT ALL CORNERS AND 50' MAXIMUM SPACING. COORDINATE WITH SPECIFICATIONS.

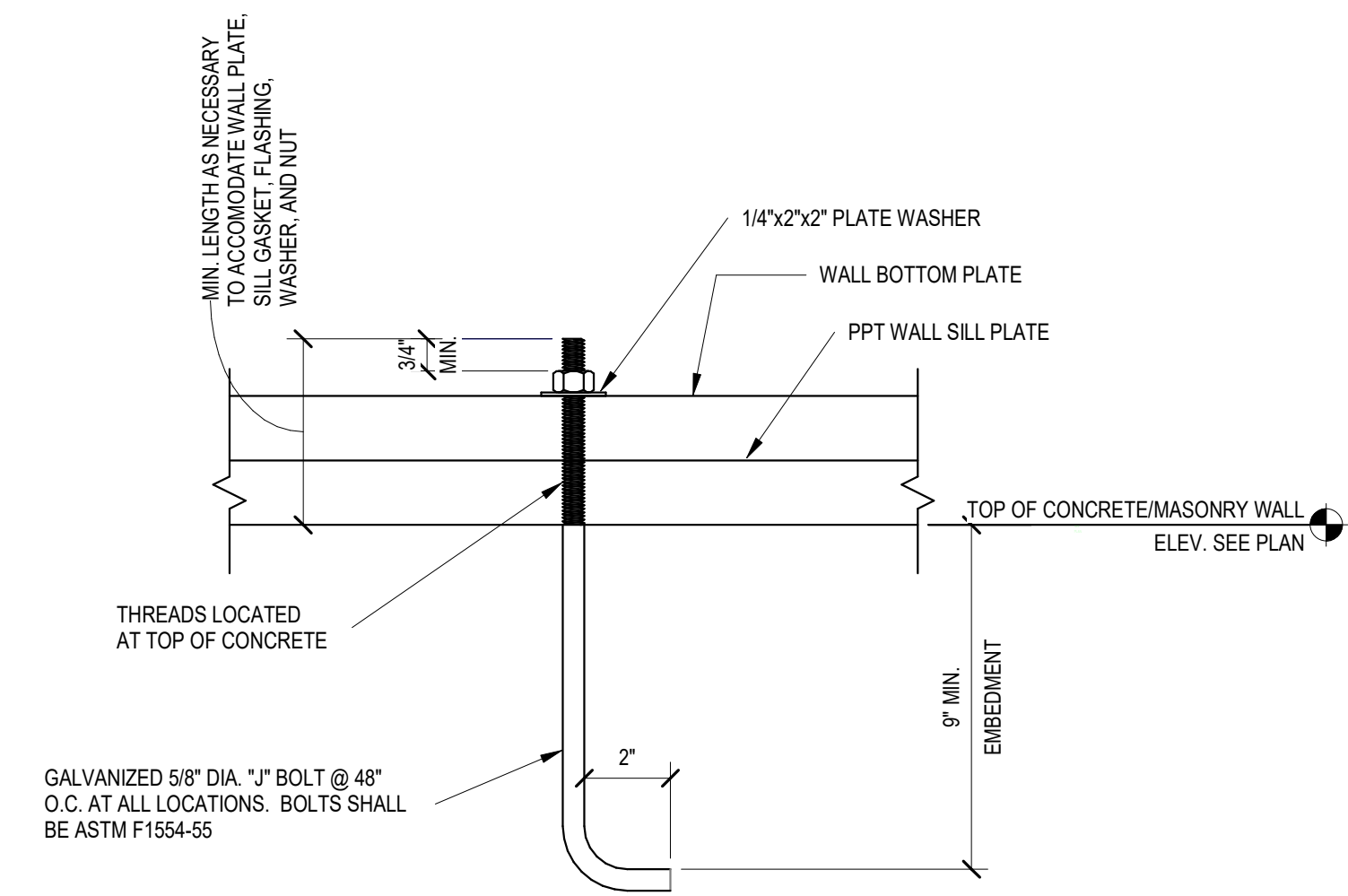
4 TYPICAL FOOTING DRAIN
1/2" = 1'-0"



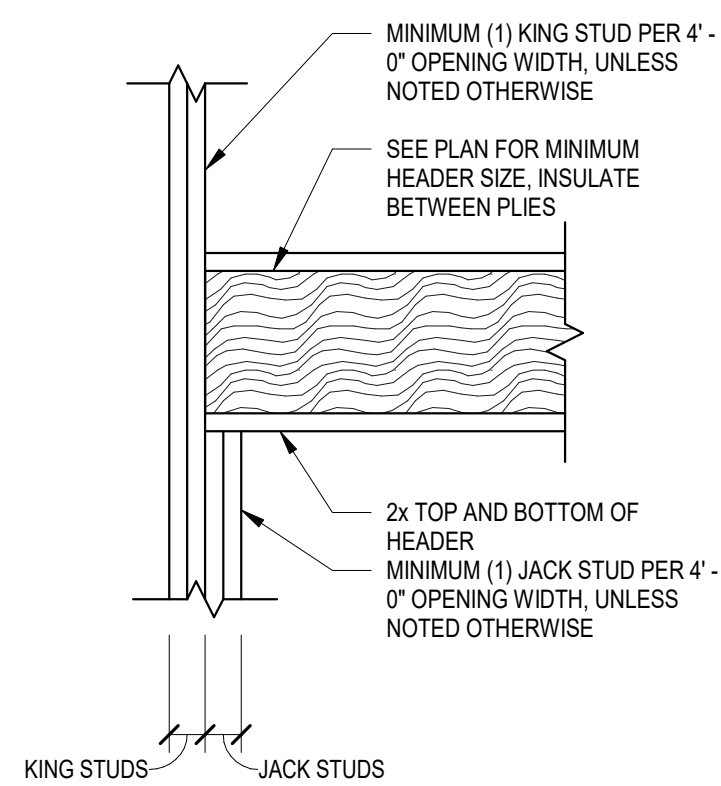
5 TYPICAL SHEAR WALL CONSTRUCTION
3/4" = 1'-0"



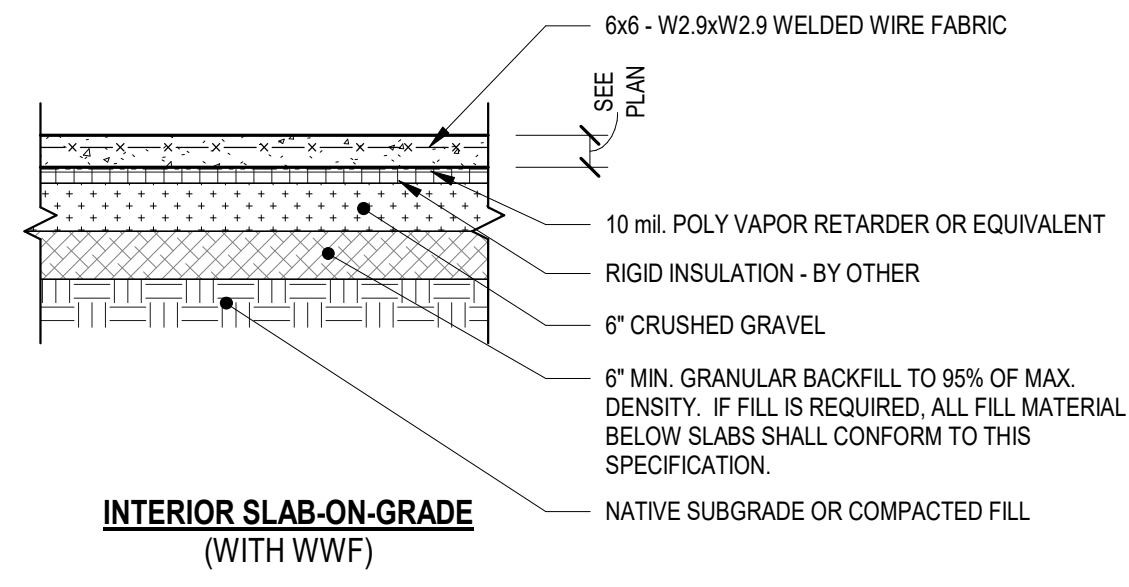
6 TYPICAL LAYOUT AND NAILING FOR FLOOR AND ROOF SHEATHING
3/4" = 1'-0"



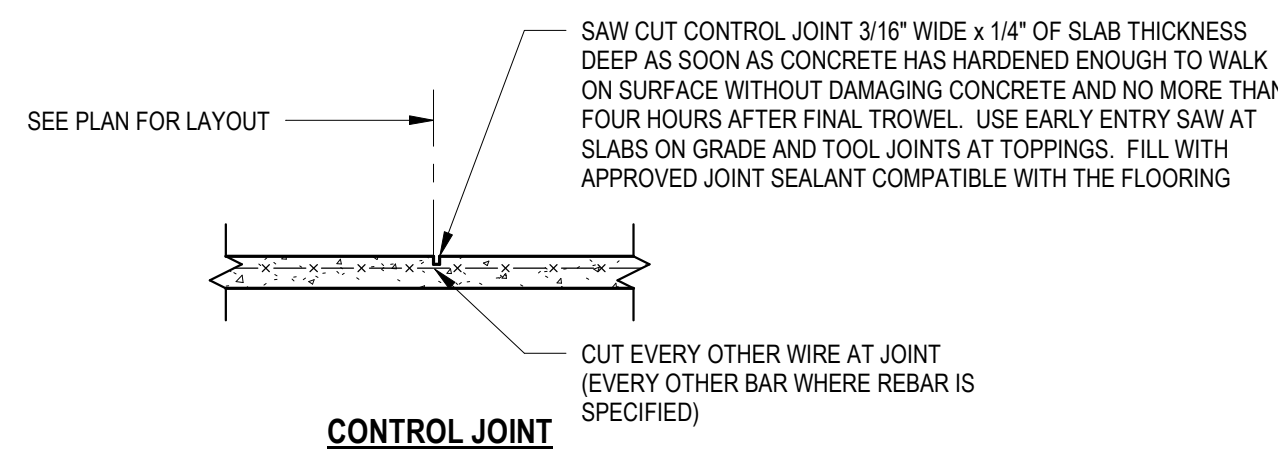
9 TYP. STUD WALL ANCHOR BOLT DETAIL
3" = 1'-0"



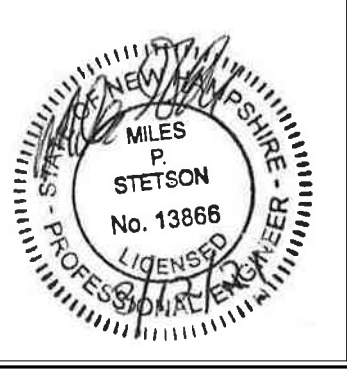
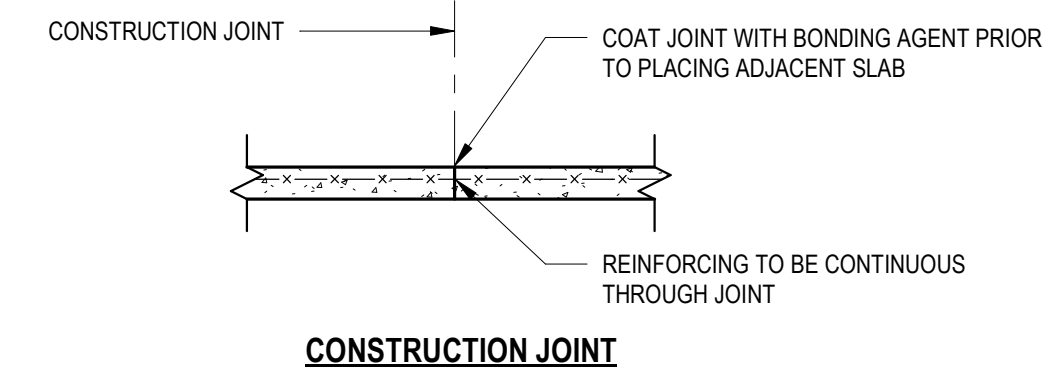
7 TYPICAL WOOD HEADER AT WALL OPENING
3/4" = 1'-0"



8 TYPICAL SLAB-ON-GRADE SECTIONS
1/2" = 1'-0"



9 TYPICAL SLAB-ON-GRADE JOINTS
1/2" = 1'-0"



Date	
Description	
No.	

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235 RT 120
PLAINFIELD, NH

TYPICAL DETAILS
NH PLAINFIELD LUNDY ADDITION

Designed By:	MAN
Checked By:	MS
Drawn By:	MAN
Scale:	As indicated
Date:	Issue Date